

IN THE CIRCUIT COURT OF THE 11TH JUDICIAL CIRCUIT IN AND FOR
MIAMI-DADE COUNTY, FLORIDA

COMPLEX BUSINESS LITIGATION
DIVISION

CLASS REPRESENTATION

**IN RE: CHAMPLAIN TOWERS
SOUTH COLLAPSE LITIGATION.**

CASE NO.: 2021-015089-CA-01

**DEFENDANT, 8701 COLLINS AVENUE CONDOMINIUM
ASSOCIATION, INC.'S RESPONSE IN OPPOSITION TO DEFENDANT,
8701 COLLINS DEVELOPMENT, LLC'S *EMERGENCY MOTION FOR
COURT APPROVAL OF ACCESS TO PROPERTY AND, ALTERNATIVELY,
MOTION TO SHORTEN TIME,*¹ AND MOTION FOR PROTECTIVE
ORDER**

Defendant, 8701 Collins Avenue Condominium Association, Inc.
(hereinafter "the 8701 Association"), by and through its undersigned counsel, files
its Response in Opposition to Defendant, 8701 Collins Development, LLC's
*Emergency Motion for Court Approval of Access to Property and, Alternatively,
Motion to Shorten Time*, and Motion for Protective Order, and as grounds for the
foregoing states as follows:

¹ The full title of Defendant, 8701 Collins Development, LLC's motion is as follows: *Emergency Motion for Court Approval of Access to Property Controlled by Defendant 8701 Collins Avenue Condominium Association, Inc. for the Purpose of Performing Joint Testing Protocol Testing and, Alternatively, Motion to Shorten Time to Respond to Request for Entry Upon Land for Inspection.*

1. On or about March 22, 2022, Plaintiffs served the 8701 Association with their Consolidated Third Amended Complaint, wherein they name the 8701 Association as a defendant in this action for the first time. Plaintiffs contend that the 8701 Association is liable for the alleged negligent design, construction, installation, and maintenance of the beach access walkway separating Eighty-Seven Park and the Champlain Towers South (“CTS”)—a meritless claim which has no statutory or common law basis, as control of the association was not transferred from developer unit owners to non-developer unit owners until May 14, 2021 (i.e., the month before the collapse).

2. The 8701 Association operates Eighty-Seven Park, a condominium. Moreover, the beach access walkway separating Eighty-Seven Park and the CTS was previously submitted to the 8701 Association and is considered Condominium Property. An easement exists which allows the public to traverse the beach access walkway.

3. Notwithstanding 8701 Collins Development, LLC’s designation of its motion as an “emergency motion,” 8701 Collins Development, LLC conveniently overlooks that it has had since at least January 21, 2022,² to secure access to Eighty-Seven Park. Had it undertaken such efforts months ago, it would have had

² January 21, 2022, is the date of the *Joint Protocol for Testing and Material Sampling – Collapse Site and [Proposed] Order Regarding Joint Protocol for Testing and Material Sampling – Collapse Site*.

ample time to provide the 8701 Association with the requested information and assurances. *Lack of diligence by 8701 Collins Development, LLC in securing access to Eighty-Seven Park does not warrant punishing the 8701 Association and prejudicing its defense of this matter.* Moreover, as discussed herein, the protocols and orders that 8701 Collins Development, LLC relied upon do not concern Eighty-Seven Park, and they do not grant access to this property.

4. On March 23, 2022, counsel for 8701 Collins Development, LLC advised the 8701 Association of proposed testing on the site of the CTS. Counsel for 8701 Collins Development, LLC indicated that the testing would require entry upon Eighty-Seven Park. Counsel did not specify when the testing would commence.

5. Also on March 23, 2022, the 8701 Association requested from counsel for 8701 Collins Development, LLC the protocols for the aforementioned proposed testing. In response, counsel for 8701 Collins Development, LLC furnished various court filings,³ none of which reflect that this Court has mandated that the 8701

³ Specifically, counsel for 8701 Collins Development, LLC provided the following:

- Champlain Towers South Condominium Association, Inc.'s Notice of Filing Joint Access Protocol for *Non-Invasive Review and Testing – Primary Evidence Facility* and [Proposed] Order Regarding Joint Access Protocol for Non-Invasive Review and Testing – Primary Evidence Facility (emphasis added);
- Order Regarding Joint Access Protocol for *Non-Invasive Review and Testing – Primary Evidence Facility* – Phase One, dated March 15, 2022 (emphasis added);

Association allow entry upon Eighty-Seven Park. Rather, the documents concern on-site and off-site materials, components, remnants, and debris of CTS. Significantly, the very *Joint Protocol for Testing and Material Sampling – Collapse Site and [Proposed] Order Regarding Joint Protocol for Testing and Material Sampling – Collapse Site* provided by 8701 Collins Development, LLC to the 8701 Association states as follows:

Nothing in this Protocol shall serve to confirm authorization or access to conduct inspections or testing on any property or portion thereof that is not under the ownership and control of the Champlain Towers South Condominium Association (in the “CTS Property”). There may be several boring, test pits and other tests anticipated under this Protocol that are not located on the CTS Property and are subject to access authorized by the property owner and/or Court approval.

See Composite Exhibit “A” (emphasis added).

6. Notwithstanding **repeated** requests by the 8701 Association for information regarding the scope of the requested entry upon Eighty-Seven Park, 8701 Collins Development, LLC has failed to provide it with the requested information:

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- Champlain Towers South Condominium Association, Inc.’s Notice of Filing Joint Protocol for **Testing and Material Sampling – Collapse Site** and [Proposed] Order Regarding Joint Protocol for Testing and Material Sampling – Collapse Site (emphasis added); and
 - Order Regarding Joint Protocol for **Testing and Material Sampling – Collapse Site**, dated January 21, 2022 (emphasis added).

See Composite Exhibit “A.”

- a. To date, the 8701 Association has not been informed of (i) the duration of the testing (including, but not limited to, the start and end times of the proposed testing, and dates on which access to Eighty-Seven Park is required), (ii) the machinery or equipment that will be taken onto Eighty-Seven Park, and (iii) the anticipated number of persons which will require entry upon Eighty-Seven Park.
- b. Furthermore, the 8701 Association has not received the requested reasonable assurances concerning potential liability for any personal injury or property damage which may occur during, or as a result of, the proposed testing. For instance, the 8701 Association has not received proof that the contractors/subcontractors selected to undertake the proposed testing—including, but not limited to, Geosyntec—are bonded. Nor has the 8701 Association received copies of certificates of insurance which would provide coverage to the 8701 Association for personal injury or property damage which may occur during, or as a result of, the proposed testing.
- c. Moreover, the 8701 Association has not received reasonable assurances regarding site safety and the measures which will be implemented to ensure same.

- d. Lastly, despite the 8701 Association's repeated requests for a protocol which is specific to Eighty-Seven Park, counsel for 8701 Collins Development, LLC has not provided same; rather, they have simply directed the 8701 Association's attention to the *Joint Protocol for Testing and Material Sampling – Collapse Site and [Proposed] Order Regarding Joint Protocol for Testing and Material Sampling – Collapse Site*, which, *as reflected in the very title of the protocol, is specific to the CTS site, not to Eighty-Seven Park.*

7. The aforementioned requested information and assurances are of monumental importance because Eighty-Seven Park is an occupied condominium. It is anticipated that the proposed entry and testing will be disruptive to residents (including families and children) and visitors. The residents are entitled to adequate notice and assurances.

8. On the basis of the foregoing, on April 13, 2022, the 8701 Association advised counsel for 8701 Collins Development, LLC that it would not consider testing on Eighty-Seven Park until it received the requested information and retained the appropriate experts.

9. The 8701 Association further notes that if this Court permits the proposed testing to occur on Eighty-Seven Park at this juncture, the 8701 Association will be unduly prejudiced in its defense of this matter. The 8701

Association was named as a defendant merely three weeks ago. It is still in the process of retaining all of the appropriate experts. Hence, allowing testing on Eighty-Seven Park to proceed on April 18th will preclude the 8701 Association the opportunity to have its experts present.

10. Under Fla. R. Civ. P. 1.350(b), “[t]he party to whom [a request for entry upon land for inspection and other purposes] is directed shall serve a written response within 30 days after service of the request.” (emphasis added.) Notably, requests made pursuant to Rule 1.350 “shall specify a reasonable time, place, and manner of making the inspection or performing the related acts.” The 8701 Association was served with 8701 Collins Development, LLC’s Request for Entry upon Eighty-Seven Park on April 14, 2022, past 10:00 P.M. *See* Exhibit “B.” The Request for Entry seeks access to Eighty-Seven Park on April 15, 2022—i.e., less than twenty-four-hours after serving the request. Moreover, as evidenced by Rule 1.350(b), the 8701 Association was unequivocally entitled to the requested information described in Paragraph 6 of the present Response—again, none of which it has been provided.

11. Also on April 14, 2022, 8701 Collins Development, LLC filed its *Emergency Motion for Court Approval of Access to Property Controlled by Defendant 8701 Collins Avenue Condominium Association, Inc. for the Purpose of Performing Joint Testing Protocol Testing and, Alternatively, Motion to Shorten*

Time to Respond to Request for Entry Upon Land for Inspection. See Exhibit “C.”

Therein, 8701 Collins Development, LLC requests that this Court (i) deny the 8701 Association its right to serve a response within 30 days of the Request for Entry, and (ii) mandate that entry be permitted on April 15, 2022, with testing to commence no later than April 18, 2022. In support of drastically curtailing the 8701 Association’s time to respond to the Request for Entry, 8701 Collins Development, LLC cites in Paragraph 1 of its emergency motion to this Court’s order requiring litigants to “‘promptly mobilize and begin invasive testing at the collapse site at the beginning of February’ and require[ing] that all such testing be ‘performed pursuant to the agreed upon Champlain Towers South Collapse Investigation: Joint Protocol for Testing and Material Sampling – Collapse Site (the ‘Joint Testing Protocol’)’” However, the 8701 Collins Development, LLC conveniently overlooks that it has had since at least January 21, 2022, to secure access to Eighty-Seven Park. Had it undertaken such efforts months ago, this would have allowed it ample time to provide the 8701 Association with the requested information and assurances. **Lack of diligence by 8701 Collins Development, LLC in securing access to Eighty-Seven Park does not warrant punishing the 8701 Association and prejudicing its defense of this matter.**

12. Pursuant to Fla. R. Civ. P. 1.280(c), “for good cause shown” a court “may make any order to protect a party or person from annoyance, embarrassment,

oppression, or undue burden or expense that justice requires,” including ordering that discovery not be had or that it be had “only on specified terms and conditions.” For the reasons discussed above, good cause exists for precluding entry and testing upon Eighty-Seven Park at this juncture: the 8701 Association (i) has not received the requested information and assurances regarding the proposed testing, (ii) is still retaining experts, and (iii) is being asked to grant access to Eighty-Seven Park less than twenty-four-hours after being served with a formal request pursuant to Fla. R. Civ. P. 1.350. Hence, allowing access to Eighty-Seven Park on April 15th—and allowing testing on Eighty-Seven Park to commence on April 18th—would impose an undue burden on the 8701 Association.

WHEREFORE, Defendant, 8701 Collins Avenue Condominium Association, Inc., respectfully requests that this Honorable Court **deny** 8701 Collins Development, LLC’s *Emergency Motion for Court Approval of Access to Property and, Alternatively, Motion to Shorten Time*, and that it **grant** 8701 Collins Avenue Condominium Association, Inc.’s Motion for Protective Order.

CERTIFICATE OF SERVICE

I certify that a copy hereof has been furnished to all counsel of record, by e-mail, on April 15, 2022.

**FALK, WAAS, HERNANDEZ, SOLOMON,
MENDLESTEIN & DAVIS, P.A.**

*Attorneys for Defendant, 8701 Collins
Avenue Condominium Association, Inc.*

135 San Lorenzo Avenue, Suite 500

Coral Gables, FL 33146

Telephone: (305) 447-6500

Facsimile: (305) 447-1777

Email: serviceehernandez@falkwaas.com

servicegfalk@falkwaas.com

servicejhernandez@falkwaas.com

By: /s/ *Edward Hernandez*

Edward Hernandez

Florida Bar No.: 527475

Glenn P. Falk

Florida Bar No.: 161669

Jessica M. Hernandez

Florida Bar No.: 1025436

Composite Exhibit “A”

**IN THE CIRCUIT COURT OF THE ELEVENTH JUDICIAL
CIRCUIT IN AND FOR MIAMI-DADE COUNTY, FLORIDA**

CASE NO: 2021-015089-CA-01

SECTION: CA43

JUDGE: Michael Hanzman

In re:

Champlain Towers South Collapse Litigation.

**NOTICE OF FILING *JOINT PROTOCOL FOR TESTING AND MATERIAL SAMPLING –
COLLAPSE SITE* AND [PROPOSED] ORDER REGARDING *JOINT PROTOCOL FOR
TESTING AND MATERIAL SAMPLING – COLLAPSE SITE***

Champlain Towers South Condominium Association, Inc. (the “Association”), through its Court-appointed Receiver Michael I. Goldberg, hereby files the *Joint Protocol for Testing and Material Sampling – Collapse Site* (Exhibit **A**) and proposed *Order Regarding Joint Protocol for Testing and Material Sampling – Collapse Site* (Exhibit **B**).

Dated: January 21, 2022

Respectfully submitted,

AKERMAN LLP

201 East Las Olas Boulevard, Suite 1800

Fort Lauderdale, Florida 33301-2229

Telephone: (954) 463-2700

Facsimile: (954) 463-2224

By: /s/ Andrew P. Gold

Andrew P. Gold, Esq.

Florida Bar No. 612367

Primary email: andrew.gold@akerman.com

Secondary email: jill.parnes@akerman.com

Christopher Carver, Esq.

Florida Bar No. 993580

Primary email: christopher.carver@akerman.com

Secondary e-mail: cary.gonzalez@akerman.com

and

Brenda Radmacher, Esq.

California Bar No. 185265

Admitted Pro Hac Vice

brenda.radmacher@akerman.com

AKERMAN LLP
601 West Fifth Street, Suite 300
Los Angeles, California 90071
Attorneys for Receiver/Association

CERTIFICATE OF SERVICE

I HEREBY CERTIFY that on January 21, 2021, a true and correct copy of this *Notice of Filing Joint Protocol for Testing and Material Sampling – Collapse Site and [proposed] Order Regarding Joint Protocol for Testing and Material Sampling – Collapse Site* was filed electronically through the Florida Court’s E-Filing Portal, which will provide electronic service of the filing to all counsel of record.

By: /s/ Christopher S. Carver
Attorney

Exhibit A

CHAMPLAIN TOWERS SOUTH COLLAPSE INVESTIGATION:
Joint Protocol for Testing and Material Sampling – Collapse Site

In accordance with the Court Order dated September 1, 2021, this document presents the protocol for testing and material sampling at the collapse site (the “Protocol”) and has been developed considering input from involved Participants.

DEFINITIONS

- **Consultant** – Geosyntec Consultants, Inc., an independent consultant approved by the Receiver to coordinate, plan, oversee, and provide factual reporting as defined in this Protocol to the Experts/Participants. The Consultant will employ Testing Agencies and Contractors to meet the goals of the Protocol. Payment to Consultant for its fees and costs shall be paid by all Participants as defined in the "Payment of Costs" section below.
- **Contractor** – Company, or companies, approved by the Receiver to engage in providing construction, samples, borings, security, or other support for execution of the Protocol. The Contractors shall be engaged by the Consultant. Payment for services shall be remitted to Consultant for payment to Contractors. Payment for Contractors' services shall be paid by all Participants as defined in the "Payment of Costs" section below.
- **Court** – Circuit Court of the 11th Judicial Circuit in and for Miami-Dade County, Florida.
- **Expert** – Engineers (or other disciplines) investigating the collapse on behalf of Participants.
- **Litigation** -- *In Re: Champlain Towers South Collapse Litigation*, Case No. 2021-015089-CA-01, pending in the Eleventh Judicial Circuit Court in and for Miami-Dade County, Florida
- **Participant** – A party or entity authorized or defined by the *Order Approving Protocol for Inspection, Documentation, and Storage of Components, Remnants, and Debris of The Champlain Towers South Collapse*. Notwithstanding anything to contrary herein, non-parties to the Litigation are not Participants; *however*, such non-parties may participate in the testing and investigations set forth in this Protocol if authorized to do so by the Court.
- **Protocol** – This document, titled *CHAMPLAIN TOWERS SOUTH COLLAPSE INVESTIGATION: Proposed Joint Protocol for Testing and Material Sampling – Collapse Site*.
- **Receiver** – Michael I. Goldberg, Esq., on behalf of the Champlain Towers South Condominium Association, Inc.
- **Testing Agency** – An independent company, approved by the Receiver and working under the direction of the Consultant and/or Receiver to facilitate surveying, sampling, borings, materials testing, and report factual results. The Testing Agency will use Contractors as necessary to carry out the Protocol. The Testing Agencies shall be engaged by the Consultant. Payment to Testing Agencies for their services shall be paid by all Participants as defined in the "Payment of Costs" section below.

GENERAL

Scope. The Protocol applies to the Champlain Towers South site. A separate or amended protocol for testing and materials sampling at the primary evidence facility will be developed later.

Objective. The Protocol is intended to provide reliable *factual* information needed to support the collapse investigations by the Experts, including but not necessarily limited to the following:

- Mechanical, physical, and chemical characteristics of construction materials
- Subsurface conditions, including soils, rocks, water, pile caps, and piles

- Physical configuration of structural and waterproofing components, including the plaza deck, perimeter foundation walls, shear walls, columns, and basement slab.

Reporting. It is envisioned that the Consultant, who does not represent Participants, will obtain this factual information. The factual information will be shared among Participants. Except as noted below, reporting to Participants shall include factual information only (without interpretation or opinion), including the following:

- Testing standards and methodology
- Documentation of the locations and extraction of samples for materials testing
- Test results in a format usable for engineering analysis and interpretation
- Given that geophysical testing requires interpretation to prepare result reports, one report for each geophysical test shall be prepared for the benefit of all parties. Geophysical consultants shall prepare a written narrative that shows the basis for any interpretation in each report.

Project Updates. The Consultant will conduct daily meetings at the end of each day at the site to discuss the work completed on that day and the work anticipated for the following day. Additionally, the Consultant will conduct weekly virtual meetings to discuss the work completed during that week and the multi-week look-ahead planned work. The Consultant will issue project updates weekly. The project updates will include:

- Field testing/sampling
 - Anticipated start dates and testing duration
 - At least one week of notice prior to commencement of testing
- Laboratory testing
 - Anticipated start dates and testing duration
 - At least one week of notice prior to commencement of testing
- Issues/deviations encountered at site
- Daily site visit reports (to be uploaded to a secure Extranet)

Photos and videotaping of site sampling operations; videotaping of laboratory testing shall be conducted and agreed upon for each laboratory test.

Extranet. The Consultant will provide data, photos, videos, and other factual information on a secure FTP/Sharepoint site (or equivalent).

Access to Site. The site is located at 8777 Collins Ave. in Surfside, Florida. The entrance and security check-in trailer are in the northeast corner of the site. Participants, Experts, Testing Agencies, Contractors, and other site visitors seeking access shall comply with the following:

- Submit all requests for access to the site through the Receiver.
- Notify the Receiver at least two (2) business days before the planned site visit.
- Sign a waiver releasing the Receiver of liability and provide proof of liability insurance coverage, naming the Receiver as additional insured.
- Comply with established procedures for signing in and signing out. These procedures include but are not limited to surrendering driver's licenses and wearing wristbands during each visit.

Observation of Site Sampling and Testing. Participants and Experts shall be permitted to observe sampling and testing conducted at the site and laboratories as agreed upon by all Participants prior to testing. For laboratory testing, virtual observation of testing will be provided and the ability to comment on test procedure for the first of any certain type of test, Participants shall be permitted to visit a lab and observe the first test. The first test of each test type shall also be videorecorded and promptly shared with the Participants for review and comments. The entirety of any test performed at a laboratory on a sample or material must be recorded by video if the sample or material either (i) cannot be reconstituted or retested, or (ii) will be altered by reason of the performance of the

test. Site testing can be observed in person, and any objections to the methodology can be raised on site, during daily meetings, and/or weekly virtual meetings.

Safety. All site visitors shall comply with state and federal safety regulations and the site safety protocol established by the Consultant. The Receiver may prohibit an individual from accessing the site in the event such individual fails to adhere to state and federal safety regulations and the site safety protocol established by the Receiver.

Samples. Except as permitted herein or expressly approved by the Receiver for additional testing, samples of any materials shall not be taken or removed from the site.

Reserved/Backup Samples. In addition to samples designated for testing under the Protocol, additional samples shall be taken and reserved for additional testing to address failed tests, ambiguous results, or disputed results. These samples shall be identified in the Testing Plans.

Testing Plans. Consultant and its Contractors and testing agencies shall prepare a detailed testing plan of each field and laboratory test to be performed meeting the requirements of this Protocol. The testing plans shall be shared for review by all Participants prior to executing the work.

If consensus is not reached, any dissenting Participant may bring the disagreement to the Receiver's attention for resolution. If a resolution is not achieved, counsel for the Participant may choose to bring the disagreement to the Court for resolution.

Chain of Custody. All sample extraction and preparation in the field will be documented by photo and videotaping as agreed upon by the Participants. Samples will be extracted, protected per appropriate testing standards, and labeled. A chain of custody form will be issued for each sample/specimen. Samples will be placed in appropriate containers for the intended laboratory testing. After testing, the samples will be stored at a secure location (TBD) and preserved for future testing or observation.

Additional or Specialized Testing. Each Participant may, if they choose, spend additional days on site after the joint testing is complete to take additional samples for private testing and analysis by a laboratory of their choosing or to conduct additional testing on site. The Consultant will notify the Participants when the sampling for the Protocol is complete and such additional testing can begin. If the services of the Consultant are needed for additional or specialized testing, Participants will submit additional or specialized testing requests in writing to the Consultant. The Consultant will coordinate sampling and scheduling of samples. Any such additional samples taken or testing conducted over the additional days on site shall occur immediately after work performed under this Protocol. The Consultant/Contractor will remain on site following the conclusion of this Protocol for any Participant that wishes to take additional samples. Any additional or specialized testing, or any portion of it, may be completed by the Participant's own consultants and contractors without the participation of the Consultant (1) should the Participant so choose at its discretion, or (2) should the Consultant and its contractors lack the capacity, expertise or preparation to accomplish the additional testing. The Receiver shall be consulted for the scheduling of any additional or specialized testing and the Receiver shall make the site available for such additional or specialized testing coordinated with the Consultant and the Receiver. If any non-party Participant wishes to conduct additional testing, leave of Court is required. All Participants may observe additional specialized testing at the site.

For geotechnical testing using the Consultant's services, any Participant that chooses to remain on site following the conclusion of this Protocol for the purpose of collecting additional samples will be limited to additional borings at locations to be identified and provided in advance to the Consultant to be conducted within the agreed upon period for additional testing. The location(s) where any additional samples are taken from will be appropriately documented by the Consultant. For concrete and rebar sampling, the Participants shall be authorized to take companion samples of material taken under this Protocol during the agreed upon period for additional testing or as required by the Consultant or, depending upon the sequencing of work, while joint geotechnical testing is underway provided that the taking of additional concrete and rebar sampling will not interfere with other work on site.

The Receiver will permit Non-Destructive Testing to be coordinated and proceed before sampling.

Payment of Costs. Except for the costs attributable to additional or specialized testing, which costs are the sole responsibility of the Participant seeking such additional or specialized testing, each Participant's or other third-party's allocation of the costs incurred through performance of this Protocol shall be determined pursuant to an order from the Court.

Audio and Video Recording. Some or all of the work performed pursuant to this Protocol will be recorded via video and/or audio. Although the purpose of the video and/or audio recordings is to document and record the work performed pursuant to this Protocol, it is understood that said video and/or audio recordings may also capture conversations or statements made by on-site Participants and/or the Consultant. It is expressly agreed between the Participants that any conversations or statements that are captured and/or recorded in the video and/or audio recordings of the work performed pursuant to this Protocol shall not be admissible at trial for any purpose and may not be used by any Participant in this litigation or any other legal or administrative proceeding, including but not limited to at depositions, in hearings, or in any filings.

Water Control. Until sampling and site testing is complete, the Receiver will continue to arrange for removal of excess water that accumulates on the lower-level slab from leakage and rain. The Consultant shall implement a comprehensive dewatering program throughout the entirety of the slab and as otherwise as necessary throughout the site.

General. Nothing in this Protocol shall serve to confirm authorization or access to conduct inspections or testing on any property or portion thereof that is not under the ownership and control of the Champlain Towers South Condominium Association (in the "CTS Property"). There may be several boring, test pits and other tests anticipated under this Protocol that are not located on the CTS Property and are subject to access authorized by the property owner and/or Court approval.

TESTING AGENCIES AND CONTRACTORS

Testing Agencies and Contractors extracting samples shall be named (including credentials) by the Consultant and selected by consensus among the Participants for the testing scope listed in Table 2. Laboratories performing the work shall be ISO 17025, AASHTO R 18 or USACE accredited (or acceptable equivalent) for the tests to be performed. Persons performing the work shall have documentation/training to perform the specific tests and/or tasks, (e.g., ACI certified laboratory technician performing compressive strength tests on concrete). All testing shall, at a minimum, meet the industry standard of care. The Consultant will coordinate the extraction and delivery of samples to Testing Agencies and collect and deliver testing reports to the Receiver. A Participant may request permission from the Receiver to visit (in person or virtually) one or more testing agency to examine samples and view testing procedures. Such visits will be coordinated by the Receiver and Consultant.

Table 1. Testing Agencies and Contractors

Specialty	Testing Scope	Testing Agency/Contractor [website]
Structural	Demolition	To be determined
	Coring	To be determined
	Surveying	To be determined
Surveyor	Surveying	To be determined
Laboratory Testing	Concrete	To be determined
	Metals	To be determined
Petrography	Petrography	To be determined
Non-Destructive Testing	Ground penetrating radar	To be determined
Geophysics	MASW/ER	To be determined
	Seismograph	To be determined

Specialty	Testing Scope	Testing Agency/Contractor [website]
Geotechnical	Borings and observation wells	To be determined
	Test pits	To be determined
	Pile integrity testing (PIT)	To be determined
	Crosshole seismic testing	To be determined
	Laboratory testing	To be determined
	Seismic CPT testing	To be determined
	Parallel seismic testing for deep foundations and sheet piles	To be determined

TESTING METHODS AND STANDARDS

The Consultant, Testing Agencies, and Contractors shall adhere to the following standards for tests carried out. Proposed deviations from the standards shall be brought to the attention of all Participants and Experts through the Receiver. In general, non-destructive testing will occur before sampling. In addition, the following should be noted:

- General descriptions of the test methods are provided in Appendix A.
- Proposed testing locations are provided in Appendix B.

Table 2. Testing Standards

Category	Test	Destructive	Standard/Guide
Concrete Materials	C1 Compressive strength	Yes	ASTM C39 and C42
	C2 Elastic modulus and Poisson's ratio	Yes	ASTM C469
	C3 Splitting tensile strength	Yes	ASTM C496
	C4 Air void parameters	Yes	ASTM C457
	C5 Petrography*	Yes	ASTM C856 and C1723
	C6 Chloride profile	Yes	ASTM C1218 and C1152
	C7 Ionic Species (e.g., sulphur, magnesium, sodium, and potassium) profile	Yes	ASTM C114
Reinforcing Steel	R1 Reinforcing bar location survey using ground penetrating radar (GPR)	No	ASTM D6432 and ACI 228.2R-13
	R2 Tensile properties	Yes	ASTM A615 and ASTM A370
	R3 Rockwell hardness	Yes	ASTM E18
	R4 Chemistry	Yes	ASTM E415, ASTM A751
	R5 Section Loss / Pitting	Yes	ASTM G46, ACI 364.14T-17
Geotechnical	G1 Soil/rock borings**	Yes	ASTM D1586, FDOT Soils and Foundation Handbook (as applicable) Participants request including pitcher samplers (ASTM D7015, ASTM D1587), X-ray of undisturbed samples (ASTM D4452), rock coring with inner spring barrel (ASTM D2113), RQD (D6032). Soil/rock characterization (ASTM D2487, D2488, and D5878). Permeability tests (ASTM D5912)
	G2 Observation well	Yes	ASTM D5092; ASTM D5912.
	G3 Test pit excavation	Yes	N/A
	G4 Pile integrity testing (PIT)	Yes	ASTM D5882
	G5 Crosshole seismic testing	No***	ASTM D4428
	G6 Grain size analysis	Yes	ASTM D6913, ASTM D7928
	G7 Moisture content	Yes	ASTM D2216
	G8 Organic content	Yes	ASTM D2974
	G9 Incremental consolidation	Yes	ASTM D2435
	G10 Chlorides	Yes	AASHTO T291
	G11 Electrical resistivity	Yes	ASTM G57
	G12 pH	Yes	ASTM D4972 or G51
	G13 Sulfates	Yes	ASTM D516
	G14 Sulfides	Yes	SM4500
	G15 Unconfined compression of rock	Yes	ASTM D7012C
	G16 Unit weight of Rock Cores	Yes	ASTM D7263
	G17 Specific Gravity	Yes	ASTM D854, ASTM D5778
	G18 Seismic CPT	Yes	ASTM D5778
	G19 Parallel Seismic Testing	Yes	ACI 228.2R and ASTM D8381
	G20 Drained and Undrained Triaxial Testing of Fine-Grained Soil Samples	Yes	ASTM D7181, ASTM D4767, ASTM D2850
	G21 Seismograph	No	N/A

Category	Test	Destructive	Standard/Guide
	G22 MASW/ER	No	NA
* Due to the nature of Petrographic testing samples will be provided to labs as requested by Participants and available			
**Includes SPT, and obtaining and testing of soil and rock samples			
***Uses borings; however, the test is non-destructive			

CONCRETE SAMPLING AND TESTING PLAN

The table below lists tests of concrete core samples. Final locations are to be determined by the consensus of the Experts and depend on site conditions and locations of interest and shall be provided to the Receiver at least seven (7) days prior to the commencement of the concrete core sampling.

Table 3. Concrete Sampling and Testing Plan

Core Sample Phase/Location	Element	Original Element Location	Number of Cores***	Test Type and Number (See Table 2)				
				C1 Comp.	C2 E & v	C3 Tens.	C4 & C5* Voids/Petro	C6 & C7 Cl & Ionic Species
Site	Slab	Basement	33	13	3**	3	3	3
Site	Perimeter Wall	Basement	33	13	3**	3	3	3
Site	Pile Cap	Below Basement Slab	22	6	4	4	4	0
Site	Pile	Below Pile Cap	22	6	4	4	4	0
Site	Pool Deck Slab (remaining area at SE corner of site, including all layers (i.e., structural slab, topping, waterproofing, pavers)	Pool Deck at Lobby Floor	8	3	1**	2	1	1
Site	Shear Walls	Gridline M at Basement and West Core	12	6	2**	0	2	0
Site	Columns (rubble within extant rebar)	Various columns on gridlines E, L, M, O, and Q	15	0	0	0	10	0
Site	Beams	Between Gridlines N-O and 8-10	9	6	2	0	2	0
Total***			154	53	19	16	29	7

*Tests C4 and C5 will be conducted on the same core sample; separate core samples are needed for all other tests.

**One of the three cores used for compressive strength will also be used to determine modulus of elasticity and Poisson's ratio.

***Collect spare samples at each location for backup/additional testing (Consultant/Testing Agency to provide plan)

REINFORCING STEEL SAMPLING AND TESTING PLAN

Table 4 lists the general location and number of samples. Final locations are to be determined by the consensus of the Experts and depend on site conditions and locations of interest and shall be provided to the Receiver at least seven (7) days prior to the commencement of the steel sampling.

Table 4. Reinforcement Sampling and Testing Plan

Bar Sample Location	Element	Original Element Location	Bar Size	Number of Bars*****	Test Type and Number 8 (See Table 2)				
					R1* GPR	R2** Tens.	R3** Hard.	R4** Chem.	R5 Sect. Loss
Site	Slab	Basement	#5	6	4	6	3	1	6
Site	Wall	Basement	#5	23	4	23	23	4	23
Site	Column	Basement	N/A	N/A	2	0	0	0	0
Site	East Shear Wall	Basement	#10/#11	0	N/A	0	0	0	0
Site	Slab	Pool Deck	#4/#5****	16	0	16	16	4	16
Site	South Wall	Pool Deck	#5*****	10	0	0	0	0	10
Site	Columns (rebar protruding through slab)	Type N Columns***	#7	3	N/A	3	3	1	3
Site	Columns (rebar protruding through slab)	Various Type A and L Columns	#10	3	N/A	3	3	1	3
Site	Columns (rebar protruding through slab)	Various Type C, D, and G Columns	#11	3	N/A	3	3	1	3
Site	Column	Various Ties from Columns	#3/#4/#5	3	N/A	3	3	1	3
Total				67	10	57	54	13	67

*Non-Destructive Testing (approximately 10 ft x 10 ft area)

**Tests R1, R2 and R3 to be conducted on the same bar sample

***No. 7 rebar remaining in the slab at columns that supported the pool deck will be over-cored and removed for inspection only.

****Two bars top and bottom bars in each direction

*****Exposed Vertical Bars, perform fractography examination of exposed bar ends.

*****Collect spare samples at each location for backup/additional testing (Consultant/Testing Agency to provide plan)

ADDITIONAL STRUCTURAL INSPECTION/SAMPLING PLAN

The following inspections and samples shall be taken by the Consultant and examined on site or in the lab as applicable:

1. Plaza pool deck investigative work, including five (5) exploratory borings in the overburden, examination, field testing, and sampling of the waterproofing for laboratory testing. NOTE: This waterproofing sampling, field testing, and exploratory work must happen before destructive work on the underlying structure.
2. Pool deck slab N/S construction joint probe:
 - a. Provide additional pool deck slab shoring as required;
 - b. Shore top edge of adjoining privacy wall against lateral out-of-plane movement as required;
 - c. Locate all bars passing through construction joint along a 4 ft length of construction joint;
 - d. In the presence of all interested parties and under video observation, sawcut and remove a 4 ft (north-south) x 3 ft (east-west) slab of the existing pool deck slab and topping slab along the joint, taking care not to damage or drop the slab during sawcutting and removal;
 - e. Two (2) sawcut openings in the west edge of the remaining pool deck plaza to expose the cold joint assembly;
 - f. One (1) sawcut opening in the west edge of the remaining pool deck plaza to expose the slab-to-hot tub wall connection;
 - g. One (1) sawcut in the north edge of the remaining pool deck plaza to expose the slab-to-corbel connection;
 - h. Seal the preserved slab in heavy plastic and set aside for retrieval by laboratory for controlled and video-documented deconstruction; and
 - i. Measure and photo document the remaining sawcut cross section of the construction joint.
3. Privacy wall probe:
 - a. Provide additional pool deck slab shoring as required;
 - b. Shore top edge of adjoining privacy wall against lateral out-of-plan movement as required;
 - c. In the location where the pool deck slab is remaining west of the pool, GPR a 4 ft wide section (wider if necessary) of the CMU wall to locate the wall vertical bar(s) and dowels,
 - d. Make two vertical sawcuts, 4 ft apart and capturing the location of the vertical bar(s), to isolate CMU wall to be inspected/demolished from remainder of privacy wall;
 - e. In the presence of all interested parties and under video observation, sawcut and remove a 3 ft (east-west) x 3 ft (tall) portion of the existing wall, taking care not to damage or drop the wall during sawcutting and removal;
 - f. Seal the preserved wall portion in heavy plastic and set aside for retrieval by laboratory for controlled and video-documented deconstruction; and
 - g. Measure and photo document the remaining sawcut cross section of the wall.
4. South foundation wall to pool deck slab joint probe:
 - a. Provide additional pool deck slab shoring as required;
 - b. Shore top edge of adjoining privacy wall against lateral out-of-plan movement as required;
 - c. Within the 4 ft wide privacy wall probe, GPR a 3 ft x 3 ft area of the pool deck slab and a 3 ft x 3 ft area of the supporting wall to locate the rebar;
 - d. In the presence of all interested parties and under video surveillance, sawcut a 3 ft (north-south) x 3 ft (east-west) slab of the existing pool deck slab and topping slab along the joint, taking care not to damage or drop the slab during sawcutting;
 - e. After sawcutting through the slab, in the presence of all interested parties and under video observation, continue to sawcut the supporting wall and remove the L-shaped portion of the slab-to-wall connection as shown in page 6 of the attached figures;
 - f. Seal the preserved wall portion in heavy plastic and set aside for retrieval by laboratory for controlled and video-documented deconstruction;
 - g. Measure and photo document the remaining sawcut cross section of the slab and wall;
 - h. Provide six (6) test pits (as shown on SK-03) to expose the beach access walkway assembly and the top of the south perimeter sheet pile wall driven for the construction of 8777 Collins Ave. Note,

- these are not test pits to the bottom of the foundation wall;
- i. Provide six (6) exploratory openings in the exposed interior face of the southern perimeter basement wall that align with the test pits in item (h). Exploratory openings shall expose the sheet pile wall and embedded steel reinforcement; and
 - j. Visually survey and use a carpenter/mason's hammer sound the exposed interior face of the wall to map cracks and detect delamination as described in ASTM D4580. At delaminated locations perform selective demolition of the concrete perimeter wall from the basement interior to expose the sheet pile wall and embedded steel reinforcement. Remove one (1) sample of embedded reinforcement steel and sheet pile wall at each opening location.
5. Measure the depth of the foundation wall sheet piling at two locations along the south wall using parallel seismic testing (cross-reference test G19);
 6. Survey top edge geometry of sheet piling at 20 ft on center maximum, capturing all abrupt elevation changes (absolute elevation or elevation relative to bottom of pool deck slab):
 - a. Where required, chip the concrete to expose the top of steel sheet pile;
 - b. Optically survey the top of the sheet piles at 20 ft on center maximum, capturing all abrupt elevation changes;
 - c. Optically survey the elevation of the bottom of pool deck slab near the pool;
 - d. Reference the surveyed elevations to a datum.
 7. Take 1 ft x 1 ft samples of sheet pile at two locations along the south wall for measuring thickness, material properties, and corrosion quantity and quality (uniform loss or pitting);
 8. Chip concrete at each shear wall boundary zone to expose boundary element ties for observation and documentation by all interested parties;
 9. Provide test pit at undocumented column at grid O/8.5;
 10. Provide two (2) exploratory openings to expose the top of the west perimeter sheet pile wall driven for the construction of 8777 Collins Ave;
 11. Provide two (2) exploratory openings to expose the top of the north perimeter sheet pile wall driven for the construction of 8777 Collins Ave;
 12. Provide two (2) exploratory openings to expose the top of the east perimeter sheet pile wall driven for the construction of 8777 Collins Ave;
 13. Provide two (2) exploratory openings to expose the slab-to-wall connection at the bottom of the south perimeter basement wall.
 14. Provide one (1) exploratory opening to expose the slab-to-wall connection at the bottom of the north, west and east perimeter basement wall; and
 15. Perform level survey of the basement slab on grade capturing slab elevation at all column locations, adjoining location at the perimeter wall as applicable, shear walls, and the midspans of bays. In addition, the surveyed elevations to a datum must be referenced.

GEOTECHNICAL INVESTIGATION PLAN

The following geotechnical plan is developed as a basis for understanding site conditions and the foundation system. The test details and standards are to be developed based on the Experts' input and the Consultant engaged in carrying out the plan. The table lists general locations of borings and tests/samples. Final locations are to be determined by consensus of the Participants and will depend on site conditions and locations of interest. Details on sampling within the borings are shown in the attached plan (Appendix B) and described in section *G1 Soil/Rock Borings* (Appendix A).

The locations and numbers of tests for G6-G17 will be recommended by the Experts in consultation with the Consultant and approved by the Receiver and/or the Court. Note: Nothing in this Protocol shall serve to confirm authorization or access to conduct inspections or testing on any property or portion thereof that is not CTS Property. There may be several boring and test pits anticipated under this Protocol that are not located on the CTS Property and are subject to access authorized by the property owner and/or Court approval. The Protocol schedule shall be adjusted if circumstances outside of the Receiver's, the Consultant's, or the Participants' control, such as, but not limited to, permitting delays, access to adjoining properties, contractors' availability, equipment and material shortages, damage to samples collected, weather events, or COVID-related issues, materially impact the schedule.

Table 5. Geotechnical Sampling and Testing Plan – Receiver to coordinate any request for additional testing by site purchaser

Sample/Test Location	Test Type and Number*** (See Table 2)						
	G1 Borings	G2 Obs. Wells	G3* Test Pits	G4 PIT	G5 Crosshole Seis.	G18 CPT	G19 PSL
Slab Corners	0	0	0	0	0	0	0
Within Building Footprint	33	6	23	22	5	12	0
Outside Building Footprint	7	1	5	5	0	2	4
Total	40	7	28	27	5	14	4

*Core drill through pile cap and full length of pile

**Non-Destructive Testing

SCHEDULING AND ESTIMATED TIME REQUIRED ONSITE

The table below shows the estimated start date and time on site required for sampling and testing.

Table 6. Estimated testing start date and time required onsite

Sampling/Test	Estimated Start Date	Estimated Time Required on Site
Concrete Sampling	TBD	TBD
Steel Sampling	TBD	TBD
Concrete/Steel Laboratory Testing	TBD	TBD
G1 Soil Borings, G2 Observation Wells, G5 Crosshole Seismic, G18 Seismic CPT, G20	TBD	TBD

G3 Test Pits, G4 Pile Integrity Testing, G19 Parallel Seismic	TBD	TBD
G6-G17,G20 Laboratory Testing	TBD	TBD
G21, G22 MASW/ER, Seismographs	TBD	TBD

APPENDIX A

This appendix is intended to provide a brief description and outline requirements for each testing method in the Protocol. These descriptions should not be considered comprehensive and Participants should consult with their Experts for further information. Note: for all testing, the most recent and applicable testing standards and guides are to be used.

SITE SAFETY

The safety and wellbeing of all individuals on site, as well as the safety and wellbeing of those that may be impacted by the work performed under this Protocol shall at all times be the first and foremost priority of all individuals and companies involved in the performance of work. All work shall be performed in accordance with all local, state and federal rules and safety regulations and guidelines, including OSHA. Consultant will develop a site safety plan for the work which anyone entering the site must accept and adhere to. If anything in this Protocol calls for work to be done in a manner deemed unsafe by the Consultant or any Contractor, then said Consultant or Contractor shall not proceed with the work and all parties shall discuss how to proceed more safely. The Participants shall not be responsible for the implementation for any safety program nor have any responsibility for site safety, with the exception of making sure that they themselves comply with the safety program implemented by the Consultant. The Consultant may request that Receiver expel a Contractor or Participant that violates the safety plan.

CONCRETE TESTING

Concrete Core Removal

All core samples shall be taken using a 4-inch diameter core barrel (core diameter = \pm 3.7-inch) or largest barrel suitable to fit between bars at a specific location. Each sample shall be marked, measured (diameter and overall length), described, and photographed, including the size and condition of any embedded reinforcement.

Core samples shall be taken and conditioned per ASTM C42. Before coring, the surface shall be sounded with a hammer as described by ASTM D4580, and reinforcing bars in the vicinity shall be located using ground penetrating radar (GPR) or pachymeter. Cores for petrographic examination will be taken through near-surface reinforcing bars, especially where delamination planes have been identified by sounding. Otherwise, coring operations should avoid reinforcement. Core holes shall be inspected to determine if any cracks in the core samples are also present in the concrete. Cores intended for compression testing or splitting tensile testing shall be retaken if the cores include reinforcement, large voids, or cracks that preclude obtaining a 6-inch long sawn sample without such characteristics. After cores have been drilled, wipe off surface drill water and allow remaining surface moisture to evaporate. When surfaces appear dry, but not later than 1 hour after drilling, place cores in separate plastic bags or nonabsorbent containers, seal to prevent moisture loss, and label for identification.

Core holes through the Level 1 slab and basement wall shall be patched as required to prevent groundwater entry. Table 4 lists the general location and number of cores. Concrete core samples will be extracted with a diamond impregnated, water-cooled core drill. After removal, the core holes will be photographed and then patched with a rapid-setting, cementitious repair material. The condition and location of each core will be documented with photographs and notes.

C1 Compressive Strength

To determine concrete compressive strength, concrete cores shall be removed using a coring drill, trimmed, prepared for testing using sulfur capping, and loaded in a loading frame at a rate within a specified range. The compressive strength of the specimen will be calculated by dividing the maximum axial load sustained by the sample by the cross-sectional area of the specimen. Concrete core samples will be prepared and tested according to ASTM

C42 *Standard Test Method for Obtaining and Testing Drilled Cores and Sawed Beams of Concrete* and ASTM C39 *Standard Test Method for Compressive Strength of Cylindrical Concrete Specimens*

C2 Elastic Modulus and Poisson's Ratio

These parameters will be measured using appropriate instrumentation in a loading frame during compressive loading of the specimens. Elastic modulus and Poisson's ratio will be measured according to ASTM C469 *Standard Test Method for Static Modulus of Elasticity and Poisson's Ratio of Concrete in Compression*.

C3 Splitting Tensile Strength

Splitting tensile strength testing provides measurements to calculate the concrete tensile strength of the tested specimen. This test method consists of applying a diametral compressive force at a rate within a prescribed range along the length of a concrete core specimen in a load frame until failure occurs. The maximum load sustained by the specimen is divided by appropriate geometrical factors to obtain the splitting tensile strength. Splitting tensile strength will be measured in accordance with ASTM C496 *Standard Test Method for Splitting Tensile Strength of Cylindrical Concrete Specimens*.

C4 Air Void Parameters

Determining air void parameters involves examining a concrete cross-section under a microscope. One of three procedures will be used to determine air void parameters, including specific surface, void frequency, spacing factor, and paste air ratio of the air-void system in hardened concrete:

1. Linear-traverse method
2. Modified point-count method
3. Contrast-enhanced method

Air void parameters will be determined in accordance with ASTM C457 *Standard Test Method for Microscopical Determination of Parameters of the Air-Void System in Hardened Concrete*.

C5 Petrography

A petrographic examination consists of a series of observations of hardened samples that are interpreted to draw conclusions about the composition, quality, water to cement ratio, and probable cause(s) of potential problems associated with concrete or other construction materials. To conduct a petrographic examination, a core sample will generally be sliced in half lengthwise. One of the resultant halves will be lapped (polished) to accentuate the appearance of the components of the concrete (air voids, aggregates, and cement paste). Additional thin sections and smaller polished sections can also be made for petrographic microscope and scanning electron microscope (SEM) evaluations of cement paste and including microscopic chemical analysis.

A powder mount is obtained by scratching off a very small portion of the paste and mounting the resultant powder on a glass microscope slide in immersion liquid. Examination of a powder mount allows the properties of the cement paste to be studied in detail. Petrographic examination will be carried out in accordance with ASTM C856 *Standard Practice for Petrographic Examination of the Hardened Concrete* and ASTM C1723 *Standard Guide for Examination of Hardened Concrete Using Scanning Electron Microscopy*.

All samples and epoxied discs shall be preserved for review by other experts if needed and access granted when requested. The petrographic report shall include descriptions of the method(s) of preparation of the samples and full documentation of the images and other aspects of the samples that the lab personnel have based their interpretations on. The preparation and cutting of the samples by the lab technicians shall be video recorded.

C6 Chloride Profile

Chloride ion content testing will be performed on selected core samples to determine whether chloride contamination from salts has accumulated to sufficient levels to promote corrosion of reinforcing steel. Selected core samples will be cut into slices at pre-determined depths, and the slices will be pulverized to facilitate chloride

content analysis in general conformance with ASTM C1152 *Test Method for Acid-Soluble Chloride in Mortar and Concrete* and ASTM C1218 *Standard Test Method for Water-Soluble Chloride in Mortar and Concrete*. The acid-soluble chloride content represents both water-soluble and chemically-bound chloride within the cement paste and any chloride that may be present in the aggregate. The water-soluble chloride content represents only the chlorides in the cement paste and aggregates that are soluble in water. Slices will be taken at depth prescribed by ASTM C1556 *Standard Test Method for Determining the Apparent Chloride Diffusion Coefficient of Cementitious Mixtures by Bulk Diffusion*, Table 1. Table 1 in C1556 describes the depth at which slices should be taken given a known water to cement ratio. Thus, the water to cement ratio should be determined from petrographic analysis and then used to determine the number of slices to be analyzed chloride and other ionic species.

C7 Ionic Species Profile

The same powder used for analysis of chloride content by ASTM C1218 and ASTM C1152 can be used to determine the content of sulfur, magnesium, sodium, and potassium found in the concrete by utilizing testing methods described in ASTM C114 *Standard Test Methods for Chemical Analysis of Hydraulic Cement*. In fact, C1152 and C1218 rely on the method for chloride analysis in C114. As these elements are part of the ocean salts or cement paste, changes in depth and concrete location can be useful in pinpointing deterioration mechanisms.

REINFORCING STEEL TESTING

Reinforcing Steel Sample Removal

All reinforcing steel samples shall be extracted such that the samples are straight and undamaged by chipping hammers or other removal equipment. Deep saw cutting along and perpendicular to the bar is envisioned. Samples shall be at least 36 inches long. Each extracted sample shall be measured and photographed. Repair of sample locations may be necessary to prevent water intrusion.

R1 Reinforcing Bar Location Survey Using Ground Penetrating Radar (GPR) – Non-Destructive

To determine location (depth and spacing) and concrete cover of the reinforcing steel, ground penetrating radar (GPR) will be used. In addition to the non-destructive testing, limited destructive measures (e.g., drilling small ~1/2 in. diameter holes) will be used for correlation and calibration. GPR involves the use of a high-frequency radar antenna, which transmits electromagnetic radar pulses along with a discrete longitudinal scan at the surface of a structural element. Electromagnetic signals reflected from material interfaces having different dielectric properties (i.e., reinforcing steel embedded in concrete) are collected by the antennae and displayed graphically for direct interpretation in the field. Guidelines for GPR considered during this work included ACI 228.2R-13 *Report on Nondestructive Test Methods for Evaluation of Concrete in Structures* and ASTM D6432 *Standard Guide for Using the Surface Ground Penetrating Radar Method for Subsurface Investigation*.

R2 Tensile Properties

Tensile properties of reinforcing bar specimens include yield strength, ultimate tensile strength, elongation, and elastic modulus. Tensile properties will be determined by testing as-rolled bar specimens in a load frame. Testing of tensile property shall be carried out in accordance with the applicable portions of ASTM A615 *Standard Specification for Deformed and Plain Carbon-Steel Bars for Concrete Reinforcement* and ASTM A370 *Standard Test Methods and Definitions for Mechanical Testing of Steel Products*.

R3 Rockwell Hardness

The Rockwell hardness test is an indentation hardness test that involves the use of a verified machine to force a diamond spheroconical indenter or tungsten carbide (or steel) ball indenter into the surface of a material. Hardness is determined based on the size of the indentation. Rockwell hardness testing will be carried out in accordance with ASTM E18 *Standard Test Methods for Rockwell Hardness of Metallic Materials*.

R4 Chemistry

A spectrometric analysis will be used to determine the quantity of carbon, iron, and residual elements in carbon steel (copper, nickel, arsenic, lead, tin, chromium, etc.) for the determination of compliance with compositional specifications (in this case, ASTM A615). Testing will be carried out in accordance with ASTM E415 *Standard Test Method for Analysis of Carbon and Low-Alloy Steel by Spark Atomic Emission Spectrometry*.

R5 Section Loss/Pitting

The extent of section loss and pitting on samples (particularly those tested in R2) will be determined according to ASTM G46 *Standard Guide for Examination and Evaluation of Pitting Corrosion* and ACI 364.14T-17 *Section Loss Determination of Damaged or Corroded Reinforcing Steel Bars*.

GEOTECHNICAL INVESTIGATION

G1 Soil/Rock Borings

The following geotechnical investigation is proposed:

- Forty (40) borings with approximate locations as annotated in the attached Appendix B are proposed. Borings SPT-8, SPT-9, SPT-10 and SPT 14, which lie outside the perimeter of the Champlain tower walls to the south, will be performed with a smaller rig that fits in the narrow passageway.
- All borings are generally expected to extend to a rock layer at about 80 feet deep unless determined otherwise by the Participants.
- Boreholes for sampling shall be kept full of drilling fluid at all times. If needed, bentonite or suitable alternative should be added to keep the borehole full of drilling fluid and avoid borehole squeeze.
- The driller will open the SPT sampler and allow Participants to visually inspect the contents. Samples from the SPT sampler will be collected at various depths within the boreholes as directed by the Consultant and sealed in watertight containers. The drilling contractor will perform SPT testing and obtain samples at a maximum interval of every 5 feet, or as otherwise directed by the Participants. SPT samples will be collected continuously as directed by the Participants.
- Samples of the interbedded limestone/sand stratum will be obtained using Pitcher samplers in addition to split-spoon samples during the initiation of the subsurface investigation. The Consultant, in agreement with the Participants, shall evaluate and determine the need for subsequent Pitcher sampling as the subsurface investigation continues; however, pitcher sampling shall not be eliminated and shall be incorporated as a supplement in select borings to provide a comparison between split-spoon samples and pitcher samples. An alternative to the Pitcher sampler may be the use of a triple-tube core barrel to increase sample recovery and quality.
- The drilling subcontractor shall use methods and procedures to maximize core recovery. Coring shall be in accordance with FDOT standards. In addition, coring should be continuous, whether 5-ft or 10-ft core runs are used.
- Samples of soil and rock cores will be retrieved and shipped for further laboratory testing at a mutually agreed upon geotechnical testing laboratory. The Consultant will develop a protocol for adequate sample preservation and shipping.
- All Participants will be allowed to visually observe the joint investigation. Participants will not direct the drilling contractor for the forty (40) joint borings. All soil and rock samples will be allowed to be visually evaluated by all Participants on-site during the investigation.

The forty (40) boring locations are to be drilled by the same mutually agreed upon drilling contractor who will work with the Consultant to observe, log samples, and provide investigation reports. The drilling contractor shall provide sample boring and drilling logs and list of items to be included in the visual description for review by Participants. The Consultant will provide a sample boring log also for review by Participants. The Consultant will provide reports including, but not limited to, visual-manual soil identification per ASTM D2487 and D2488, SPT blow counts, RQD

and water level observations. The same contractor shall remain on site after the completion of the 40 borings so each Participant can perform additional tests, if so requested, for an agreed upon duration of time. The testing performed by each Participant after the first 40 borings can be visually observed by all Participants; however, the samples extracted will belong to the specific Participant coordinating the testing and can be destructively tested as needed.

In the standard penetration test (SPT) procedure, a 2-inch outer diameter split-barrel sampler is driven into the soil by repeatedly dropping a 140-pound hammer from a height of 30 inches. The length of the samplers can be 18 or 24 inches. The number of blows required to advance the sampler every 6 inches are recorded. The sum of the second and third blows is recorded as the SPT N values for that depth. SPT N values are utilized to correlate strength parameters for the soils. Accompanying energy measurements will be reported. Soil and rock borings shall be carried out in accordance with ASTM D1586 *Standard Test Method for Standard Penetration Test (SPT) and Split-Barrel Sampling of Soils*.

- Soil borings will be advanced using the standard penetration test (SPT) procedure described above unless otherwise directed by the Consultant's geologist or engineer. The test provides samples for identification purposes and provides a measure of penetration resistance which can be used for geotechnical design purposes. The standard penetration test recovers a highly disturbed sample, which is generally not suitable for tests that measure properties of the in-situ soil structure, such as density, strength, and consolidation characteristics. The samples will be labeled, sealed, and transported to a laboratory for further index and classification testing. If fine-grained material is encountered, thin-walled tube samples should be used for obtaining intact specimens of fine-grained soils for laboratory tests to determine engineering properties. Similarly, if soils containing cohesive materials (clays) are encountered, thin-walled tube sampling should be performed to obtain minimally-disturbed samples. Drained and undrained triaxial testing will be performed on any samples of clay to establish strength parameters.
- The Consultant should provide draft investigation logs every week or sooner of all work completed the prior week.

G2 Observation Wells and Water Quality Samples

A standpipe will be installed after the completion of specified boreholes. Observation wells should be screened in separate strata. Certain observation wells should be screened in the sand stratum above the interbedded limestone/sand, and others should be screened in the interbedded limestone/sand stratum itself. These standpipes will be monitored for groundwater levels in the subsequent days of the investigation. An electronic transducer connected to a datalogger shall be installed in each well. The groundwater should be monitored at least six times daily until it is determined by Participants that it can be stopped, to develop a record of seasonal and other variations. A barometer shall be installed at the site. Sufficient dataloggers shall be installed at the site to read all installed sensors with provisions for the data to be provided to the Participants at regular intervals. Water quality samples will be collected and sent to an approved laboratory for testing. Field permeability testing in the sand and the interbedded limestone/sand strata in general shall be performed accordance with ASTM D5912. The Participants shall identify the location of each field permeability test. The Participants shall fully develop the requirements and details for the observation wells after the Consultant obtains subsurface information from the borings.

Monitoring wells will be installed at the locations shown on Appendix B and any other location identified by the Participants to determine long-term, stabilized groundwater readings. A slotted pipe will be installed within the borehole and packed with sand on the outside to ensure the slots in the pipe are not clogged. A bentonite seal shall be established above the top of the screened zone. In addition, grout will be placed near the top of the pipe to ensure surface water does not percolate into the observation well. The top of the pipe will be capped and sealed to ensure precipitation does not enter the observation well. Observation wells will be installed and monitored in accordance with ASTM D5092 *Standard Practice for Design and Installation of Groundwater Monitoring Wells*.

Samples at 10-ft intervals within the borehole shall be obtained for the purposes of water quality sample testing. Samples shall be obtained in accordance with the FDOT Soils and Foundation Handbook for investigations for Structures and shall be tested in accordance with Section 4.10 – Environmental Corrosion Test Guidelines.

G3 Test Pit Excavation

Two (2) test pit excavations (TP-5 and TP-17) and full-length cores at the piles identified as TP-5 and TP-17 on Appendix B shall be performed prior to commencement of any borings in order to establish the lengths of the borings. The remainder of the test pits shall be performed after the borings are completed.

The structural slab and remaining column shall be extracted labeled and set aside for inspection and additional sampling if necessary. Care should be taken to remove the column and structural slab as to minimize damage.

Test pits shall be sufficiently deep to expose the pile caps and the top 2 to 3 feet of the pile. At each test pit, core drill or saw cut through the pile cap and portion of the pile to extract samples and permit pile integrity testing. Test pits shall be dewatered as required to permit observation and pile integrity testing. Contractor shall cut through concrete, remove materials and provide needed dewatering under the direction of the Consultant. At the basement wall, test pits shall be sufficiently deep to expose or extend to the elevation of the bottom of the foundation wall.

Walkway test pits shall be sufficiently deep to expose or extend to the elevation of the bottom of the foundation wall. Test pits shall be dewatered as required to permit observation. Contractor shall cut through concrete, remove materials and provide needed dewatering under the direction of the designated engineer/geologist.

The Consultant will engineer each test pit excavation for safe access. The test pit designs will be shared with all parties. OSHA standards will be complied with in relation to design, execution and access.

G4 Pile Integrity Testing (PIT)

PIT Testing is performed by placing accelerometers at several locations on the pile and then hitting the pile or as necessary to obtain conclusive and accurate test results, with different size hammers and then measuring the response of the pile to the impact. The results are used to estimate the length of the pile. For higher quality of results, it is recommended to access the pile at the top and the side. Pile integrity testing will be performed in accordance with ASTM D5882 *Standard Test Method for Low Strain Impact Integrity Testing of Deep Foundations*.

PIT will be performed by an agreed upon subcontractor. The PIT subcontractor will work under the direction of the Consultant and, subsequent to the testing, will provide raw data and interpretive reports to all Participants. Depending on the data available for the design and as-built length of piles, PIT must be performed a few days prior to the geotechnical boring investigation for all Phase 1 test pits to determine the necessary depth of borings. Sufficient time will be provided to allow interpretation of PIT data and if inconclusive results are obtained, alternative testing techniques should be implemented. Refer to G19 Parallel Seismic logging.

G5 Crosshole Seismic Testing

Crosshole seismic testing will be performed between boreholes as directed during the field investigation as identified in Appendix B. Crosshole Seismic Testing will measure compression and shear waves between two adjacent boreholes at various depths according to ASTM D4428. The Consultant will determine the final holes to be tested based on findings of the borehole sampling and initial test results. Additional borings shall be made if necessary to satisfy the requirements of ASTM D4428. A crosshole testing plan will be provided by the consultant prior to execution.

G6 through G17, Laboratory Testing

Select samples from the field investigation of all joint borings and the test pits will be tested in a commonly agreed upon laboratory proposed by the Consultant. Specific tests and testing locations will be agreed upon on a rolling basis by all Participants as the field work is completed, and drilling logs have been provided. Reasonable efforts will

be made depending on available sample quantities to provide every requested test. Participants may request to use samples that were not selected for laboratory testing as part of this Protocol, for laboratory testing at their own expenses. The Consultant shall provide weekly drilling/investigation logs and testing will be agreed upon and ordered every week based on the field observations and the drill logs.

G6 Grain Size Analysis

Soils consist of particles of various shapes and sizes. The particle-size distribution (gradation) of soil can be determined from representative specimen following ASTM D6913 *Standard Test Methods for Particle-Size Distribution (Gradation) of Soils Using Sieve Analysis*. In cases where the gradation of particles smaller than No. 200 (75- μ m) sieve is needed, *Test Method D7928* may be used. All samples subjected to grain size evaluation with more than 10% fines should include hydrometer testing such that the D10 of the material can be determined for estimation of permeability.

G7 Moisture content

The moisture content of soil and rock can be determined in the laboratory by drying the material and measuring the reduction in mass of the material. The water content is the ratio of the mass of water contained in the pore spaces of soil or rock material to the solid mass of particles expressed as a percentage. The test is performed according to ASTM D2216 *Standard Test Methods for Laboratory Determination of Water (Moisture) Content of Soil and Rock by Mass*.

G8 Organic Content

Percentage of organic matter is important for classifying or evaluating peat or other organic soil, and for geotechnical and general soil classification purposes. This test can be used to determine the moisture content, ash content, and percent organic matter in the soil. The test is performed according to ASTM D2974 *Standard Test Methods for Moisture, Ash, and Organic Matter of Peat and Other Organic Soils*.

G9 Incremental Consolidation

The magnitude and rate of consolidation of fine-grained soil can be determined when it is restrained laterally and drained axially while subjected to incrementally applied controlled-stress loading. The test is performed on undisturbed soil samples in accordance with ASTM D2435 *Standard Test Methods for One-Dimensional Consolidation Properties of Soils Using Incremental Loading*. Loading increments specified for the lab will need to be agreed upon by all Participants.

At the request of any Participant, testing will be performed on reconstituted samples of sand to obtain stiffness parameters.

G10 Electrical Resistivity

Electrical resistivity for representative samples will be determined in situ and in the laboratory. Measurement of soil resistivity is used for the control of corrosion of buried structures and for the estimation of expected corrosion rates. The test will be performed according to ASTM G57 *Standard Test Method for Field Measurement of Soil Resistivity Using the Wenner Four-Electrode Method*.

G11 Chlorides, G12 pH, G13 Sulfates, G14 Sulfides

This group of tests is performed in a laboratory setting to determine chloride ion, sulfate ion, sulfide content, and pH level in the representative samples. The principal use of the pH test is to supplement soil resistivity measurements and thereby identify conditions under which the corrosion of metals in soil may be accentuated. The individual tests will be performed according to ASTM D512 *Standard Test Methods for Chloride Ion in Water*, ASTM D516 *Standard Test Method for Sulfate Ion in Water*, SM 4500 *Sulfide by Standard Methods Committee*, and G51 *Standard Test*

Method for Measuring pH of Soil for Use in Corrosion Testing. Water samples from bailed observation wells for groundwater chemistry testing shall be obtained.

G15 Unconfined Compression of Rock

The strength of intact rock core specimens in uniaxial compression will be determined following ASTM D7012C *Standard Test Methods for Compressive Strength and Elastic Moduli of Intact Rock Core Specimens under Varying States of Stress and Temperatures*. In the laboratory, a rock core specimen is cut to length and the ends are machined flat if possible and if not possible, as otherwise determined by the Participants. The specimen is placed in a loading frame, where the axial load on the specimen is increased and measured continuously until the sample fails. The uniaxial compressive strength is calculated as the maximum load at failure divided by the cross-sectional area of the specimen. Adequate instrumentation shall be included to determine Young's modulus.

G16 Unit weight of Rock Cores

Unit weight of rock cores will be determined in general accordance with ASTM D7263 *Standard Test Methods for Laboratory Determination of Density and Unit Weight of Soil Specimens*, method B (direct measurement). For a rock core specimen of cylindrical shape, the dimensions and mass of a specimen are measured. The density and unit weight are then calculated using these direct measurements.

G17 Specific Gravity

Specific gravity of soil samples will be determined in general accordance with ASTM D854, *Standard Test Methods for Specific Gravity of Soil Solids by Water Pycnometer*. These test methods cover the determination of the specific gravity of soil solids passing a sieve using a water pycnometer. These test methods are not viable for solids that can be altered by these methods, are contaminated with a substance that prohibits the use of these methods, or are highly organic (such as fibrous matter which floats in water).

G18 Seismic Cone Penetration Testing (CPT)

Seismic cone penetration testing estimates the small strain shear modulus of the soil by measuring shear wave velocities in the ground. The seismic CPT testing is performed as part of CPTU testing. The CPTU test will additionally measure continuous point and frictional resistance during penetration of the cone, pore pressures, and dissipation of pore pressures with time at constant depths. This testing will be carried out in general accordance with ASTM D5778 *Standard Test Method for Electronic Friction Cone and Piezocone Penetration Testing of Soils*.

The following geotechnical investigation is proposed:

- Seismic CPTs capped at 400-500 tsf with approximate locations as annotated in the attached Appendix B.
- CPTs will be pushed until refusal.
- Continuous measurements of point resistance, frictional resistance, pore pressure and shear wave velocity will be performed while the cone is pushed at a constant rate of 2 cm/sec. Three dissipation tests for each CPT location will be performed to establish groundwater and dissipation properties at those depths as directed by the Consultant.
- No sample recovery is performed with this testing.

The Seismic CPTs will be performed by the same mutually agreed upon contractor who will work with Consultant to observe and provide investigation reports. The Seismic CPT contractor will provide both raw data and reports including, but not limited to, cone tip resistance, frictional resistance, pore pressure measurement with depth, and pore pressure measurements with time during dissipation testing. Permeability values determined from pore pressure dissipation tests will be included.

G19 Parallel Seismic Logging

Parallel Seismic Logging (PSL) will be performed at indicated locations as annotated in the attached Appendix B. Boreholes will have casing and grout installed for the PSL testing that will be performed subsequent to the drilling activities. This testing involves sending a transceiver down the cased borehole to measure the depth of the buried sheet pile wall and will be carried out in general accordance with ASTM D8381 *Standard Test Methods for Measuring the Depth of Deep Foundations by Parallel Seismic Logging*.

Parallel seismic logging shall be accomplished as a supplemental means of determination of foundation lengths in the event that PIT interpretations are deemed inconclusive.

G20 Drained and Undrained Triaxial Testing of Fine-Grained Soil Samples

If fine-grained material is encountered and if thin-walled tube samples are used for obtaining intact specimens of fine-grained soils for laboratory tests to determine engineering properties, then triaxial testing will be performed on the specimens. This testing will be carried out in general accordance with ASTM D7181 *Standard Method for Consolidated Drained Triaxial Compression Test for Soils*, ASTM D4767 *Standard Test Method for Consolidated Undrained Triaxial Compression Test for Cohesive Soils*, and/or ASTM D2850 *Standard Test Method for Unconsolidated-Undrained Triaxial Compression Test on Cohesive Soils*.

At the request of any Participant, testing will be performed on reconstituted samples of sand to obtain shear strength parameters.

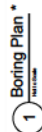
The Participants shall endeavor, but are not required, to agree on the Protocol of testing G20 including initial stress state before shearing, stress levels, strain rate, and drainage conditions during testing.

G21 Seismograph

Vibration monitoring shall be accomplished via a calibrated seismograph throughout the duration of all work accomplished on site at various locations, including these shown on Appendix B, with the monitors installed per location at different distances. The vibration monitoring shall start one week prior to the beginning of testing under the Protocol to establish a baseline at a trigger level of 0.05 in/s or lower, as required to record ambient/background levels; and shall continue 24- hours per day and 7-days per week for the duration of the work under the Protocol.

G22 MASW/ER

MASW and ER geophysical testing shall be accomplished along the entire length of the perimeter wall and on the extent of the basement slab.

Project
Champlain Towers South - Joint Testing Protocol

Project
Champlain Towers South - Joint Testing
8777 Collins Ave. Surfside, Florida 33154

Project
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Sheet Title

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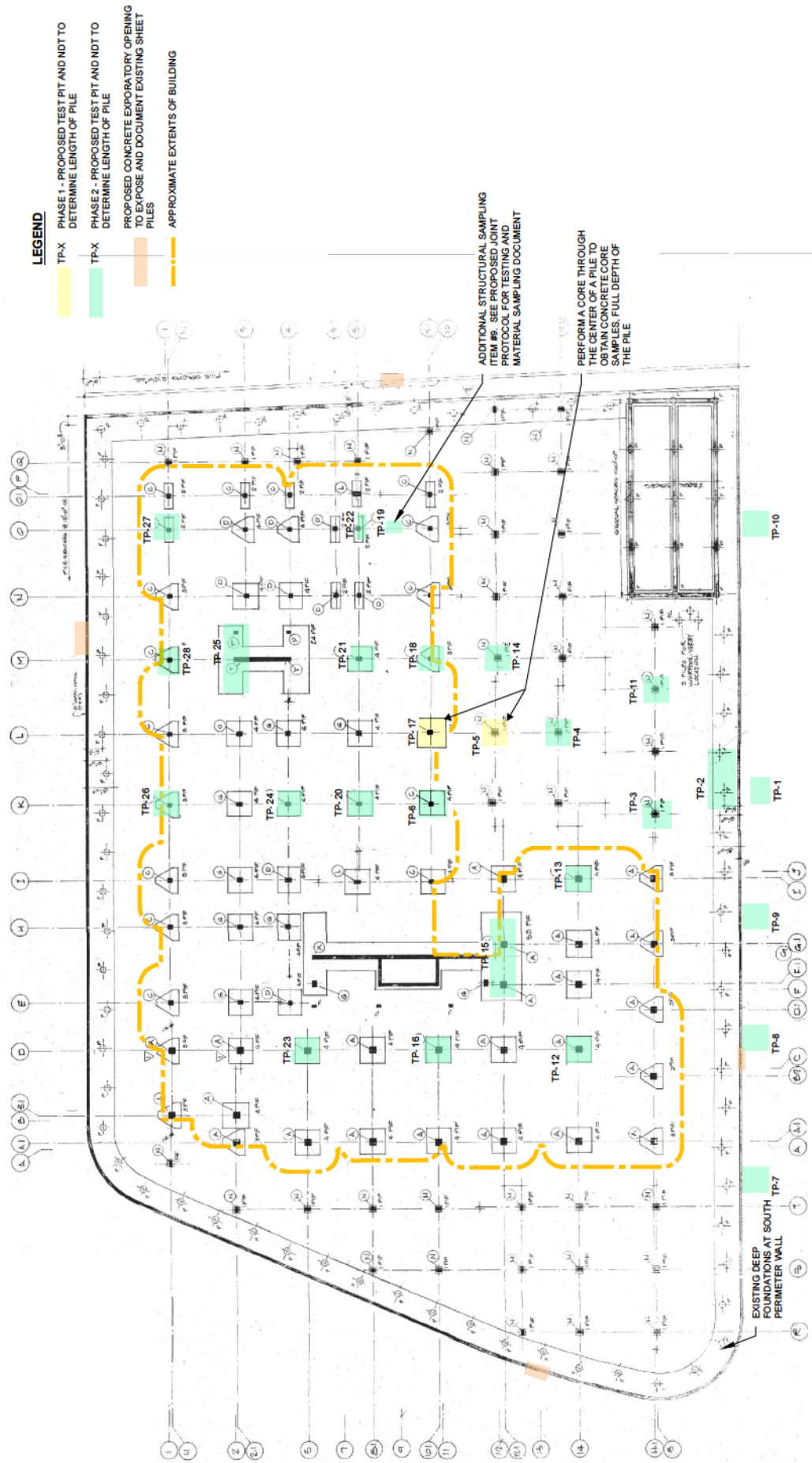
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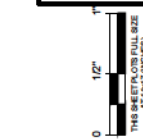
* BASE PLAN FROM DRAWINGS TITLED "CHAMPLAIN TOWERS SOUTH", PREPARED BY BREITERMAN JURADO & ASSOCIATES CONSULTING ENGINEERS, DATED 8/27/79

Appendix B Champlain Towers South Investigation - Joint Protocol for Testing and Material Sampling (Collapse Site)

5



1 Test Pits *



Revised: January 13, 2022

Proj. No.	2021.4155
Date	December 10, 2021
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Scale	As Noted

Project
Champlain Towers South - Joint Testing Protocol
8777 Colina Ave., Sunrise, Florida 33154

Sheet Title

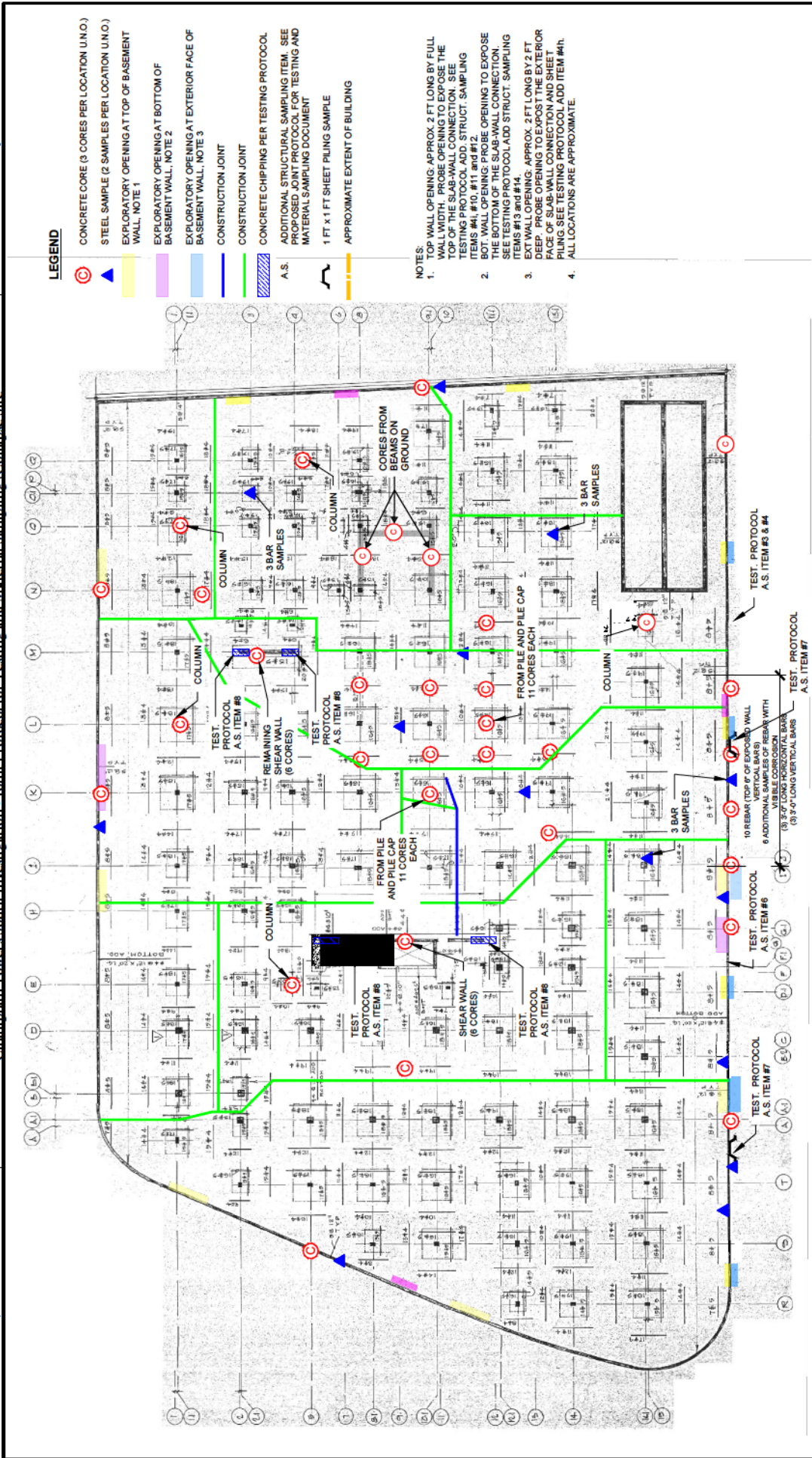
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SK-02

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Appendix B
Champlain Towers South Investigation - Joint Protocol for Testing and Material Sampling (Collapse Site)



1 Material Samples and Exploratory Opening Plan

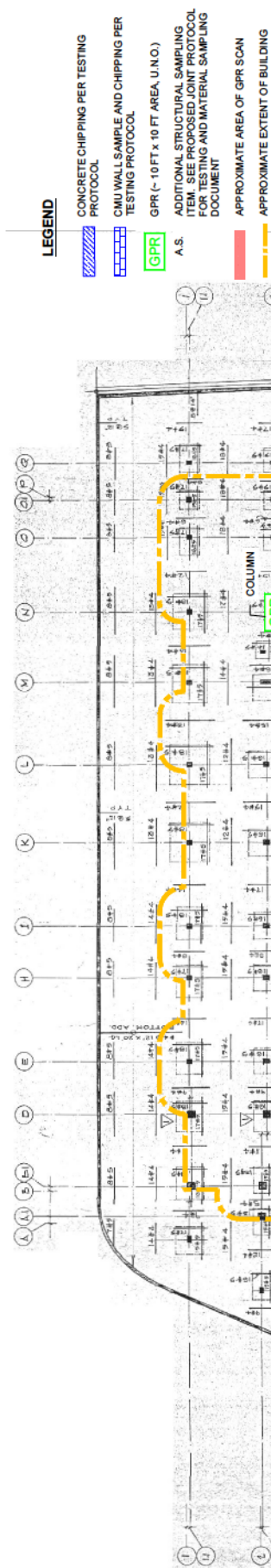
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Sheet Title	Material Samples and Exploratory Openings
Project No.	2021.4155
Date	December 10, 2021
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Scale	As Noted

SK-03

* BASE PLAN FROM DRAWINGS TITLED "CHAMPLAIN TOWERS SOUTH" PREPARED BY BREITHEIMAN JURADO & ASSOCIATES CONSULTING ENGINEERS, DATED 9/27/19

Appendix B Champlain Towers South Investigation - Joint Protocol for Testing and Material Sampling (Collapse Site)

5



1 Basement Plan

Revised: January 13, 2022

Project	Champlain Towers South - Joint Testing Protocol	Proj. No.	2021.4155
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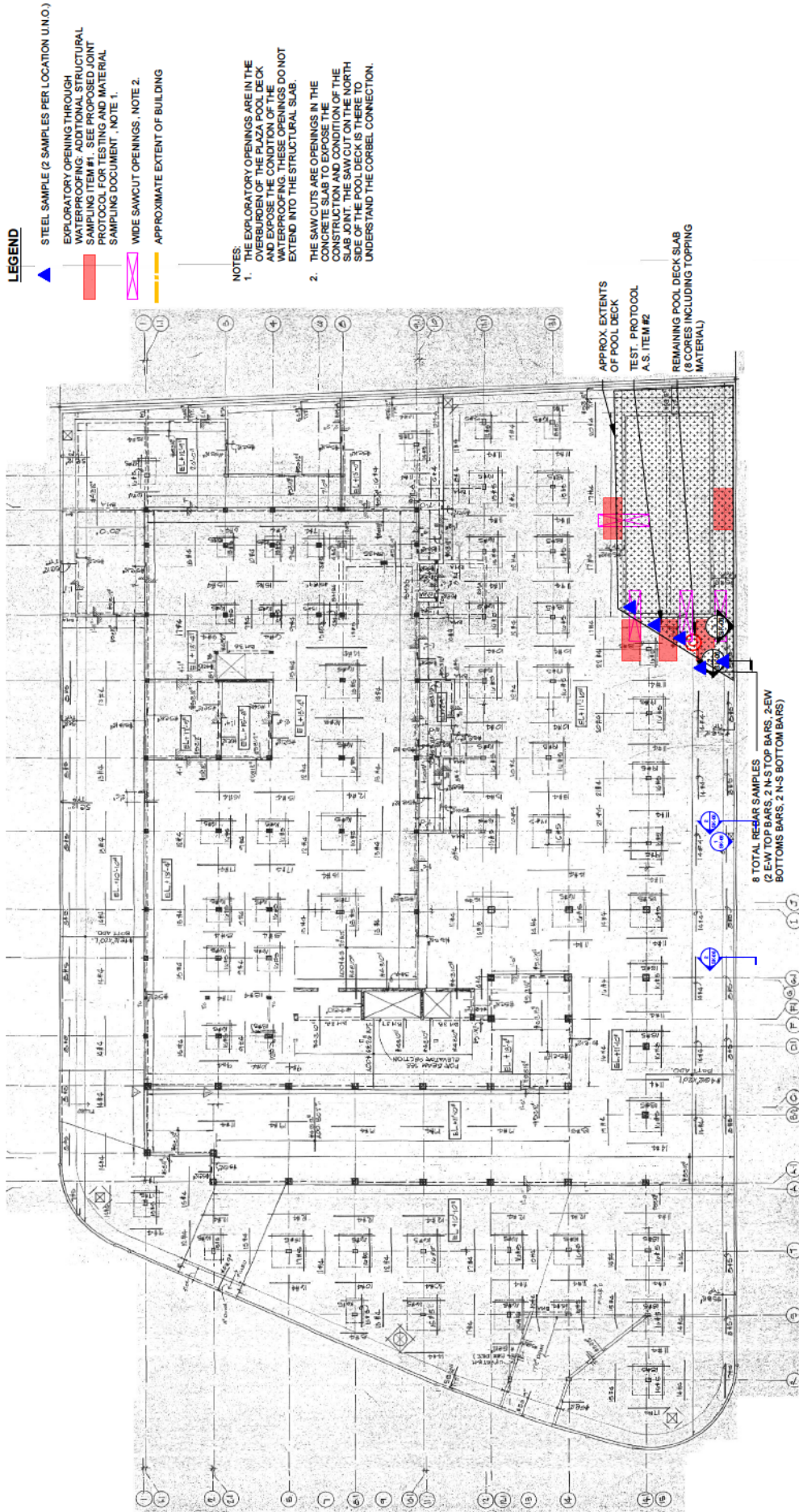
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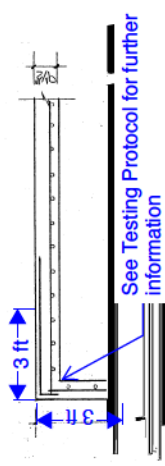
Appendix B Champlain Towers South Investigation - Joint Protocol for Testing and Material Sampling (Collapse Site)



Project Champlain Towers South - Joint Testing Protocol 8777 Colina Ave., Sunrise, Florida 33154		Proj. No. 2021.4155 Date December 10, 2021 Drawn SKA Checked MFF Scale As Noted
Engineers WJE ARCHITECTS MATERIALS SCIENTISTS Wiss, Janney, Elstner Associates, Inc. 330 Princeton Road Northbrook, Illinois 60062 Tel: 847.277.2000 Fax: 847.490.0304 www.wje.com		SK-05 Sheet No.
Title Pool Deck Level		Revised January 13, 2022

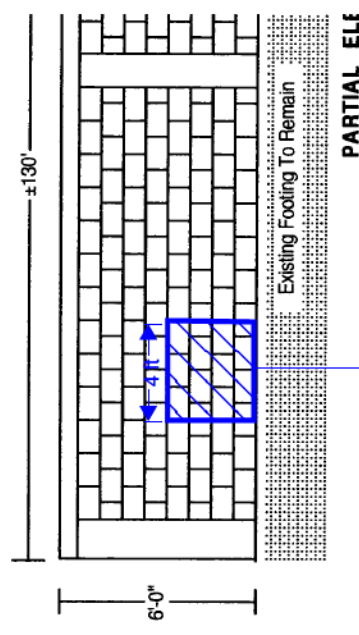
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TESTING PROTOCOL ADDITIONAL
STRUCTURAL ITEM #4

NOTE:
AT GRIDLINE K, REMOVAL CONSISTS OF THE
PRIVACY WALL, FOUNDATION WALL, AND
RESIDUAL SLAB.
AT GRIDLINE H, REMOVAL CONSISTS OF THE
FOUNDATION WALL AND RESIDUAL SLAB.



TESTING PROTOCOL ADDITIONAL
STRUCTURAL ITEM #3

See Testing Protocol for further information

Project Champlain Towers South - Joint Testing Protocol 8777 Collins Ave., Suite 3154		Proj. No. 2021.4155
Sheet Title Additional Items		Date December 10, 2021
WJE ENGINEERS ARCHITECTS MATHEMATICAL SCIENTISTS Wiss, Janney, Elstner Associates, Inc. 330 Piquette Road Naperville, IL 60563 630.272.7400 (N) 847.490.0304 (F) www.wje.com		Drawn SKA
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* BASE PLAN FROM UNDATED DRAWING SHEET NO. S1 TITLED CHAMPLAIN TOWERS SOUTH
PREPARED BY BREITERMAN JURADO & ASSOCIATES CONSULTING ENGINEERS
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Exhibit B

**IN THE CIRCUIT COURT OF THE
ELEVENTH JUDICIAL CIRCUIT
IN AND FOR MIAMI-DADE COUNTY**

**COMPLEX BUSINESS
LITIGATION DIVISION**

**IN RE: CHAMPLAIN TOWERS SOUTH
COLLAPSE LITIGATION.**

CLASS REPRESENTATION

CASE NO. 2021-015089-CA-01

**ORDER REGARDING
JOINT PROTOCOL FOR TESTING AND MATERIAL SAMPLING – COLLAPSE SITE**

THIS CAUSE came before the Court upon a series of conferences and hearings, including the December 22, 2021 evidentiary hearing, the Court’s December 30, 2021 *sua sponte* Order, the Court’s January 3, 2022 Order, and hearings on January 14, 2022 and January 21, 2022. The Court having considered the issues and positions of the parties, it is **ORDERED AND ADJUDGED** as follows:

1. The parties and their respective experts, consultants, and subcontractors shall promptly mobilize and begin invasive testing at the collapse site at the beginning of February. The invasive testing shall be performed pursuant to the agreed upon Champlain Towers South Collapse Investigation: Joint Protocol for Testing and Material Sampling – Collapse Site (the “Joint Testing Protocol”), which was submitted to the Court on January 21, 2022.

2. Unless otherwise provided in the Joint Testing Protocol, the testing performed on site will be coordinated and overseen by the independent Consultant, Geosyntec Consultants, Inc. (“Consultant”), which was chosen by the Receiver with the consent of the Parties. The Consultant, subject to review by all parties, will employ testing agencies and contractors required to carry out the goals of the Joint Testing Protocol.

3. The Joint Testing Protocol shall allow all parties adequate opportunity to collect and test evidence so as to allow the parties to properly investigate the cause of this collapse as well as the claims advanced by Plaintiffs and any potential defenses.

4. The Parties shall keep the Court informed as to the scheduling and progress of the testing being performed pursuant to the Joint Testing Protocol.

5. Except for the costs attributable to additional or specialized testing, which costs are the sole responsibility of the Participant seeking such additional or specialized testing, each Participant's or other third-party's allocation of the costs incurred through performance of this Protocol shall be determined pursuant to an order from the Court to be issued subsequently.

DONE and **ORDERED** in Chambers at Miami-Dade County, Florida, on this _____ day of January, 2022.

**IN THE CIRCUIT COURT OF THE ELEVENTH JUDICIAL
CIRCUIT IN AND FOR MIAMI-DADE COUNTY, FLORIDA**

CASE NO: 2021-015089-CA-01

SECTION: CA43

JUDGE: Michael Hanzman

In Re: Champlain Towers South Collapse Litigation

Plaintiff(s)

vs.

N/A

Defendant(s)

_____ /

**ORDER REGARDING JOINT PROTOCOL FOR TESTING AND MATERIAL
SAMPLING – COLLAPSE SITE**

THIS CAUSE came before the Court upon a series of conferences and hearings, including the December 22, 2021 evidentiary hearing, the Court’s December 30, 2021 *sua sponte* Order, the Court’s January 3, 2022 Order, and hearings on January 14, 2022 and January 21, 2022. The Court having considered the issues and positions of the parties, it is **ORDERED AND ADJUDGED** as follows:

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3. The Joint Testing Protocol shall allow all parties adequate opportunity to collect and test evidence so as to allow the parties to properly investigate the cause of this collapse as well as the claims advanced by Plaintiffs and any potential defenses.
4. The Parties shall keep the Court informed as to the scheduling and progress of the testing being performed pursuant to the Joint Testing Protocol.
5. Except for the costs attributable to additional or specialized testing, which costs are the sole responsibility of the Participant seeking such additional or specialized testing, each Participant's or other third-party's allocation of the costs incurred through performance of this Protocol shall be determined pursuant to an order from the Court to be issued subsequently.

DONE and ORDERED in Chambers at Miami-Dade County, Florida on this 21st day of January, 2022.



2021-015089-CA-01 01-21-2022 1:47 PM

Hon. Michael Hanzman

CIRCUIT COURT JUDGE

Electronically Signed

No Further Judicial Action Required on **THIS MOTION**

CLERK TO **RECLOSE** CASE IF POST JUDGMENT

Electronically Served:

Aaron Podhurst, apodhurst@podhurst.com

Aaron Podhurst, dricker@podhurst.com

Adam A Schwartzbaum, adams@moskowitz-law.com

Adam A Schwartzbaum, service@moskowitz-law.com
Adam A Schwartzbaum, dione@moskowitz-law.com
Adam Moskowitz, adam@moskowitz-law.com
Adam Moskowitz, dione@moskowitz-law.com
Adam Moskowitz, service@moskowitz-law.com
Alfred Armas, alfred@armaslaw.com
Alison E Patino, apatino@patinolaw.com
Amanda Anderson, AAnderson@insurance-counsel.com
Amanda K Anderson, eservice@insurance-counsel.com
Amanda K Anderson, aanderson@insurance-counsel.com
Amanda K Anderson, InsuranceCounsel2050@gmail.com
Andrew B. Yaffa, aby@grossmanroth.com
Andrew B. Yaffa, omb@grossmanroth.com
Andrew M Feldman, feldmana@kleinpark.com
Andrew M Feldman, montanem@kleinpark.com
Andrew M Feldman, piersonj@kleinpark.com
Andrew P. Gold, Esq., andrew.gold@akerman.com
Andrew P. Gold, Esq., jill.parnes@akerman.com
Andrew Paul Gold, andrew.gold@akerman.com
Andrew Paul Gold, jill.parnes@akerman.com
Anthony J Carriuolo, acarriuolo@bergersingerman.com
Anthony J Carriuolo, mnewland@bergersingerman.com
Anthony J Carriuolo, drt@bergersingerman.com
Anthony Perez, aperez@hsmmpa.com
Aron Raskas, araskas@gunster.com
Aron Raskas, avalido@gunster.com
Aron Raskas, eservice@gunster.com
Benjamin Fernandez, IV, bfernandez@silvasilva.com
Benjamin Fernandez, IV, tgomez@silvasilva.com
Bernadette Vazquez, bvazquez@klugerkaplan.com
Bernadette Vazquez, clong@klugerkaplan.com
Bradford R. Sohn, brad@bradsohnlaw.com
Bradford R. Sohn, paralegal@bradsohnlaw.com
Bradford R. Sohn, service.bradsohnlaw@gmail.com
Bradley J Edwards, staff.efile@epllc.com
Bradley J Edwards, brad@epllc.com
Bradley J Edwards, maria@epllc.com
Brenda Radmacher, brenda.radmacher@akerman.com

Bret M Feldman, feldmanb@phelps.com
Bret M Feldman, yolanda.vazquez@phelps.com
Brian S Dervishi, bdervishi@wdpalaw.com
Brian S Dervishi, service@wdpalaw.com
Bruce Alan Katzen, bkatzen@klugerkaplan.com
Bruce Alan Katzen, cfalla@klugerkaplan.com
Bruce Alan Katzen, probate@klugerkaplan.com
Carlos A. Velasquez, CVelasquez@VDLawyers.com
Carlos A. Velasquez, Andrea@VDLawyers.com
Carlos E Silva, csilva@silvasilva.com
Carlos E Silva, mromera@silvasilva.com
Carlos M. Macias, macias@leesfield.com
Carlos M. Macias, becerra@leesfield.com
Carlos M. Macias, abreu@leesfield.com
Caroline Catchpole Spradlin, caroline.spradlin@phelps.com
Caroline Catchpole Spradlin, samantha.powell@phelps.com
Carolyn M. Luna, cluna@patinolaw.com
Christine L. Welstead, christine.welstead@bowmanandbrooke.com
Christine L. Welstead, ashleigh.carroll@bowmanandbrooke.com
Christine L. Welstead, lisa.morales@bowmanandbrooke.com
Christopher L Barnett, barnettch@gtlaw.com
Christopher L Barnett, nicauda@gtlaw.com
Christopher S Carver, christopher.carver@akerman.com
Christopher S Carver, cary.gonzalez@akerman.com
Cole Scott &Kissane PA, Ryan.Charlson@csklegal.com
Cole Scott &Kissane PA, Ryan.Sooahoo@csklegal.com
Cole Scott &Kissane PA, Construction.FTLW@csklegal.com
Cosme Caballero, ccaballero@deutschblumberg.com
Cosme Caballero, kdominguez@deutschblumberg.com
Cosme Caballero, rvargas@deutschblumberg.com
Cosme Caballero, Esq., ccaballero@deutschblumberg.com
Cosme Caballero, Esq., bblumberg@deutschblumberg.com
Curtis Miner, curt@colson.com
Curtis Miner, claudiav@colson.com
Curtis Miner, eservice@colson.com
Daniel Y Gielchinsky, dan@dgimlaw.com
Daniel Y Gielchinsky, tatiana@dgimlaw.com
David B. Weinstein, weinsteind@gtlaw.com

David B. Weinstein, thomasm@gtlaw.com
David L Rosinsky Esq., LUKSFLL-Pleadings@LS-LAW.COM
David L Rosinsky Esq., Drosinsky@insurancedefense.net
David L Rosinsky Esq., drosenbaum@insurancedefense.net
David M Wells, dwells@gunster.com
David M Wells, dculmer@gunster.com
David M. Murray, dmmurray@miami-airport.com
David M. Murray, dmmurray@miami-airport.com
David M. Murray, rmartin@miami-airport.com
David M. Wells, dwells@gunster.com
David M. Wells, dculmer@gunster.com
David M. Wells, eservice@gunster.com
David Stanoch, david@honiklaw.com
Dorian N. Daggs, ddaggs@hsmmpa.com
Douglas J. Kress, dkress@schwedpa.com
Douglas J. Kress, lrandell@schwedpa.com
Douglas J. Kress, amarcus@schwedpa.com
Dustin C. Blumenthal, dblumenthal@goldbergsegalla.com
Dustin C. Blumenthal, ppowers@goldbergsegalla.com
E. Bruce Johnson, johnson@jambg.com
E. Bruce Johnson, young@jambg.com
Edward Marod, emarod@gunster.com
Edward Marod, dpeterson@gunster.com
Edward R. Blumberg, erb@deutschblumberg.com
Edward R. Blumberg, rmitchell@deutschblumberg.com
Edward R. Blumberg, kdominguez@deutschblumberg.com
Elliot H. Scherker, scherkere@gtlaw.com
Elliot H. Scherker, miamiappellateservice@gtlaw.com
Eric P. Hockman, ehockman@wsh-law.com
Eric P. Hockman, lmartinez@wsh-law.com
Eric Page Hockman, ehockman@wsh-law.com
Eric Page Hockman, lmartinez@wsh-law.com
Eric S Kay, ekay@kttlaw.com
Eric S Kay, ga@kttlaw.com
Eric S Kay, agarcia@kttlaw.com
Felipe Gonzalez, fgonzalez@switkeslaw.com
Felipe Gonzalez, paralegal@switkeslaw.com
Felipe Gonzalez, paralegal@switkeslaw.com

Frank Florio, FFlorio@gunster.com
George R Truitt Jr., george.truitt@csklegal.com
George R Truitt Jr., construction.miami@csklegal.com
Gonzalo Barr, gbarr@dldlawyers.com
Gonzalo Barr, viviane@dldlawyers.com
Gonzalo Dorta, grd@dortalaw.com
Gonzalo R Dorta, grd@dortalaw.com
Gonzalo R Dorta, jpedraza@dortalaw.com
Gonzalo R Dorta, jgonzalez@dortalaw.com
Graham LippSmith, g@lippsmith.com
Graham LippSmith, mb@lippsmith.com
Graham LippSmith, cca@lippsmith.com
H. Clay Roberts, gloria@robertspa.com
H. Clay Roberts, roberts@robertspa.com
H. Clay Roberts, jennifer@robertspa.com
Hali E Marsocci, Hali@RomanoLawGroup.com
Hali E Marsocci, Becky@RomanoLawGroup.com
Hali E Marsocci, Service@RomanoLawGroup.com
Harley S. Tropin Esq., hst@kttlawn.com
Harley S. Tropin Esq., sf@kttlawn.com
Harley Tropin, hst@kttlawn.com
Henry Lawrence Perry, lperry@perry-young.com
Henry Lawrence Perry, kroberson@perry-young.com
Henry Lawrence Perry, frichard@perry-young.com
Henry N. Wixon, henry.wixon@nist.gov
Houston S. Park III, parkh@kleinpark.com
Houston S. Park III, harriss@kleinpark.com
Howard Kraft Pita, spita@powlawfirm.com
Howard Kraft Pita, lalvarez@powlawfirm.com
Howard Kraft Pita, pitaservice@powlawfirm.com
Howard M Bushman, howard@moskowitz-law.com
Howard M Bushman, dione@moskowitz-law.com
JOHN H RUIZ, jruiz@msprecoverylawfirm.com
JOHN H RUIZ, serve@msprecoverylawfirm.com
JOHN H RUIZ, jruiz@msprecovery.com
Janel C. Diamond, jdiamond@gunster.com
Javier A. Lopez Esq., jal@kttlawn.com
Javier A. Lopez Esq., ya@kttlawn.com

Javier A. Lopez Esq., fsr@kttlaw.com
Javier Zapata, jzapata@miamidade.gov
Jeffrey Lee Newsome II, jeffrey.newsome@phelps.com
Jeffrey Lee Newsome II, samantha.powell@phelps.com
Jeffrey Michael Cohen, jmcohen@cfjblaw.com
Jeffrey Michael Cohen, schacon@carltonfields.com
Jeffrey Michael Cohen, miaecf@cfdom.net
Jillian M Askren, askrenj@gtlaw.com
Jillian M Askren, thomasm@gtlaw.com
Joel L McNabney, joel.mcnabney@clydeco.us
Joel L McNabney, Audrie.Finney@clydeco.us
Joel L McNabney, miainsfilings@clydeco.us
John B. Morgan, jmorgan@forthepeople.com
John B. Morgan, kmitnik@forthepeople.com
John B. Morgan, andrew@forthepeople.com
John Davis, jdavis@slackdavis.com
John H. Ruiz, jruiz@msprecoverylawfirm.com
John H. Ruiz, serve@msprecoverylawfirm.com
John H. Ruiz, afernandez@msprecoverylawfirm.com
John Romano, John@RomanoLawGroup.com
John Scarola, _scarolateam@searcylaw.com
John Scarola, mmccann@searcylaw.com
John Scarola, _scarolateam@searcylaw.com
Jonathan E Kanov, jekanov@mdwgc.com
Jonathan E Kanov, kafriday@mdwgc.com
Jordi Gusó, jguso@bergersingerman.com
Jordi Gusó, drt@bergersingerman.com
Jordi Gusó, fsellers@bergersingerman.com
Jorge A Calil Esq., jorge@jcalillaw.com
Jorge A Calil Esq., jeannie.calillaw@gmail.com
Jorge L. Piedra, jpiedra@kttlaw.com
Jorge L. Piedra, ga@kttlaw.com
Jorge Silva, jsilva@silvasilva.com
Jorge Silva, csilva@silvasilva.com
Jorge Silva, hsoto@silvasilva.com
Joseph H. Serota, jserota@wsh-law.com
Joseph H. Serota, lmartinez@wsh-law.com
Joseph M. Kaye, joseph@moskowitz-law.com

Joseph M. Kaye, dione@moskowitz-law.com
Josh M Rubens, jrubens@klugerkaplan.com
Josh M Rubens, cfalla@klugerkaplan.com
Josh M Rubens, cfernandez@klugerkaplan.com
Joshua Talcovitz, TalcovitzJ@kleinpark.com
Jourdan Leslie Weltman, jw0326@universalproperty.com
Jourdan Leslie Weltman, subrogationservice@universalproperty.com
Jourdan Leslie Weltman, vm1217@universalproperty.com
Judd G. Rosen, pleadings@goldbergandrosen.com
Judd G. Rosen, jgrsecy@goldbergandrosen.com
Julia Holden-Davis, JHoldenDavis@gunster.com
Julia Holden-Davis, dholland@gunster.com
Julian S. Geraci Esq., jgeraci@pbcgov.org
Julian S. Geraci Esq., aairey@pbcgov.org
Karen B Parker, kparker@kbparkerlaw.com
Karen B Parker, fmartinez@kbparkerlaw.com
Karen B Parker, ebacker@kbparkerlaw.com
Kenneth R Drake, kendrake@dldlawyers.com
Kenneth R Drake, viviane@dldlawyers.com
Kerry L. Burns, kburns@bergersingerman.com
Kerry L. Burns, efile@bergersingerman.com
Laritza Orozco, Orozcol@kleinpark.com
Laura Adams, lauraadams@miamisao.com
Lauren E. Morse, laurenm@miamidade.gov
Lauren E. Morse, olga1@miamidade.gov
Lauren E. Morse, hern@miamidade.gov
Lauren Morse, lauren.morse@miamidade.gov
Luis Eduardo Suarez, lsuarez@hsmpa.com
Luis Eduardo Suarez, filings@hsmpa.com
Manual Arteaga-Gomez, aag@grossmanroth.com
Marc J. Gottlieb, marc.gottlieb@akerman.com
Marc J. Gottlieb, joyce.gutierrez@akerman.com
Marc J. Gottlieb, Esq., marc.gottlieb@akerman.com
Marc J. Gottlieb, Esq., joyce.gutierrez@akerman.com
Mark A Boyle, Eservice@Insurance-Counsel.com
Mark A Boyle, MBoyle@Insurance-Counsel.com
Mark A Boyle, InsuranceCounsel2050@gmail.com
Mark J. Heise, mheise@hsmpa.com

Mark J. Sullivan, sullivanm@kleinpark.com
Mark J. Sullivan, piersonj@kleinpark.com
Mark J. Sullivan, montanem@kleinpark.com
Mark R. Antonelli, mantonelli@gaebemullen.com
Mark R. Antonelli, cgreer@gaebemullen.com
Mark R. Antonelli, lbeggs@gaebemullen.com
Matthew Wildner, mjwildner@mdwgc.com
Matthew Wildner, kafriday@mdwgc.com
Meghan C Moore Moore, meghan.moore@flastergreenberg.com
Meghan C Moore Moore, betty.plasencia@flastergreenberg.com
Michael Caballero, mcaballero@hsmpa.com
Michael F. Suarez, MFS-KD@kubickidraper.com
Michael F. Suarez, mfs@kubickidraper.com
Michael I. Goldberg, michael.goldberg@akerman.com
Michael I. Goldberg, charlene.cerda@akerman.com
Michael I. Goldberg, kimberly.smiley@akerman.com
Michael J Thomas, thomasmic@gtlaw.com
Michael J Thomas, cordovam@gtlaw.com
Michael J Thomas, flservice@gtlaw.com
Michael S Hooker, michael.hooker@phelps.com
Michael S Hooker, guy.mcconnell@phelps.com
Michael S Hooker, renee.hogue@phelps.com
Mitchel Chusid, mchusid@ritterchusid.com
Mitchel Chusid, karenz@ritterchusid.com
Mitchel Chusid, ifeliciano@ritterchusid.com
Mustafa Hasan Dandashly, pleadings@goldbergandrosen.com
Mustafa Hasan Dandashly, mdandashly@goldbergandrosen.com
Mustafa Hasan Dandashly, evelyn@goldbergandrosen.com
Nicole Juarez, njuarez@hsmpa.com
Oscar E. Marrero, oem@marrerolegal.com
Patricia Melville, pmelville@hsmpa.com
Paul A. Avron, pavron@bergersingerman.com
Paul J. Schwiep, pschwiep@coffeyburlington.com
Paul J. Schwiep, yvb@coffeyburlington.com
Paul J. Schwiep, service@coffeyburlington.com
Paul Jon Layne, playne@silvasilva.com
Paul Jon Layne, mromera@silvasilva.com
Paul Jon Layne, vramos@silvasilva.com

Paul S Singerman, singerman@bergersingerman.com
Paul S Singerman, mdiaz@bergersingerman.com
Paul S Singerman, efile@bergersingerman.com
Paul Steven Labiner, Pleadings@PIALawCenter.com
Rachel W. Furst, rwf@grossmanroth.com
Rachel Wagner Furst, rwf@grossmanroth.com
Ralph George Patino, service@patinolaw.com
Ralph George Patino, rpatino@patinolaw.com
Ralph George Patino, cluna@patinolaw.com
Rami Shmuely, rshmuely@cmslawgroup.com
Randy M Weber, rmweber@pwndlawfirm.com
Randy M Weber, ngarcia@pwndlawfirm.com
Ricardo Manoel Martinez-Cid, rmcid@podhurst.com
Ricardo Manoel Martinez-Cid, RMCTeam@podhurst.com
Ricardo Manoel Martinez-Cid, lbarrington@podhurst.com
Robert J. Mongeluzzi, rmongeluzzi@smbb.com
Robert J. Mongeluzzi, jgoodman@smbb.com
Robert J. Mongeluzzi, sdordick@smbb.com
Robert L Switkes, Rswitkes@switkeslaw.com
Robert L Switkes, paralegal@switkeslaw.com
Robert L Switkes, bzappala@switkeslaw.com
Robert M Klein, kleinr@kleinpark.com
Robert M Klein, piersonj@kleinpark.com
Robert M Klein, carrillom@kleinpark.com
Roselyn Lax, rlax@hsmipa.com
Ruben Honik, ruben@honiklaw.com
Ryan A Waton, rwaton@zdlaw.com
Ryan J. Yaffa, rjy@grossmanroth.com
Ryan M. Charlson, ryan.charlson@csklegal.com
Ryan M. Charlson, nicole.kaufman@csklegal.com
Ryan Thomas Hopper, hopperr@gtlaw.com
Ryan Thomas Hopper, ryan.t.hopper@gmail.com
Ryan Thomas Hopper, thomasm@gtlaw.com
STEVEN JEFFREY BRODIE, sbrodie@carltonfields.com
STEVEN JEFFREY BRODIE, ldelpino@carltonfields.com
Scott Andrew Hiaasen, shiaasen@coffeyburlington.com
Scott Andrew Hiaasen, lperez@coffeyburlington.com
Scott Andrew Hiaasen, service@coffeyburlington.com

Sergio Bueno, sergio.bueno@clydeco.us
Sergio Bueno, Audrie.Finney@clydeco.us
Sergio Bueno, miainsfilings@clydeco.us
Sergio L Mendez, sergio@mendezandmendezlaw.com
Sergio L Mendez, service@mendezandmendezlaw.com
Sergio L Mendez, lorena@mendezandmendezlaw.com
Seth M Lehrman, staff.efile@epllc.com
Seth M Lehrman, seth@epllc.com
Seth M Lehrman, iris@epllc.com
Seth M Schimmel, seth.schimmel@phelps.com
Seth M Schimmel, yolanda.vazquez@phelps.com
Shannon Lyn Nunez del Prado, sdelprado@pwwlawfirm.com
Stuart Z Grossman, szg@grossmanroth.com
Stuart Z Grossman, lka@grossmanroth.com
Tal J Lifshitz, tj1@kttlaw.com
Tal J Lifshitz, ya@kttlaw.com
Tal J Lifshitz, fsr@kttlaw.com
Tal J. Lifshitz, tj1@kttlaw.com
Tal J. Lifshitz, ya@kttlaw.com
Thomas A. Robes, Trobes@robeslawgroup.com
Thomas A. Robes, jbuchko@robeslawgroup.com
Thomas A. Robes, dmarie@robeslawgroup.com
Todd Romano, Todd@RomanolawGroup.com
Valerie Shea, vshea@goldbergsegalla.com
Wanda Monteverde, monteverdew@kleinpark.com
William F. Merlin Jr, cmerlin@merlinlawgroup.com
William F. Merlin Jr, ssmith@merlinlawgroup.com
William F. Merlin Jr, trodriguez@merlinlawgroup.com
William J Tinsley Jr., william.tinsley@phelps.com
William J Tinsley Jr., yolanda.vazquez@phelps.com
William P. Mulligan, wpm@grossmanroth.com
William P. Mulligan, lka@grossmanroth.com
Yechezkel Rodal, chezky@forthepeople.com
Yechezkel Rodal, rmongeluzzi@smbb.com
Yechezkel Rodal, jgoodman@smbb.com
Yechezkel Rodal, chezky@forthepeople.com
Yitzhak Levin, ylevin@levinlitigation.com
Yitzhak Levin, service@levinlitigation.com

Yummy Marrero, Ymarrero@gaebemullen.com

Physically Served:

**IN THE CIRCUIT COURT OF THE ELEVENTH JUDICIAL
CIRCUIT IN AND FOR MIAMI-DADE COUNTY, FLORIDA**

CASE NO: 2021-015089-CA-01

SECTION: CA43

JUDGE: Michael Hanzman

In Re: Champlain Towers South Collapse Litigation

Plaintiff(s)

vs.

N/A

Defendant(s)

_____ /

**ORDER REGARDING JOINT ACCESS PROTOCOL FOR NON-INVASIVE REVIEW
AND TESTING – PRIMARY EVIDENCE FACILITY – PHASE ONE**

THIS CAUSE came before the Court upon a series of conferences and hearings, including the December 22, 2021 evidentiary hearing, the Court's December 30, 2021, *sua sponte* Order, the Court's January 3, 2022 Order, and hearings on January 14, 2022, January 21, 2022, and thereafter. The Court having considered the issues and positions of the parties, it is **ORDERED AND ADJUDGED** as follows:

1. The *CHAMPLAIN TOWERS SOUTH COLLAPSE INVESTIGATION: Joint Access Protocol for Non-invasive Review and Testing – Primary Evidence Facility – PHASE ONE* ("Primary Evidence Facility Protocol"; attached as Exhibit A) is APPROVED and ADOPTED as an ORDER of the Court.^{[\[1\]](#)}
 2. Activities at the Primary Evidence Facility shall proceed and continue as set forth in the *Primary Evidence Facility Protocol* and a PHASE TWO protocol shall be developed on the testing and sampling of the materials at the Primary Evidence Facility.
-

^[1] Capitalized terms shall have their meaning as defined in the *Primary Evidence Facility Protocol*.

DONE and **ORDERED** in Chambers at Miami-Dade County, Florida on this 15th day of March, 2022.



2021-015089-CA-01 03-15-2022 4:43 PM

Hon. Michael Hanzman

CIRCUIT COURT JUDGE

Electronically Signed

No Further Judicial Action Required on **THIS MOTION**

CLERK TO **RECLOSE** CASE IF POST JUDGMENT

Electronically Served:

Aaron Podhurst, apodhurst@podhurst.com
Aaron Podhurst, dricker@podhurst.com
Adam A Schwartzbaum, adams@moskowitz-law.com
Adam A Schwartzbaum, service@moskowitz-law.com
Adam A Schwartzbaum, dione@moskowitz-law.com
Adam Moskowitz, adam@moskowitz-law.com
Adam Moskowitz, dione@moskowitz-law.com
Adam Moskowitz, service@moskowitz-law.com
Alfred Armas, alfred@armaslaw.com
Alison E Patino, apatino@patinolaw.com
Amanda Anderson, AAnderson@insurance-counsel.com
Amanda K Anderson, eservice@insurance-counsel.com
Amanda K Anderson, aanderson@insurance-counsel.com
Amanda K Anderson, InsuranceCounsel2050@gmail.com
Ana Angelica Viciano, anita@miamidade.gov
Ana Angelica Viciano, mbarber@miamidade.gov
Andrew B. Yaffa, aby@grossmanroth.com
Andrew B. Yaffa, omb@grossmanroth.com
Andrew M Feldman, feldmana@kleinpark.com

Andrew M Feldman, montanem@kleinpark.com
Andrew M Feldman, piersonj@kleinpark.com
Andrew P. Gold, Esq., andrew.gold@akerman.com
Andrew P. Gold, Esq., jill.parnes@akerman.com
Andrew Paul Gold, andrew.gold@akerman.com
Andrew Paul Gold, jill.parnes@akerman.com
Andrew Paul Gold, bobbi.engelke@akerman.com
Anthony J Carriuolo, acarriuolo@bergersingerman.com
Anthony J Carriuolo, mnewland@bergersingerman.com
Anthony J Carriuolo, drt@bergersingerman.com
Anthony Perez, aperez@hempa.com
Aron Raskas, araskas@gunster.com
Aron Raskas, avalido@gunster.com
Aron Raskas, eservice@gunster.com
Benjamin Fernandez IV, bfernandez@silvasilva.com
Benjamin Fernandez IV, cvereen@silvasilva.com
Benjamin Fernandez, IV, bfernandez@silvasilva.com
Benjamin Fernandez, IV, tgomez@silvasilva.com
Bernadette Vazquez, bvazquez@klugerkaplan.com
Bernadette Vazquez, clong@klugerkaplan.com
Bradford R. Sohn, brad@bradsohnlaw.com
Bradford R. Sohn, paralegal@bradsohnlaw.com
Bradford R. Sohn, service.bradsohnlaw@gmail.com
Bradley J Edwards, staff.efile@epllc.com
Bradley J Edwards, brad@epllc.com
Bradley J Edwards, maria@epllc.com
Brenda Radmacher, brenda.radmacher@akerman.com
Bret M Feldman, feldmanb@phelps.com
Bret M Feldman, yolanda.vazquez@phelps.com
Brian S Dervishi, bdervishi@wdpalaw.com
Brian S Dervishi, service@wdpalaw.com
Bruce Alan Katzen, bkatzen@klugerkaplan.com
Bruce Alan Katzen, cfalla@klugerkaplan.com
Bruce Alan Katzen, probate@klugerkaplan.com
Carlos A. Velasquez, CVelasquez@VDLawyers.com
Carlos A. Velasquez, Andrea@VDLawyers.com
Carlos E Silva, csilva@silvasilva.com
Carlos E Silva, mromera@silvasilva.com

Carlos M. Macias, macias@leesfield.com
Carlos M. Macias, becerra@leesfield.com
Carlos M. Macias, abreu@leesfield.com
Carmen M. Ortega-Rivero, carmen.ortega-rivero@akerman.com
Carmen M. Ortega-Rivero, wendy.gonzalez@akerman.com
Caroline Catchpole Spradlin, caroline.spradlin@phelps.com
Caroline Catchpole Spradlin, samantha.powell@phelps.com
Carolyn M. Luna, cluna@patinolaw.com
Christine L. Welstead, christine.welstead@bowmanandbrooke.com
Christine L. Welstead, ashleigh.carroll@bowmanandbrooke.com
Christine L. Welstead, lisa.morales@bowmanandbrooke.com
Christopher L Barnett, barnettch@gtlaw.com
Christopher L Barnett, nicauda@gtlaw.com
Christopher S Carver, christopher.carver@akerman.com
Christopher S Carver, cary.gonzalez@akerman.com
Cole Scott &Kissane PA, Ryan.Charlson@csklegal.com
Cole Scott &Kissane PA, Ryan.Sooahoo@csklegal.com
Cole Scott &Kissane PA, Construction.FTLW@csklegal.com
Cosme Caballero, ccaballero@deutschblumberg.com
Cosme Caballero, kdominguez@deutschblumberg.com
Cosme Caballero, Esq., ccaballero@deutschblumberg.com
Cosme Caballero, Esq., bblumberg@deutschblumberg.com
Curtis Miner, curt@colson.com
Curtis Miner, claudiav@colson.com
Curtis Miner, eservice@colson.com
Daniel Y Gielchinsky, dan@dgimlaw.com
Daniel Y Gielchinsky, eservice@dgimlaw.com
Daniel Y Gielchinsky, colleen@dgimlaw.com
David B. Weinstein, weinsteind@gtlaw.com
David B. Weinstein, thomasm@gtlaw.com
David L Rosinsky Esq., LUKSFLL-Pleadings@LS-LAW.COM
David L Rosinsky Esq., Drosinsky@insurancedefense.net
David L Rosinsky Esq., spare@insurancedefense.net
David M Wells, dwells@gunster.com
David M Wells, dculmer@gunster.com
David M. Murray, dmmurray@miami-airport.com
David M. Murray, dmmurray@miami-airport.com
David M. Murray, rmartin@miami-airport.com

David M. Wells, dwells@gunster.com
David M. Wells, dculmer@gunster.com
David M. Wells, eservice@gunster.com
David Stanoch, david@honiklaw.com
Dorian N. Daggs, ddaggs@hsmpa.com
Douglas J. Kress, dkress@schwedpa.com
Douglas J. Kress, lrandell@schwedpa.com
Douglas J. Kress, amarcus@schwedpa.com
Dustin C. Blumenthal, dblumenthal@goldbergsegalla.com
Dustin C. Blumenthal, ppowers@goldbergsegalla.com
E. Bruce Johnson, johnson@jambg.com
E. Bruce Johnson, young@jambg.com
Edward Marod, emarod@gunster.com
Edward Marod, dpeterson@gunster.com
Edward R. Blumberg, erb@deutschblumberg.com
Edward R. Blumberg, rmitchell@deutschblumberg.com
Edward R. Blumberg, kdominguez@deutschblumberg.com
Elliot H. Scherker, scherkere@gtlaw.com
Elliot H. Scherker, miamiappellateservice@gtlaw.com
Eric P. Hockman, ehockman@wsh-law.com
Eric P. Hockman, lmartinez@wsh-law.com
Eric Page Hockman, ehockman@wsh-law.com
Eric Page Hockman, lmartinez@wsh-law.com
Eric S Kay, ekay@kttlaw.com
Eric S Kay, ga@kttlaw.com
Eric S Kay, agarcia@kttlaw.com
Felipe Gonzalez, fgonzalez@switkeslaw.com
Felipe Gonzalez, paralegal@switkeslaw.com
Felipe Gonzalez, paralegal@switkeslaw.com
Frank Florio, FFlorio@gunster.com
George R Truitt Jr., george.truitt@csklegal.com
George R Truitt Jr., construction.miami@csklegal.com
Gonzalo Barr, gbarr@dldlawyers.com
Gonzalo Barr, viviane@dldlawyers.com
Gonzalo Dorta, grd@dortalaw.com
Gonzalo R Dorta, grd@dortalaw.com
Gonzalo R Dorta, jpedraza@dortalaw.com
Gonzalo R Dorta, jgonzalez@dortalaw.com

Graham LippSmith, g@lippsmith.com
Graham LippSmith, mb@lippsmith.com
Graham LippSmith, cca@lippsmith.com
Gregory J Kahn, gkahn@boydjen.com
Gregory J Kahn, jsaiz@boydjen.com
Gregory J Kahn, mkruemmel@boydjen.com
H. Clay Roberts, gloria@robertspa.com
H. Clay Roberts, roberts@robertspa.com
H. Clay Roberts, jennifer@robertspa.com
Hali E Marsocci, Hali@RomanoLawGroup.com
Hali E Marsocci, Becky@RomanoLawGroup.com
Hali E Marsocci, Service@RomanoLawGroup.com
Harley S. Tropin Esq., hst@kttlawn.com
Harley S. Tropin Esq., sf@kttlawn.com
Harley Tropin, hst@kttlawn.com
Henry Lawrence Perry, lperry@perry-young.com
Henry Lawrence Perry, kroberson@perry-young.com
Henry Lawrence Perry, frichard@perry-young.com
Henry N. Wixon, henry.wixon@nist.gov
Houston S. Park III, parkh@kleinpark.com
Houston S. Park III, harriss@kleinpark.com
Howard Kraft Pita, spita@pwwlawfirm.com
Howard Kraft Pita, lalvarez@pwwlawfirm.com
Howard Kraft Pita, pitaservice@pwwlawfirm.com
Howard M Bushman, howard@moskowitz-law.com
Howard M Bushman, dione@moskowitz-law.com
JOHN H RUIZ, jruiz@msprecoverylawfirm.com
JOHN H RUIZ, serve@msprecoverylawfirm.com
JOHN H RUIZ, jruiz@msprecovery.com
Janel C. Diamond, jdiamond@gunster.com
Javier A. Lopez Esq., jal@kttlawn.com
Javier A. Lopez Esq., ya@kttlawn.com
Javier A. Lopez Esq., fsr@kttlawn.com
Javier Zapata, jzapata@miamidade.gov
Jeffrey Lee Newsome II, jeffrey.newsome@phelps.com
Jeffrey Lee Newsome II, samantha.powell@phelps.com
Jeffrey Michael Cohen, jmcohen@cfjblaw.com
Jeffrey Michael Cohen, schacon@carltonfields.com

Jeffrey Michael Cohen, miaecf@cfdom.net
Jennifer Glasser, jennifer.glasser@akerman.com
Jennifer Glasser, kelly.connolly@akerman.com
Jennifer Glasser, kim.stathopoulos@akerman.com
Jillian M Askren, askrenj@gtlaw.com
Jillian M Askren, thomasm@gtlaw.com
Joel L McNabney, joel.mcnabney@clydeco.us
Joel L McNabney, Audrie.Finney@clydeco.us
Joel L McNabney, miainsfilings@clydeco.us
John B. Morgan, jmorgan@forthepeople.com
John B. Morgan, kmitnik@forthepeople.com
John B. Morgan, andrew@forthepeople.com
John Davis, jdavis@slackdavis.com
John H. Ruiz, jruiz@msprecoverylawfirm.com
John H. Ruiz, serve@msprecoverylawfirm.com
John H. Ruiz, afernandez@msprecoverylawfirm.com
John Romano, John@RomanoLawGroup.com
John Scarola, _scarolateam@searcylaw.com
John Scarola, mmccann@searcylaw.com
John Scarola, _scarolateam@searcylaw.com
Jonathan E Kanov, jekanov@mdwgc.com
Jonathan E Kanov, kafriday@mdwgc.com
Jordi Gusó, jguso@bergersingerman.com
Jordi Gusó, drt@bergersingerman.com
Jordi Gusó, fsellers@bergersingerman.com
Jorge A Calil Esq., jorge@jcalillaw.com
Jorge A Calil Esq., jeannie.calillaw@gmail.com
Jorge L. Piedra, jpiedra@kttlalaw.com
Jorge L. Piedra, ga@kttlalaw.com
Jorge Silva, jsilva@silvasilva.com
Jorge Silva, csilva@silvasilva.com
Jorge Silva, hsoto@silvasilva.com
Joseph H. Serota, jserota@wsh-law.com
Joseph H. Serota, lmartinez@wsh-law.com
Joseph M. Kaye, joseph@moskowitz-law.com
Joseph M. Kaye, dione@moskowitz-law.com
Josh M Rubens, jrubens@klugerkaplan.com
Josh M Rubens, cfernandez@klugerkaplan.com

Josh M Rubens, probate@klugerkaplan.com
Joshua D Fuller, jfuller@fullerlawyers.com
Joshua D Fuller, lposch@fullerlawyers.com
Joshua Talcovitz, TalcovitzJ@kleinpark.com
Jourdan Leslie Weltman, jw0326@universalproperty.com
Jourdan Leslie Weltman, subrogationservice@universalproperty.com
Jourdan Leslie Weltman, vm1217@universalproperty.com
Judd G. Rosen, pleadings@goldbergandrosen.com
Judd G. Rosen, jgrsecy@goldbergandrosen.com
Julia Holden-Davis, JHoldenDavis@gunster.com
Julia Holden-Davis, dholland@gunster.com
Julian S. Geraci Esq., jgeraci@pbcgov.org
Julian S. Geraci Esq., aairey@pbcgov.org
Karen B Parker, kparker@kbparkerlaw.com
Karen B Parker, fmartinez@kbparkerlaw.com
Karen B Parker, ebacker@kbparkerlaw.com
Kenneth R Drake, kendrake@dldlawyers.com
Kenneth R Drake, viviane@dldlawyers.com
Kerry L. Burns, kburns@bergersingerman.com
Kerry L. Burns, efile@bergersingerman.com
Kimberly C. Hirschman, khirschman@therhlawfirm.com
Kimberly C. Hirschman, nramos@therhlawfirm.com
Laritza Orozco, Orozcol@kleinpark.com
Laura Adams, lauraadams@miamisao.com
Lauren E. Morse, laurenm@miamidade.gov
Lauren E. Morse, olgal@miamidade.gov
Lauren E. Morse, hern@miamidade.gov
Lauren Morse, lauren.morse@miamidade.gov
Lewis N Jack Jr., LNJ@florida-attorneys.com
Lewis N Jack Jr., cmf@florida-attorneys.com
Luis Eduardo Suarez, lsuarez@hsmpa.com
Luis Eduardo Suarez, filings@hsmpa.com
Manual Arteaga-Gomez, aag@grossmanroth.com
Marc J. Gottlieb, marc.gottlieb@akerman.com
Marc J. Gottlieb, joyce.gutierrez@akerman.com
Marc J. Gottlieb, Esq., marc.gottlieb@akerman.com
Marc J. Gottlieb, Esq., joyce.gutierrez@akerman.com
Maria Corghi, mcorghi@silvasilva.com

Maria Corghi, mcorghi@silvasilva.com
Mark A Boyle, Eservice@Insurance-Counsel.com
Mark A Boyle, MBoyle@Insurance-Counsel.com
Mark A Boyle, InsuranceCounsel2050@gmail.com
Mark G. Dicowden, eservice@dicowdenlaw.com
Mark G. Dicowden, service@dicowdenlaw.com
Mark G. Dicowden, mgd@dicowdenlaw.com
Mark J. Heise, mheise@hsmpa.com
Mark J. Sullivan, sullivanm@kleinpark.com
Mark J. Sullivan, piersonj@kleinpark.com
Mark J. Sullivan, montanem@kleinpark.com
Mark R. Antonelli, mantonelli@gabemullen.com
Mark R. Antonelli, cgreer@gabemullen.com
Mark R. Antonelli, lbeggs@gabemullen.com
Matthew Wildner, mjwildner@mdwcg.com
Matthew Wildner, kafriday@mdwcg.com
Meghan C Moore Moore, meghan.moore@flastergreenberg.com
Meghan C Moore Moore, betty.plasencia@flastergreenberg.com
Michael Caballero, mcaballero@hsmpa.com
Michael F. Suarez, MFS-KD@kubickidraper.com
Michael F. Suarez, mfs@kubickidraper.com
Michael I. Goldberg, michael.goldberg@akerman.com
Michael I. Goldberg, charlene.cerda@akerman.com
Michael I. Goldberg, kimberly.smiley@akerman.com
Michael J Thomas, thomasmic@gtlaw.com
Michael J Thomas, cordovam@gtlaw.com
Michael J Thomas, flservice@gtlaw.com
Michael S Hooker, michael.hooker@phelps.com
Michael S Hooker, guy.mcconnell@phelps.com
Michael S Hooker, renee.hogue@phelps.com
Mitchel Chusid, mchusid@ritterchusid.com
Mitchel Chusid, karenz@ritterchusid.com
Mitchel Chusid, ifeliciano@ritterchusid.com
Mustafa Hasan Dandashly, pleadings@goldbergandrosen.com
Mustafa Hasan Dandashly, mdandashly@goldbergandrosen.com
Mustafa Hasan Dandashly, evelyn@goldbergandrosen.com
Nicole Juarez, njuarez@hsmpa.com
Oscar E. Marrero, oem@marrerolegal.com

Patricia Melville, pmelville@hsmpla.com
Paul A. Avron, pavron@bergersingerman.com
Paul J. Schwiep, pschwiep@coffeyburlington.com
Paul J. Schwiep, yvb@coffeyburlington.com
Paul J. Schwiep, service@coffeyburlington.com
Paul Jon Layne, playne@silvasilva.com
Paul Jon Layne, mromera@silvasilva.com
Paul Jon Layne, vramos@silvasilva.com
Paul S Singerman, singerman@bergersingerman.com
Paul S Singerman, mdiaz@bergersingerman.com
Paul S Singerman, efile@bergersingerman.com
Paul Steven Labiner, Pleadings@PIALawCenter.com
Rachel Charys John, rjohn@zdlaw.com
Rachel Charys John, rjohn@zdlaw.com
Rachel Charys John, rjohn@zdlaw.com
Rachel W. Furst, rwf@grossmanroth.com
Rachel Wagner Furst, rwf@grossmanroth.com
Ralph George Patino, service@patinolaw.com
Ralph George Patino, rpatino@patinolaw.com
Ralph George Patino, cluna@patinolaw.com
Rami Shmueli, rshmueli@cmlawgroup.com
Randy M Weber, rmweber@pwwlawfirm.com
Randy M Weber, ngarcia@pwwlawfirm.com
Ricardo Manoel Martinez-Cid, rmcid@podhurst.com
Ricardo Manoel Martinez-Cid, RMCTeam@podhurst.com
Ricardo Manoel Martinez-Cid, lbarrington@podhurst.com
Robert J. Mongeluzzi, rmongeluzzi@smbb.com
Robert J. Mongeluzzi, jgoodman@smbb.com
Robert J. Mongeluzzi, sdordick@smbb.com
Robert L Switkes, Rswitkes@switkeslaw.com
Robert L Switkes, paralegal@switkeslaw.com
Robert L Switkes, bzappala@switkeslaw.com
Robert M Klein, kleinr@kleinpark.com
Robert M Klein, piersonj@kleinpark.com
Robert M Klein, carrillom@kleinpark.com
Roselyn Lax, rlax@hsmpla.com
Ruben Honik, ruben@honiklaw.com
Ryan A Waton, rwaton@zdlaw.com

Ryan J. Yaffa, rjy@grossmanroth.com
Ryan M. Charlson, ryan.charlson@csklegal.com
Ryan M. Charlson, nicole.kaufman@csklegal.com
Ryan Thomas Hopper, hopperr@gtlaw.com
Ryan Thomas Hopper, ryan.t.hopper@gmail.com
Ryan Thomas Hopper, thomasm@gtlaw.com
STEVEN JEFFREY BRODIE, sbrodie@carltonfields.com
STEVEN JEFFREY BRODIE, ldelpino@carltonfields.com
Scott Andrew Hiaasen, shiaasen@coffeyburlington.com
Scott Andrew Hiaasen, lperez@coffeyburlington.com
Scott Andrew Hiaasen, service@coffeyburlington.com
Scott D. Rembold, srembold@therhlawfirm.com
Scott D. Rembold, courtdocs@therhlawfirm.com
Sergio L Mendez, sergio@mendezandmendezlaw.com
Sergio L Mendez, service@mendezandmendezlaw.com
Sergio L Mendez, lorena@mendezandmendezlaw.com
Seth M Lehrman, staff.efile@epllc.com
Seth M Lehrman, seth@epllc.com
Seth M Lehrman, iris@epllc.com
Seth M Schimmel, seth.schimmel@phelps.com
Seth M Schimmel, yolanda.vazquez@phelps.com
Seth M Schimmel, nicole.marsade@phelps.com
Shannon Lyn Nunez del Prado, sdelprado@pwwdlawfirm.com
Stuart Z Grossman, szg@grossmanroth.com
Stuart Z Grossman, lka@grossmanroth.com
Tal J Lifshitz, tj1@kttl1aw.com
Tal J Lifshitz, ya@kttl1aw.com
Tal J Lifshitz, fsr@kttl1aw.com
Tal J. Lifshitz, tj1@kttl1aw.com
Tal J. Lifshitz, ya@kttl1aw.com
Thomas A. Robes, Trobes@robeslawgroup.com
Thomas A. Robes, jbuchko@robeslawgroup.com
Thomas A. Robes, dmarie@robeslawgroup.com
Todd Romano, Todd@RomanolawGroup.com
Valerie Shea, vshea@goldbergsegalla.com
Wanda Monteverde, monteverdew@kleinpark.com
William F. Merlin Jr, cmerlin@merlinlawgroup.com
William F. Merlin Jr, ssmith@merlinlawgroup.com

William F. Merlin Jr, trodriguez@merlinlawgroup.com
William J Tinsley Jr., william.tinsley@phelps.com
William J Tinsley Jr., yolanda.vazquez@phelps.com
William P. Mulligan, wpm@grossmanroth.com
William P. Mulligan, lka@grossmanroth.com
Yechezkel Rodal, chezky@forthepeople.com
Yechezkel Rodal, rmongeluzzi@smbb.com
Yechezkel Rodal, jgoodman@smbb.com
Yechezkel Rodal, chezky@forthepeople.com
Yitzhak Levin, ylevin@levinlitigation.com
Yitzhak Levin, service@levinlitigation.com
Yummy Marrero, Ymarrero@gaebemullen.com

Physically Served:

Exhibit A

CHAMPLAIN TOWERS SOUTH COLLAPSE INVESTIGATION:

Joint Access Protocol for Non-invasive Review and Testing – Primary Evidence Facility – PHASE ONE

WHEREAS, the named parties in the action captioned *In Re: Champlain Towers South Collapse Litigation*, Case No. 2021-015089-CA-01, pending in the Eleventh Judicial Circuit Court of Miami-Dade County, Florida, including all putative class members (the “Civil Litigants”) have an interest in physical evidence, relating to the collapse of Champlain Towers South (the “Evidence”), stored in the Primary Evidence Facility, as herein defined;

WHEREAS, the Receiver acting on behalf of the Champlain Towers South Condominium Association, Inc., Michael I. Goldberg, Esq. (the “Receiver”) has ownership of and an interest in the Evidence;

WHEREAS, the National Institute of Standards and Technology (“NIST”) has an interest in the Evidence; and

WHEREAS, the Miami-Dade County Police Department (“MDCPD”) has an interest in the Evidence;

The Civil Litigants, Receiver, NIST, and MDCPD hereby agree:

- 1) To the following access Protocol for non-invasive review and testing of the Evidence;
- 2) To work in good faith to develop a joint access protocol for cooperative management of independent invasive testing of the Evidence; and
- 3) To reserve all rights concerning any assertions or claims of legal rights of access to, or authority over, the Evidence.

ADDITIONAL DEFINITIONS

- **Expert** – Engineer (or other discipline) investigating the collapse on behalf of a Participant.
- **Participant** – A party to this Protocol, including the Receiver, NIST, MDCPD, the Receiver, and the Civil Litigants.
- **Primary Evidence Facility (PEF)** – Warehouse(s) located in Miami-Dade County, Florida and used for storage of the Evidence.
- **Protocol** – This document, titled *CHAMPLAIN TOWERS SOUTH COLLAPSE INVESTIGATION: Joint Access Protocol for Non-invasive Review and Testing – Primary Evidence Facility, Phase One*.
- **NIST Inventory of Evidence**. One or more documents prepared by NIST, or others on behalf of NIST, that indicates NIST’s numbering system for the Evidence stored at the PEF, and the number assigned.

Objective. This Protocol is intended to allow for the non-invasive review and testing by the Civil Litigants and Receiver relating to the Evidence that preserves the chain-of-custody and Evidence for all Participants, as described below for Phase One.

Scope. The Protocol applies to all physical Evidence removed from the Champlain Towers South site and stored at the PEF.

Location of Evidence. Evidence is currently being stored in a PEF, the location of which is known to the Participants. To ensure safe and effective access to the Evidence, it will be necessary to move some of the Evidence to additional PEF(s); however, Evidence at the PEF may not be moved to another PEF absent the prior written consent from the Receiver, representing for that purpose the consent of the Receiver and the Civil Litigants, which consent shall not be unreasonably withheld. All such PEFs shall be located within Miami-Dade County. All Participants shall have the opportunity to document the Evidence to be moved.


Phase One. This Protocol shall be divided into two phases. Phase One is the subject of this agreement. Phase One shall consist of non-invasive review and testing at the PEF. During Phase One, the Civil Litigants and Receiver shall coordinate with NIST to be permitted to access the entire PEF for the purposes of photographing, measuring, and laser-scanning the Evidence located at the PEF. A GPR consultant will be retained by the Civil Litigants and Receiver to accomplish up to two (2) days of ground penetrating radar scanning of select items on a list to be agreed upon by them. Additional consultant scanning shall require the agreement of the Civil Litigants and the Receiver. PEF access shall be done in a safe manner with MDCPD and NIST given full authority to manage personnel on site at any one time for safety and security purposes. To assist with the efficient operation of this Protocol, NIST shall provide the Receiver and Civil Litigants with the NIST Inventory of Evidence.

Participants shall have continuous access to the PEF to carry out the objectives of this Protocol, Monday – Friday from 9am – 5pm starting on February 21, 2022 through and until March 11, 2022. All Participants shall work in good faith and coordinate with each other at all times during the performance of this Protocol, including the modification of the schedule, if needed. NIST shall have the right to restrict access to ensure the safety and security of personnel on site and the integrity of the Evidence. Should NIST restrict access to any individual under such circumstances, the reasoning therefore shall be promptly communicated to all Participants. It is understood that MDCPD and NIST will also be accessing the PEF for non-invasive review and testing during this time period for their respective investigations. At no time shall any Participant handle a piece of Evidence with the intention of altering the Evidence in any way whatsoever without the express prior permission of NIST. Touching of clean surfaces of Evidence with GPR scan equipment shall be permitted, and when scanning, no debris on any surfaces of Evidence shall be disturbed. All participants agree that no Participant shall engage in any destructive testing (this includes penetration tests such as Windsor Probe) until the parties have either agreed upon a Protocol or declared an impasse and had the opportunity to seek Court intervention.

Phase Two. While the Participants investigate the PEF Evidence under Phase One, the Participants shall work in good faith to agree upon a protocol for access for the invasive and destructive testing of Evidence at the PEF (the "Phase Two Protocol").

IT IS SO AGREED.

National Institute of Standards and Technology

By:  **JASON AVERILL**
Digitally signed by JASON AVERILL
Date: 2022.02.25 13:42:41 -05'00'
Jason D. Averill, Division Chief
Materials and Structural Systems Division
Engineering Laboratory

Date: _____

Plaintiffs

By: _____
Jeff Goodman, Esq. (*pro hac vice*)
Saltz Mongeluzzi & Bendesky

Date: _____

Miami-Dade County

By: _____

Date: _____

8701 Collins Development, LLC

By: _____
Michael Thomas, Esq.
Greenberg Traurig, P.A.

Date: _____

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IT IS SO AGREED.

National Institute of Standards and Technology

By: _____
Jason D. Averill, Division Chief
Materials and Structural Systems Division
Engineering Laboratory

Date: _____

Plaintiffs

By: _____
Jeff Goodman, Esq. (*pro hac vice*)
Saltz Mongeluzzi & Bendesky

Date: _____

Miami-Dade County

By: 
JD Patterson
Chief Public Safety Officer

Date: 2/25/2022

JD PATTERSON
CHIEF PUBLIC SAFETY OFFICER
MIAMI-DADE COUNTY, FL

8701 Collins Development, LLC

By: _____
Michael Thomas, Esq.
Greenberg Traurig, P.A.

Date: _____

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IT IS SO AGREED.

National Institute of Standards and Technology

By: _____
Jason D. Averill, Division Chief
Materials and Structural Systems Division
Engineering Laboratory

Date: _____

Plaintiffs

By:  _____
Jeff Goodman, Esq. (pro hac vice)
Saltz Mongeluzi & Bendesky

Date: 3.8.21

Miami-Dade County

By: _____

Date: _____

8701 Collins Development, LLC

By: _____
Michael Thomas, Esq.
Greenberg Traurig, P.A.

Date: _____

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National Institute of Standards and Technology

Miami-Dade County

By: _____
Jason D. Averill, Division Chief
Materials and Structural Systems Division
Engineering Laboratory

By: _____

Date: _____

Date: _____

Plaintiffs

8701 Collins Development, LLC

By: _____
Jeff Goodman, Esq. (*pro hac vice*)
Saltz Mongeluzzi & Bendesky

By: 
Michael Thomas, Esq.
Greenberg Traurig, P.A.

Date: _____

Date: _____

Terra Group, LLC and Terra World Investments, LLC

John Moriarty & Associates of Florida, Inc.

By:


Michael Thomas, Esq.
Greenberg Traurig, P.A.

By:

Jonathan E. Kanov, Esq.
Marshall Dennehey

By:

Seth Schimmel, Esq.
Phelps Dunbar

Date:

3/10/22

Date:

NV5, Inc.

By:

George Truitt, Esq.
Ryan Charlson, Esq.
Cole, Scott & Kissane

Date:

Terra Group, LLC and Terra World Investments, LLC

John Moriarty & Associates of Florida, Inc.

By: _____

Michael Thomas, Esq.
Greenberg Traurig, P.A.

By: _____

Jonathan E. Kanov, Esq.
Marshall Dennehey

By: _____

Seth Schimmel, Esq.
Phelps Dunbar

Date: _____

Date: 3-8-22

NV5, Inc.

By: _____

George Truitt, Esq.
Ryan Charlson, Esq.
Cole, Scott & Kissane

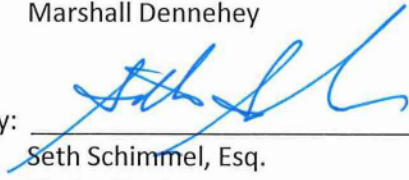
Date: _____

Terra Group, LLC and Terra World Investments, LLC

John Moriarty & Associates of Florida, Inc.

By: _____
Michael Thomas, Esq.
Greenberg Traurig, P.A.

By: _____
Jonathan E. Kanov, Esq.
Marshall Dennehey

By: _____

Seth Schimmel, Esq.
Phelps Dunbar

Date: _____

Date: 3/4/22

NV5, Inc.

By: _____
George Truitt, Esq.
Ryan Charlson, Esq.
Cole, Scott & Kissane

Date: _____

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By: _____
Jonathan E. Kanov, Esq.
Marshall Dennehey

Date: _____

By: _____
Seth Schimmel, Esq.
Phelps Dunbar

Date: _____

NV5, Inc.

By:  _____
George Truitt, Esq.
Ryan Charlson, Esq.
Cole, Scott & Kissane

Date: 3/7/2022

Michael Goldberg, Esq. in his capacity as
Receiver for the Champlain Towers
South Condominium Association, Inc.

By: Brenda Radmacher
Brenda Radmacher (*pro hac vice*)
Akerman LLP

Date: March 5, 2022

**IN THE CIRCUIT COURT OF THE ELEVENTH JUDICIAL
CIRCUIT IN AND FOR MIAMI-DADE COUNTY, FLORIDA**

CASE NO: 2021-015089-CA-01

SECTION: CA43

JUDGE: Michael Hanzman

In re:

Champlain Towers South Collapse Litigation.

NOTICE OF FILING
JOINT ACCESS PROTOCOL FOR NON-INVASIVE REVIEW AND
TESTING – PRIMARY EVIDENCE FACILITY AND
[PROPOSED] *ORDER REGARDING JOINT ACCESS PROTOCOL FOR NON-INVASIVE*
REVIEW AND TESTING – PRIMARY EVIDENCE FACILITY

Champlain Towers South Condominium Association, Inc. (the “Association”), through its Court-appointed Receiver Michael I. Goldberg, hereby files the *Joint Access Protocol for Non-invasive Review and Testing – Primary Evidence Facility* (Exhibit A) and proposed *Order Regarding Joint Access Protocol for Non-invasive Review and Testing – Primary Evidence Facility* (Exhibit B).

Dated: March 15, 2022

Respectfully submitted,

AKERMAN LLP

201 East Las Olas Boulevard, Suite 1800
Fort Lauderdale, Florida 33301-2229
Telephone: (954) 463-2700
Facsimile: (954) 463-2224

By: /s/ Christopher S. Carver

Andrew P. Gold, Esq.
Florida Bar No. 612367
Primary email: andrew.gold@akerman.com
Secondary email: jill.parnes@akerman.com
Christopher Carver, Esq.
Florida Bar No. 993580
Primary email: christopher.carver@akerman.com
Secondary e-mail: cary.gonzalez@akerman.com

and

Brenda Radmacher, Esq.
California Bar No. 185265
Admitted Pro Hac Vice
brenda.radmacher@akerman.com
AKERMAN LLP
601 West Fifth Street, Suite 300
Los Angeles, California 90071
Attorneys for Receiver/Association

CERTIFICATE OF SERVICE

I HEREBY CERTIFY that on March 15, 2022, a true and correct copy of this *Notice of Filing Joint Access Protocol for Non-invasive Review and Testing – Primary Evidence Facility and [proposed] Order Regarding Joint Access Protocol for Non-invasive Review and Testing – Primary Evidence Facility* was filed electronically through the Florida Court’s E-Filing Portal, which will provide electronic service of the filing to all counsel of record.

By: /s/ Christopher S. Carver
Attorney

Exhibit A

CHAMPLAIN TOWERS SOUTH COLLAPSE INVESTIGATION:

Joint Access Protocol for Non-invasive Review and Testing – Primary Evidence Facility – PHASE ONE

WHEREAS, the named parties in the action captioned *In Re: Champlain Towers South Collapse Litigation*, Case No. 2021-015089-CA-01, pending in the Eleventh Judicial Circuit Court of Miami-Dade County, Florida, including all putative class members (the “Civil Litigants”) have an interest in physical evidence, relating to the collapse of Champlain Towers South (the “Evidence”), stored in the Primary Evidence Facility, as herein defined;

WHEREAS, the Receiver acting on behalf of the Champlain Towers South Condominium Association, Inc., Michael I. Goldberg, Esq. (the “Receiver”) has ownership of and an interest in the Evidence;

WHEREAS, the National Institute of Standards and Technology (“NIST”) has an interest in the Evidence; and

WHEREAS, the Miami-Dade County Police Department (“MDCPD”) has an interest in the Evidence;

The Civil Litigants, Receiver, NIST, and MDCPD hereby agree:

- 1) To the following access Protocol for non-invasive review and testing of the Evidence;
- 2) To work in good faith to develop a joint access protocol for cooperative management of independent invasive testing of the Evidence; and
- 3) To reserve all rights concerning any assertions or claims of legal rights of access to, or authority over, the Evidence.

ADDITIONAL DEFINITIONS

- **Expert** – Engineer (or other discipline) investigating the collapse on behalf of a Participant.
- **Participant** – A party to this Protocol, including the Receiver, NIST, MDCPD, the Receiver, and the Civil Litigants.
- **Primary Evidence Facility (PEF)** – Warehouse(s) located in Miami-Dade County, Florida and used for storage of the Evidence.
- **Protocol** – This document, titled *CHAMPLAIN TOWERS SOUTH COLLAPSE INVESTIGATION: Joint Access Protocol for Non-invasive Review and Testing – Primary Evidence Facility, Phase One*.
- **NIST Inventory of Evidence**. One or more documents prepared by NIST, or others on behalf of NIST, that indicates NIST’s numbering system for the Evidence stored at the PEF, and the number assigned.

Objective. This Protocol is intended to allow for the non-invasive review and testing by the Civil Litigants and Receiver relating to the Evidence that preserves the chain-of-custody and Evidence for all Participants, as described below for Phase One.

Scope. The Protocol applies to all physical Evidence removed from the Champlain Towers South site and stored at the PEF.

Location of Evidence. Evidence is currently being stored in a PEF, the location of which is known to the Participants. To ensure safe and effective access to the Evidence, it will be necessary to move some of the Evidence to additional PEF(s); however, Evidence at the PEF may not be moved to another PEF absent the prior written consent from the Receiver, representing for that purpose the consent of the Receiver and the Civil Litigants, which consent shall not be unreasonably withheld. All such PEFs shall be located within Miami-Dade County. All Participants shall have the opportunity to document the Evidence to be moved.


Phase One. This Protocol shall be divided into two phases. Phase One is the subject of this agreement. Phase One shall consist of non-invasive review and testing at the PEF. During Phase One, the Civil Litigants and Receiver shall coordinate with NIST to be permitted to access the entire PEF for the purposes of photographing, measuring, and laser-scanning the Evidence located at the PEF. A GPR consultant will be retained by the Civil Litigants and Receiver to accomplish up to two (2) days of ground penetrating radar scanning of select items on a list to be agreed upon by them. Additional consultant scanning shall require the agreement of the Civil Litigants and the Receiver. PEF access shall be done in a safe manner with MDCPD and NIST given full authority to manage personnel on site at any one time for safety and security purposes. To assist with the efficient operation of this Protocol, NIST shall provide the Receiver and Civil Litigants with the NIST Inventory of Evidence.

Participants shall have continuous access to the PEF to carry out the objectives of this Protocol, Monday – Friday from 9am – 5pm starting on February 21, 2022 through and until March 11, 2022. All Participants shall work in good faith and coordinate with each other at all times during the performance of this Protocol, including the modification of the schedule, if needed. NIST shall have the right to restrict access to ensure the safety and security of personnel on site and the integrity of the Evidence. Should NIST restrict access to any individual under such circumstances, the reasoning therefore shall be promptly communicated to all Participants. It is understood that MDCPD and NIST will also be accessing the PEF for non-invasive review and testing during this time period for their respective investigations. At no time shall any Participant handle a piece of Evidence with the intention of altering the Evidence in any way whatsoever without the express prior permission of NIST. Touching of clean surfaces of Evidence with GPR scan equipment shall be permitted, and when scanning, no debris on any surfaces of Evidence shall be disturbed. All participants agree that no Participant shall engage in any destructive testing (this includes penetration tests such as Windsor Probe) until the parties have either agreed upon a Protocol or declared an impasse and had the opportunity to seek Court intervention.

Phase Two. While the Participants investigate the PEF Evidence under Phase One, the Participants shall work in good faith to agree upon a protocol for access for the invasive and destructive testing of Evidence at the PEF (the "Phase Two Protocol").

IT IS SO AGREED.

National Institute of Standards and Technology

By:  **JASON AVERILL**
Digitally signed by JASON AVERILL
Date: 2022.02.25 13:42:41 -05'00'

Jason D. Averill, Division Chief
Materials and Structural Systems Division
Engineering Laboratory

Date: _____

Plaintiffs

By: _____
Jeff Goodman, Esq. (*pro hac vice*)
Saltz Mongeluzzi & Bendesky

Date: _____

Miami-Dade County

By: _____

Date: _____

8701 Collins Development, LLC

By: _____
Michael Thomas, Esq.
Greenberg Traurig, P.A.

Date: _____

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National Institute of Standards and Technology

By: _____
Jason D. Averill, Division Chief
Materials and Structural Systems Division
Engineering Laboratory

Date: _____

Plaintiffs

By: _____
Jeff Goodman, Esq. (*pro hac vice*)
Saltz Mongeluzzi & Bendesky

Date: _____

Miami-Dade County

JD PATTERSON
CHIEF PUBLIC SAFETY OFFICER
MIAMI-DADE COUNTY, FL

By: 
JD Patterson
Chief Public Safety Officer

Date: 2/25/2022

8701 Collins Development, LLC

By: _____
Michael Thomas, Esq.
Greenberg Traurig, P.A.

Date: _____

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National Institute of Standards and Technology

Miami-Dade County

By: _____
Jason D. Averill, Division Chief
Materials and Structural Systems Division
Engineering Laboratory


By: _____

Date: _____

Date: _____

Plaintiffs

8701 Collins Development, LLC

By:  _____
Jeff Goodman, Esq. (pro hac vice)
Saltz Mongeluzi & Bendesky

By: _____
Michael Thomas, Esq.
Greenberg Traurig, P.A.

Date: 3.8.21

Date: _____

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National Institute of Standards and Technology

Miami-Dade County

By: _____
Jason D. Averill, Division Chief
Materials and Structural Systems Division
Engineering Laboratory

By: _____

Date: _____

Date: _____

Plaintiffs

8701 Collins Development, LLC

By: _____
Jeff Goodman, Esq. (*pro hac vice*)
Saltz Mongeluzzi & Bendesky

By: 
Michael Thomas, Esq.
Greenberg Traurig, P.A.

Date: _____

Date: _____

Terra Group, LLC and Terra World Investments, LLC

John Moriarty & Associates of Florida, Inc.

By:



Michael Thomas, Esq.
Greenberg Traurig, P.A.

By:

Jonathan E. Kanov, Esq.
Marshall Dennehey

By:

Seth Schimmel, Esq.
Phelps Dunbar

Date:

3/10/22

Date:

NV5, Inc.

By:

George Truitt, Esq.
Ryan Charlson, Esq.
Cole, Scott & Kissane

Date:

Terra Group, LLC and Terra World Investments, LLC

John Moriarty & Associates of Florida, Inc.

By: _____

Michael Thomas, Esq.
Greenberg Traurig, P.A.

By: _____

Jonathan E. Kanov, Esq.
Marshall Dennehey

By: _____

Seth Schimmel, Esq.
Phelps Dunbar

Date: _____

Date: 3-8-22

NV5, Inc.

By: _____

George Truitt, Esq.
Ryan Charlson, Esq.
Cole, Scott & Kissane

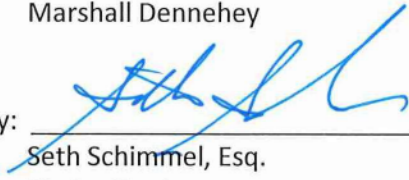
Date: _____

Terra Group, LLC and Terra World Investments, LLC

John Moriarty & Associates of Florida, Inc.

By: _____
Michael Thomas, Esq.
Greenberg Traurig, P.A.

By: _____
Jonathan E. Kanov, Esq.
Marshall Dennehey

By:  _____
Seth Schimmel, Esq.
Phelps Dunbar

Date: _____

Date: 3/4/22

NV5, Inc.

By: _____
George Truitt, Esq.
Ryan Charlson, Esq.
Cole, Scott & Kissane

Date: _____

Terra Group, LLC and Terra World Investments, LLC

John Moriarty & Associates of Florida, Inc.

By: _____
Michael Thomas, Esq.
Greenberg Traurig, P.A.

By: _____
Jonathan E. Kanov, Esq.
Marshall Dennehey

By: _____
Seth Schimmel, Esq.
Phelps Dunbar

Date: _____

Date: _____

NV5, Inc.

By:  _____
George Truitt, Esq.
Ryan Charlson, Esq.
Cole, Scott & Kissane

Date: 3/7/2022

Michael Goldberg, Esq. in his capacity as
Receiver for the Champlain Towers
South Condominium Association, Inc.

By: Brenda Radmacher
Brenda Radmacher (*pro hac vice*)
Akerman LLP

Date: March 5, 2022

Exhibit B

**IN THE CIRCUIT COURT OF THE
ELEVENTH JUDICIAL CIRCUIT
IN AND FOR MIAMI-DADE COUNTY**

**COMPLEX BUSINESS
LITIGATION DIVISION**

**IN RE: CHAMPLAIN TOWERS SOUTH
COLLAPSE LITIGATION.**

CLASS REPRESENTATION

CASE NO. 2021-015089-CA-01

**ORDER REGARDING
JOINT ACCESS PROTOCOL FOR NON-INVASIVE REVIEW AND TESTING –
PRIMARY EVIDENCE FACILITY – PHASE ONE**

THIS CAUSE came before the Court upon a series of conferences and hearings, including the December 22, 2021 evidentiary hearing, the Court’s December 30, 2021, *sua sponte* Order, the Court’s January 3, 2022 Order, and hearings on January 14, 2022, January 21, 2022, and thereafter. The Court having considered the issues and positions of the parties, it is **ORDERED AND ADJUDGED** as follows:

1. The *CHAMPLAIN TOWERS SOUTH COLLAPSE INVESTIGATION: Joint Access Protocol for Non-invasive Review and Testing – Primary Evidence Facility – PHASE ONE* (“*Primary Evidence Facility Protocol*”; attached as **Exhibit A**) is APPROVED and ADOPTED as an ORDER of the Court.¹

2. Activities at the Primary Evidence Facility shall proceed and continue as set forth in the *Primary Evidence Facility Protocol* and a PHASE TWO protocol shall be developed on the testing and sampling of the materials at the Primary Evidence Facility.

¹ Capitalized terms shall have their meaning as defined in the *Primary Evidence Facility Protocol*.

DONE and **ORDERED** in Chambers at Miami-Dade County, Florida, on this _____ day
of March, 2022.

Exhibit “B”

Exhibit “C”

IN THE CIRCUIT COURT OF THE 11TH
JUDICIAL CIRCUIT IN AND FOR
MIAMI-DADE COUNTY, FLORIDA

IN RE: CHAMPLAIN TOWERS SOUTH
COLLAPSE LITIGATION,

CLASS REPRESENTATION

CBL DIVISION

CASE NO: 2021-015089-CA-01

**DEFENDANT 8701 COLLINS DEVELOPMENT, LLC'S
REQUEST FOR ENTRY UPON LAND FOR INSPECTION**

Pursuant to Fla. R. Civ. P. 1.350(a), Defendant 8701 Collins Development, LLC ("8701 Collins Development") requests that Defendant 8701 Collins Avenue Condominium Association, Inc. (the "8701 Condo Association") provide access to property for inspection, measuring, surveying, photographing, testing, and sampling on April 18, 2022 and continuing from day to day until completion.

1. 8701 Collins Development requests access to land controlled by the 8701 Condo Association for the purpose of inspection, measuring, surveying, photographing, testing, and sampling as set forth in the Champlain Towers South Collapse Investigation: Joint Protocol for Testing and Material Sampling – Collapse Site (the "Joint Testing Protocol"). The Joint Testing Protocol was approved by the Court on January 21, 2022. Copies of the January 21 Order and the Joint Testing Protocol are attached as Composite Exhibit A.

2. The Joint Testing Protocol calls for testing to be performed in a beach access easement located over this property (the "Beach Access Area"). The Beach Access Area is situated at the northern edge of the property owned by the 8701 Condo Association and stretches from Collins Avenue eastward to the beach. The Joint Testing Protocol identifies, in Appendix B, the nature and the specific locations of the testing.

3. Testing pursuant to the Joint Testing Protocol in the Beach Access Area controlled by the 8701 Condo Association is scheduled to begin on Monday, April 18, 2022 and to continue from day to day until it is completed.

4. Accordingly, 8701 Collins Development requests access to the Beach Access Area beginning on April 15, 2022, and continuing until completion of the testing. This access shall extend to all individuals and entities necessary to complete the required testing set forth in the Joint Testing Protocol.

Dated: April 14, 2022

Respectfully submitted,

GREENBERG TRAURIG, P.A.
*Counsel for 8701 Collins Development, LLC
Terra Group, LLC and
Terra World Investments, LLC*
333 S.E. 2nd Avenue, Suite 4400
Miami, Florida 33131-3238
Telephone No.: (305) 579-0506
Facsimile No.: (305) 579-0717
Email: thomasmic@gtlaw.com
Email: barnettch@gtlaw.com

By: Michael J. Thomas
MICHAEL J. THOMAS
Florida Bar No.: 21309

CERTIFICATE OF SERVICE

I CERTIFY that a true and correct copy of the foregoing was furnished, either via transmission of Notices of Service of Court Document generated by the E-Portal or in some other authorized manner for those counsel or parties who are excused from e-mail service on this 14th day of April 2022.

/s/ Michael J. Thomas
MICHAEL J. THOMAS

EXHIBIT “A”

CHAMPLAIN TOWERS SOUTH COLLAPSE INVESTIGATION:
Joint Protocol for Testing and Material Sampling – Collapse Site

In accordance with the Court Order dated September 1, 2021, this document presents the protocol for testing and material sampling at the collapse site (the “Protocol”) and has been developed considering input from involved Participants.

DEFINITIONS

- **Consultant** – Geosyntec Consultants, Inc., an independent consultant approved by the Receiver to coordinate, plan, oversee, and provide factual reporting as defined in this Protocol to the Experts/Participants. The Consultant will employ Testing Agencies and Contractors to meet the goals of the Protocol. Payment to Consultant for its fees and costs shall be paid by all Participants as defined in the "Payment of Costs" section below.
- **Contractor** – Company, or companies, approved by the Receiver to engage in providing construction, samples, borings, security, or other support for execution of the Protocol. The Contractors shall be engaged by the Consultant. Payment for services shall be remitted to Consultant for payment to Contractors. Payment for Contractors' services shall be paid by all Participants as defined in the "Payment of Costs" section below.
- **Court** – Circuit Court of the 11th Judicial Circuit in and for Miami-Dade County, Florida.
- **Expert** – Engineers (or other disciplines) investigating the collapse on behalf of Participants.
- **Litigation** -- *In Re: Champlain Towers South Collapse Litigation*, Case No. 2021-015089-CA-01, pending in the Eleventh Judicial Circuit Court in and for Miami-Dade County, Florida
- **Participant** – A party or entity authorized or defined by the *Order Approving Protocol for Inspection, Documentation, and Storage of Components, Remnants, and Debris of The Champlain Towers South Collapse*. Notwithstanding anything to contrary herein, non-parties to the Litigation are not Participants; *however*, such non-parties may participate in the testing and investigations set forth in this Protocol if authorized to do so by the Court.
- **Protocol** – This document, titled *CHAMPLAIN TOWERS SOUTH COLLAPSE INVESTIGATION: Proposed Joint Protocol for Testing and Material Sampling – Collapse Site*.
- **Receiver** – Michael I. Goldberg, Esq., on behalf of the Champlain Towers South Condominium Association, Inc.
- **Testing Agency** – An independent company, approved by the Receiver and working under the direction of the Consultant and/or Receiver to facilitate surveying, sampling, borings, materials testing, and report factual results. The Testing Agency will use Contractors as necessary to carry out the Protocol. The Testing Agencies shall be engaged by the Consultant. Payment to Testing Agencies for their services shall be paid by all Participants as defined in the "Payment of Costs" section below.

GENERAL

Scope. The Protocol applies to the Champlain Towers South site. A separate or amended protocol for testing and materials sampling at the primary evidence facility will be developed later.

Objective. The Protocol is intended to provide reliable *factual* information needed to support the collapse investigations by the Experts, including but not necessarily limited to the following:

- Mechanical, physical, and chemical characteristics of construction materials
- Subsurface conditions, including soils, rocks, water, pile caps, and piles

- Physical configuration of structural and waterproofing components, including the plaza deck, perimeter foundation walls, shear walls, columns, and basement slab.

Reporting. It is envisioned that the Consultant, who does not represent Participants, will obtain this factual information. The factual information will be shared among Participants. Except as noted below, reporting to Participants shall include factual information only (without interpretation or opinion), including the following:

- Testing standards and methodology
- Documentation of the locations and extraction of samples for materials testing
- Test results in a format usable for engineering analysis and interpretation
- Given that geophysical testing requires interpretation to prepare result reports, one report for each geophysical test shall be prepared for the benefit of all parties. Geophysical consultants shall prepare a written narrative that shows the basis for any interpretation in each report.

Project Updates. The Consultant will conduct daily meetings at the end of each day at the site to discuss the work completed on that day and the work anticipated for the following day. Additionally, the Consultant will conduct weekly virtual meetings to discuss the work completed during that week and the multi-week look-ahead planned work. The Consultant will issue project updates weekly. The project updates will include:

- Field testing/sampling
 - Anticipated start dates and testing duration
 - At least one week of notice prior to commencement of testing
- Laboratory testing
 - Anticipated start dates and testing duration
 - At least one week of notice prior to commencement of testing
- Issues/deviations encountered at site
- Daily site visit reports (to be uploaded to a secure Extranet)

Photos and videotaping of site sampling operations; videotaping of laboratory testing shall be conducted and agreed upon for each laboratory test.

Extranet. The Consultant will provide data, photos, videos, and other factual information on a secure FTP/Sharepoint site (or equivalent).

Access to Site. The site is located at 8777 Collins Ave. in Surfside, Florida. The entrance and security check-in trailer are in the northeast corner of the site. Participants, Experts, Testing Agencies, Contractors, and other site visitors seeking access shall comply with the following:

- Submit all requests for access to the site through the Receiver.
- Notify the Receiver at least two (2) business days before the planned site visit.
- Sign a waiver releasing the Receiver of liability and provide proof of liability insurance coverage, naming the Receiver as additional insured.
- Comply with established procedures for signing in and signing out. These procedures include but are not limited to surrendering driver's licenses and wearing wristbands during each visit.

Observation of Site Sampling and Testing. Participants and Experts shall be permitted to observe sampling and testing conducted at the site and laboratories as agreed upon by all Participants prior to testing. For laboratory testing, virtual observation of testing will be provided and the ability to comment on test procedure for the first of any certain type of test, Participants shall be permitted to visit a lab and observe the first test. The first test of each test type shall also be videorecorded and promptly shared with the Participants for review and comments. The entirety of any test performed at a laboratory on a sample or material must be recorded by video if the sample or material either (i) cannot be reconstituted or retested, or (ii) will be altered by reason of the performance of the

test. Site testing can be observed in person, and any objections to the methodology can be raised on site, during daily meetings, and/or weekly virtual meetings.

Safety. All site visitors shall comply with state and federal safety regulations and the site safety protocol established by the Consultant. The Receiver may prohibit an individual from accessing the site in the event such individual fails to adhere to state and federal safety regulations and the site safety protocol established by the Receiver.

Samples. Except as permitted herein or expressly approved by the Receiver for additional testing, samples of any materials shall not be taken or removed from the site.

Reserved/Backup Samples. In addition to samples designated for testing under the Protocol, additional samples shall be taken and reserved for additional testing to address failed tests, ambiguous results, or disputed results. These samples shall be identified in the Testing Plans.

Testing Plans. Consultant and its Contractors and testing agencies shall prepare a detailed testing plan of each field and laboratory test to be performed meeting the requirements of this Protocol. The testing plans shall be shared for review by all Participants prior to executing the work.

If consensus is not reached, any dissenting Participant may bring the disagreement to the Receiver's attention for resolution. If a resolution is not achieved, counsel for the Participant may choose to bring the disagreement to the Court for resolution.

Chain of Custody. All sample extraction and preparation in the field will be documented by photo and videotaping as agreed upon by the Participants. Samples will be extracted, protected per appropriate testing standards, and labeled. A chain of custody form will be issued for each sample/specimen. Samples will be placed in appropriate containers for the intended laboratory testing. After testing, the samples will be stored at a secure location (TBD) and preserved for future testing or observation.

Additional or Specialized Testing. Each Participant may, if they choose, spend additional days on site after the joint testing is complete to take additional samples for private testing and analysis by a laboratory of their choosing or to conduct additional testing on site. The Consultant will notify the Participants when the sampling for the Protocol is complete and such additional testing can begin. If the services of the Consultant are needed for additional or specialized testing, Participants will submit additional or specialized testing requests in writing to the Consultant. The Consultant will coordinate sampling and scheduling of samples. Any such additional samples taken or testing conducted over the additional days on site shall occur immediately after work performed under this Protocol. The Consultant/Contractor will remain on site following the conclusion of this Protocol for any Participant that wishes to take additional samples. Any additional or specialized testing, or any portion of it, may be completed by the Participant's own consultants and contractors without the participation of the Consultant (1) should the Participant so choose at its discretion, or (2) should the Consultant and its contractors lack the capacity, expertise or preparation to accomplish the additional testing. The Receiver shall be consulted for the scheduling of any additional or specialized testing and the Receiver shall make the site available for such additional or specialized testing coordinated with the Consultant and the Receiver. If any non-party Participant wishes to conduct additional testing, leave of Court is required. All Participants may observe additional specialized testing at the site.

For geotechnical testing using the Consultant's services, any Participant that chooses to remain on site following the conclusion of this Protocol for the purpose of collecting additional samples will be limited to additional borings at locations to be identified and provided in advance to the Consultant to be conducted within the agreed upon period for additional testing. The location(s) where any additional samples are taken from will be appropriately documented by the Consultant. For concrete and rebar sampling, the Participants shall be authorized to take companion samples of material taken under this Protocol during the agreed upon period for additional testing or as required by the Consultant or, depending upon the sequencing of work, while joint geotechnical testing is underway provided that the taking of additional concrete and rebar sampling will not interfere with other work on site.

The Receiver will permit Non-Destructive Testing to be coordinated and proceed before sampling.

Payment of Costs. Except for the costs attributable to additional or specialized testing, which costs are the sole responsibility of the Participant seeking such additional or specialized testing, each Participant's or other third-party's allocation of the costs incurred through performance of this Protocol shall be determined pursuant to an order from the Court.

Audio and Video Recording. Some or all of the work performed pursuant to this Protocol will be recorded via video and/or audio. Although the purpose of the video and/or audio recordings is to document and record the work performed pursuant to this Protocol, it is understood that said video and/or audio recordings may also capture conversations or statements made by on-site Participants and/or the Consultant. It is expressly agreed between the Participants that any conversations or statements that are captured and/or recorded in the video and/or audio recordings of the work performed pursuant to this Protocol shall not be admissible at trial for any purpose and may not be used by any Participant in this litigation or any other legal or administrative proceeding, including but not limited to at depositions, in hearings, or in any filings.

Water Control. Until sampling and site testing is complete, the Receiver will continue to arrange for removal of excess water that accumulates on the lower-level slab from leakage and rain. The Consultant shall implement a comprehensive dewatering program throughout the entirety of the slab and as otherwise as necessary throughout the site.

General. Nothing in this Protocol shall serve to confirm authorization or access to conduct inspections or testing on any property or portion thereof that is not under the ownership and control of the Champlain Towers South Condominium Association (in the "CTS Property"). There may be several boring, test pits and other tests anticipated under this Protocol that are not located on the CTS Property and are subject to access authorized by the property owner and/or Court approval.

TESTING AGENCIES AND CONTRACTORS

Testing Agencies and Contractors extracting samples shall be named (including credentials) by the Consultant and selected by consensus among the Participants for the testing scope listed in Table 2. Laboratories performing the work shall be ISO 17025, AASHTO R 18 or USACE accredited (or acceptable equivalent) for the tests to be performed. Persons performing the work shall have documentation/training to perform the specific tests and/or tasks, (e.g., ACI certified laboratory technician performing compressive strength tests on concrete). All testing shall, at a minimum, meet the industry standard of care. The Consultant will coordinate the extraction and delivery of samples to Testing Agencies and collect and deliver testing reports to the Receiver. A Participant may request permission from the Receiver to visit (in person or virtually) one or more testing agency to examine samples and view testing procedures. Such visits will be coordinated by the Receiver and Consultant.

Table 1. Testing Agencies and Contractors

Specialty	Testing Scope	Testing Agency/Contractor [website]
Structural	Demolition	To be determined
	Coring	To be determined
	Surveying	To be determined
Surveyor	Surveying	To be determined
Laboratory Testing	Concrete	To be determined
	Metals	To be determined
Petrography	Petrography	To be determined
Non-Destructive Testing	Ground penetrating radar	To be determined
Geophysics	MASW/ER	To be determined
	Seismograph	To be determined

Specialty	Testing Scope	Testing Agency/Contractor [website]
Geotechnical	Borings and observation wells	To be determined
	Test pits	To be determined
	Pile integrity testing (PIT)	To be determined
	Crosshole seismic testing	To be determined
	Laboratory testing	To be determined
	Seismic CPT testing	To be determined
	Parallel seismic testing for deep foundations and sheet piles	To be determined

TESTING METHODS AND STANDARDS

The Consultant, Testing Agencies, and Contractors shall adhere to the following standards for tests carried out. Proposed deviations from the standards shall be brought to the attention of all Participants and Experts through the Receiver. In general, non-destructive testing will occur before sampling. In addition, the following should be noted:

- General descriptions of the test methods are provided in Appendix A.
- Proposed testing locations are provided in Appendix B.

Table 2. Testing Standards

Category	Test	Destructive	Standard/Guide
Concrete Materials	C1 Compressive strength	Yes	ASTM C39 and C42
	C2 Elastic modulus and Poisson's ratio	Yes	ASTM C469
	C3 Splitting tensile strength	Yes	ASTM C496
	C4 Air void parameters	Yes	ASTM C457
	C5 Petrography*	Yes	ASTM C856 and C1723
	C6 Chloride profile	Yes	ASTM C1218 and C1152
	C7 Ionic Species (e.g., sulphur, magnesium, sodium, and potassium) profile	Yes	ASTM C114
Reinforcing Steel	R1 Reinforcing bar location survey using ground penetrating radar (GPR)	No	ASTM D6432 and ACI 228.2R-13
	R2 Tensile properties	Yes	ASTM A615 and ASTM A370
	R3 Rockwell hardness	Yes	ASTM E18
	R4 Chemistry	Yes	ASTM E415, ASTM A751
	R5 Section Loss / Pitting	Yes	ASTM G46, ACI 364.14T-17
Geotechnical	G1 Soil/rock borings**	Yes	ASTM D1586, FDOT Soils and Foundation Handbook (as applicable) Participants request including pitcher samplers (ASTM D7015, ASTM D1587), X-ray of undisturbed samples (ASTM D4452), rock coring with inner spring barrel (ASTM D2113), RQD (D6032). Soil/rock characterization (ASTM D2487, D2488, and D5878). Permeability tests (ASTM D5912)
	G2 Observation well	Yes	ASTM D5092; ASTM D5912.
	G3 Test pit excavation	Yes	N/A
	G4 Pile integrity testing (PIT)	Yes	ASTM D5882
	G5 Crosshole seismic testing	No***	ASTM D4428
	G6 Grain size analysis	Yes	ASTM D6913, ASTM D7928
	G7 Moisture content	Yes	ASTM D2216
	G8 Organic content	Yes	ASTM D2974
	G9 Incremental consolidation	Yes	ASTM D2435
	G10 Chlorides	Yes	AASHTO T291
	G11 Electrical resistivity	Yes	ASTM G57
	G12 pH	Yes	ASTM D4972 or G51
	G13 Sulfates	Yes	ASTM D516
	G14 Sulfides	Yes	SM4500
	G15 Unconfined compression of rock	Yes	ASTM D7012C
	G16 Unit weight of Rock Cores	Yes	ASTM D7263
	G17 Specific Gravity	Yes	ASTM D854, ASTM D5778
	G18 Seismic CPT	Yes	ASTM D5778
	G19 Parallel Seismic Testing	Yes	ACI 228.2R and ASTM D8381
	G20 Drained and Undrained Triaxial Testing of Fine-Grained Soil Samples	Yes	ASTM D7181, ASTM D4767, ASTM D2850
	G21 Seismograph	No	N/A

Category	Test	Destructive	Standard/Guide
	G22 MASW/ER	No	NA
* Due to the nature of Petrographic testing samples will be provided to labs as requested by Participants and available			
**Includes SPT, and obtaining and testing of soil and rock samples			
***Uses borings; however, the test is non-destructive			

CONCRETE SAMPLING AND TESTING PLAN

The table below lists tests of concrete core samples. Final locations are to be determined by the consensus of the Experts and depend on site conditions and locations of interest and shall be provided to the Receiver at least seven (7) days prior to the commencement of the concrete core sampling.

Table 3. Concrete Sampling and Testing Plan

Core Sample Phase/Location	Element	Original Element Location	Number of Cores***	Test Type and Number (See Table 2)				
				C1 Comp.	C2 E & v	C3 Tens.	C4 & C5* Voids/Petro	C6 & C7 Cl & Ionic Species
Site	Slab	Basement	33	13	3**	3	3	3
Site	Perimeter Wall	Basement	33	13	3**	3	3	3
Site	Pile Cap	Below Basement Slab	22	6	4	4	4	0
Site	Pile	Below Pile Cap	22	6	4	4	4	0
Site	Pool Deck Slab (remaining area at SE corner of site, including all layers (i.e., structural slab, topping, waterproofing, pavers)	Pool Deck at Lobby Floor	8	3	1**	2	1	1
Site	Shear Walls	Gridline M at Basement and West Core	12	6	2**	0	2	0
Site	Columns (rubble within extant rebar)	Various columns on gridlines E, L, M, O, and Q	15	0	0	0	10	0
Site	Beams	Between Gridlines N-O and 8-10	9	6	2	0	2	0
Total***			154	53	19	16	29	7

*Tests C4 and C5 will be conducted on the same core sample; separate core samples are needed for all other tests.

**One of the three cores used for compressive strength will also be used to determine modulus of elasticity and Poisson's ratio.

***Collect spare samples at each location for backup/additional testing (Consultant/Testing Agency to provide plan)

REINFORCING STEEL SAMPLING AND TESTING PLAN

Table 4 lists the general location and number of samples. Final locations are to be determined by the consensus of the Experts and depend on site conditions and locations of interest and shall be provided to the Receiver at least seven (7) days prior to the commencement of the steel sampling.

Table 4. Reinforcement Sampling and Testing Plan

Bar Sample Location	Element	Original Element Location	Bar Size	Number of Bars*****	Test Type and Number 8 (See Table 2)				
					R1* GPR	R2** Tens.	R3** Hard.	R4** Chem.	R5 Sect. Loss
Site	Slab	Basement	#5	6	4	6	3	1	6
Site	Wall	Basement	#5	23	4	23	23	4	23
Site	Column	Basement	N/A	N/A	2	0	0	0	0
Site	East Shear Wall	Basement	#10/#11	0	N/A	0	0	0	0
Site	Slab	Pool Deck	#4/#5****	16	0	16	16	4	16
Site	South Wall	Pool Deck	#5*****	10	0	0	0	0	10
Site	Columns (rebar protruding through slab)	Type N Columns***	#7	3	N/A	3	3	1	3
Site	Columns (rebar protruding through slab)	Various Type A and L Columns	#10	3	N/A	3	3	1	3
Site	Columns (rebar protruding through slab)	Various Type C, D, and G Columns	#11	3	N/A	3	3	1	3
Site	Column	Various Ties from Columns	#3/#4/#5	3	N/A	3	3	1	3
Total				67	10	57	54	13	67

*Non-Destructive Testing (approximately 10 ft x 10 ft area)

**Tests R1, R2 and R3 to be conducted on the same bar sample

***No. 7 rebar remaining in the slab at columns that supported the pool deck will be over-cored and removed for inspection only.

****Two bars top and bottom bars in each direction

*****Exposed Vertical Bars, perform fractography examination of exposed bar ends.

*****Collect spare samples at each location for backup/additional testing (Consultant/Testing Agency to provide plan)

ADDITIONAL STRUCTURAL INSPECTION/SAMPLING PLAN

The following inspections and samples shall be taken by the Consultant and examined on site or in the lab as applicable:

1. Plaza pool deck investigative work, including five (5) exploratory borings in the overburden, examination, field testing, and sampling of the waterproofing for laboratory testing. NOTE: This waterproofing sampling, field testing, and exploratory work must happen before destructive work on the underlying structure.
2. Pool deck slab N/S construction joint probe:
 - a. Provide additional pool deck slab shoring as required;
 - b. Shore top edge of adjoining privacy wall against lateral out-of-plane movement as required;
 - c. Locate all bars passing through construction joint along a 4 ft length of construction joint;
 - d. In the presence of all interested parties and under video observation, sawcut and remove a 4 ft (north-south) x 3 ft (east-west) slab of the existing pool deck slab and topping slab along the joint, taking care not to damage or drop the slab during sawcutting and removal;
 - e. Two (2) sawcut openings in the west edge of the remaining pool deck plaza to expose the cold joint assembly;
 - f. One (1) sawcut opening in the west edge of the remaining pool deck plaza to expose the slab-to-hot tub wall connection;
 - g. One (1) sawcut in the north edge of the remaining pool deck plaza to expose the slab-to-corbel connection;
 - h. Seal the preserved slab in heavy plastic and set aside for retrieval by laboratory for controlled and video-documented deconstruction; and
 - i. Measure and photo document the remaining sawcut cross section of the construction joint.
3. Privacy wall probe:
 - a. Provide additional pool deck slab shoring as required;
 - b. Shore top edge of adjoining privacy wall against lateral out-of-plan movement as required;
 - c. In the location where the pool deck slab is remaining west of the pool, GPR a 4 ft wide section (wider if necessary) of the CMU wall to locate the wall vertical bar(s) and dowels,
 - d. Make two vertical sawcuts, 4 ft apart and capturing the location of the vertical bar(s), to isolate CMU wall to be inspected/demolished from remainder of privacy wall;
 - e. In the presence of all interested parties and under video observation, sawcut and remove a 3 ft (east-west) x 3 ft (tall) portion of the existing wall, taking care not to damage or drop the wall during sawcutting and removal;
 - f. Seal the preserved wall portion in heavy plastic and set aside for retrieval by laboratory for controlled and video-documented deconstruction; and
 - g. Measure and photo document the remaining sawcut cross section of the wall.
4. South foundation wall to pool deck slab joint probe:
 - a. Provide additional pool deck slab shoring as required;
 - b. Shore top edge of adjoining privacy wall against lateral out-of-plan movement as required;
 - c. Within the 4 ft wide privacy wall probe, GPR a 3 ft x 3 ft area of the pool deck slab and a 3 ft x 3 ft area of the supporting wall to locate the rebar;
 - d. In the presence of all interested parties and under video surveillance, sawcut a 3 ft (north-south) x 3 ft (east-west) slab of the existing pool deck slab and topping slab along the joint, taking care not to damage or drop the slab during sawcutting;
 - e. After sawcutting through the slab, in the presence of all interested parties and under video observation, continue to sawcut the supporting wall and remove the L-shaped portion of the slab-to-wall connection as shown in page 6 of the attached figures;
 - f. Seal the preserved wall portion in heavy plastic and set aside for retrieval by laboratory for controlled and video-documented deconstruction;
 - g. Measure and photo document the remaining sawcut cross section of the slab and wall;
 - h. Provide six (6) test pits (as shown on SK-03) to expose the beach access walkway assembly and the top of the south perimeter sheet pile wall driven for the construction of 8777 Collins Ave. Note,

- these are not test pits to the bottom of the foundation wall;
- i. Provide six (6) exploratory openings in the exposed interior face of the southern perimeter basement wall that align with the test pits in item (h). Exploratory openings shall expose the sheet pile wall and embedded steel reinforcement; and
 - j. Visually survey and use a carpenter/mason's hammer sound the exposed interior face of the wall to map cracks and detect delamination as described in ASTM D4580. At delaminated locations perform selective demolition of the concrete perimeter wall from the basement interior to expose the sheet pile wall and embedded steel reinforcement. Remove one (1) sample of embedded reinforcement steel and sheet pile wall at each opening location.
5. Measure the depth of the foundation wall sheet piling at two locations along the south wall using parallel seismic testing (cross-reference test G19);
 6. Survey top edge geometry of sheet piling at 20 ft on center maximum, capturing all abrupt elevation changes (absolute elevation or elevation relative to bottom of pool deck slab):
 - a. Where required, chip the concrete to expose the top of steel sheet pile;
 - b. Optically survey the top of the sheet piles at 20 ft on center maximum, capturing all abrupt elevation changes;
 - c. Optically survey the elevation of the bottom of pool deck slab near the pool;
 - d. Reference the surveyed elevations to a datum.
 7. Take 1 ft x 1 ft samples of sheet pile at two locations along the south wall for measuring thickness, material properties, and corrosion quantity and quality (uniform loss or pitting);
 8. Chip concrete at each shear wall boundary zone to expose boundary element ties for observation and documentation by all interested parties;
 9. Provide test pit at undocumented column at grid O/8.5;
 10. Provide two (2) exploratory openings to expose the top of the west perimeter sheet pile wall driven for the construction of 8777 Collins Ave;
 11. Provide two (2) exploratory openings to expose the top of the north perimeter sheet pile wall driven for the construction of 8777 Collins Ave;
 12. Provide two (2) exploratory openings to expose the top of the east perimeter sheet pile wall driven for the construction of 8777 Collins Ave;
 13. Provide two (2) exploratory openings to expose the slab-to-wall connection at the bottom of the south perimeter basement wall.
 14. Provide one (1) exploratory opening to expose the slab-to-wall connection at the bottom of the north, west and east perimeter basement wall; and
 15. Perform level survey of the basement slab on grade capturing slab elevation at all column locations, adjoining location at the perimeter wall as applicable, shear walls, and the midspans of bays. In addition, the surveyed elevations to a datum must be referenced.

GEOTECHNICAL INVESTIGATION PLAN

The following geotechnical plan is developed as a basis for understanding site conditions and the foundation system. The test details and standards are to be developed based on the Experts' input and the Consultant engaged in carrying out the plan. The table lists general locations of borings and tests/samples. Final locations are to be determined by consensus of the Participants and will depend on site conditions and locations of interest. Details on sampling within the borings are shown in the attached plan (Appendix B) and described in section *G1 Soil/Rock Borings* (Appendix A).

The locations and numbers of tests for G6-G17 will be recommended by the Experts in consultation with the Consultant and approved by the Receiver and/or the Court. Note: Nothing in this Protocol shall serve to confirm authorization or access to conduct inspections or testing on any property or portion thereof that is not CTS Property. There may be several boring and test pits anticipated under this Protocol that are not located on the CTS Property and are subject to access authorized by the property owner and/or Court approval. The Protocol schedule shall be adjusted if circumstances outside of the Receiver's, the Consultant's, or the Participants' control, such as, but not limited to, permitting delays, access to adjoining properties, contractors' availability, equipment and material shortages, damage to samples collected, weather events, or COVID-related issues, materially impact the schedule.

Table 5. Geotechnical Sampling and Testing Plan – Receiver to coordinate any request for additional testing by site purchaser

Sample/Test Location	Test Type and Number*** (See Table 2)						
	G1 Borings	G2 Obs. Wells	G3* Test Pits	G4 PIT	G5 Crosshole Seis.	G18 CPT	G19 PSL
Slab Corners	0	0	0	0	0	0	0
Within Building Footprint	33	6	23	22	5	12	0
Outside Building Footprint	7	1	5	5	0	2	4
Total	40	7	28	27	5	14	4

*Core drill through pile cap and full length of pile

**Non-Destructive Testing

SCHEDULING AND ESTIMATED TIME REQUIRED ONSITE

The table below shows the estimated start date and time on site required for sampling and testing.

Table 6. Estimated testing start date and time required onsite

Sampling/Test	Estimated Start Date	Estimated Time Required on Site
Concrete Sampling	TBD	TBD
Steel Sampling	TBD	TBD
Concrete/Steel Laboratory Testing	TBD	TBD
G1 Soil Borings, G2 Observation Wells, G5 Crosshole Seismic, G18 Seismic CPT, G20	TBD	TBD

G3 Test Pits, G4 Pile Integrity Testing, G19 Parallel Seismic	TBD	TBD
G6-G17,G20 Laboratory Testing	TBD	TBD
G21, G22 MASW/ER, Seismographs	TBD	TBD

APPENDIX A

This appendix is intended to provide a brief description and outline requirements for each testing method in the Protocol. These descriptions should not be considered comprehensive and Participants should consult with their Experts for further information. Note: for all testing, the most recent and applicable testing standards and guides are to be used.

SITE SAFETY

The safety and wellbeing of all individuals on site, as well as the safety and wellbeing of those that may be impacted by the work performed under this Protocol shall at all times be the first and foremost priority of all individuals and companies involved in the performance of work. All work shall be performed in accordance with all local, state and federal rules and safety regulations and guidelines, including OSHA. Consultant will develop a site safety plan for the work which anyone entering the site must accept and adhere to. If anything in this Protocol calls for work to be done in a manner deemed unsafe by the Consultant or any Contractor, then said Consultant or Contractor shall not proceed with the work and all parties shall discuss how to proceed more safely. The Participants shall not be responsible for the implementation for any safety program nor have any responsibility for site safety, with the exception of making sure that they themselves comply with the safety program implemented by the Consultant. The Consultant may request that Receiver expel a Contractor or Participant that violates the safety plan.

CONCRETE TESTING

Concrete Core Removal

All core samples shall be taken using a 4-inch diameter core barrel (core diameter = \pm 3.7-inch) or largest barrel suitable to fit between bars at a specific location. Each sample shall be marked, measured (diameter and overall length), described, and photographed, including the size and condition of any embedded reinforcement.

Core samples shall be taken and conditioned per ASTM C42. Before coring, the surface shall be sounded with a hammer as described by ASTM D4580, and reinforcing bars in the vicinity shall be located using ground penetrating radar (GPR) or pachymeter. Cores for petrographic examination will be taken through near-surface reinforcing bars, especially where delamination planes have been identified by sounding. Otherwise, coring operations should avoid reinforcement. Core holes shall be inspected to determine if any cracks in the core samples are also present in the concrete. Cores intended for compression testing or splitting tensile testing shall be retaken if the cores include reinforcement, large voids, or cracks that preclude obtaining a 6-inch long sawn sample without such characteristics. After cores have been drilled, wipe off surface drill water and allow remaining surface moisture to evaporate. When surfaces appear dry, but not later than 1 hour after drilling, place cores in separate plastic bags or nonabsorbent containers, seal to prevent moisture loss, and label for identification.

Core holes through the Level 1 slab and basement wall shall be patched as required to prevent groundwater entry. Table 4 lists the general location and number of cores. Concrete core samples will be extracted with a diamond impregnated, water-cooled core drill. After removal, the core holes will be photographed and then patched with a rapid-setting, cementitious repair material. The condition and location of each core will be documented with photographs and notes.

C1 Compressive Strength

To determine concrete compressive strength, concrete cores shall be removed using a coring drill, trimmed, prepared for testing using sulfur capping, and loaded in a loading frame at a rate within a specified range. The compressive strength of the specimen will be calculated by dividing the maximum axial load sustained by the sample by the cross-sectional area of the specimen. Concrete core samples will be prepared and tested according to ASTM

C42 *Standard Test Method for Obtaining and Testing Drilled Cores and Sawed Beams of Concrete* and ASTM C39 *Standard Test Method for Compressive Strength of Cylindrical Concrete Specimens*

C2 Elastic Modulus and Poisson's Ratio

These parameters will be measured using appropriate instrumentation in a loading frame during compressive loading of the specimens. Elastic modulus and Poisson's ratio will be measured according to ASTM C469 *Standard Test Method for Static Modulus of Elasticity and Poisson's Ratio of Concrete in Compression*.

C3 Splitting Tensile Strength

Splitting tensile strength testing provides measurements to calculate the concrete tensile strength of the tested specimen. This test method consists of applying a diametral compressive force at a rate within a prescribed range along the length of a concrete core specimen in a load frame until failure occurs. The maximum load sustained by the specimen is divided by appropriate geometrical factors to obtain the splitting tensile strength. Splitting tensile strength will be measured in accordance with ASTM C496 *Standard Test Method for Splitting Tensile Strength of Cylindrical Concrete Specimens*.

C4 Air Void Parameters

Determining air void parameters involves examining a concrete cross-section under a microscope. One of three procedures will be used to determine air void parameters, including specific surface, void frequency, spacing factor, and paste air ratio of the air-void system in hardened concrete:

1. Linear-traverse method
2. Modified point-count method
3. Contrast-enhanced method

Air void parameters will be determined in accordance with ASTM C457 *Standard Test Method for Microscopical Determination of Parameters of the Air-Void System in Hardened Concrete*.

C5 Petrography

A petrographic examination consists of a series of observations of hardened samples that are interpreted to draw conclusions about the composition, quality, water to cement ratio, and probable cause(s) of potential problems associated with concrete or other construction materials. To conduct a petrographic examination, a core sample will generally be sliced in half lengthwise. One of the resultant halves will be lapped (polished) to accentuate the appearance of the components of the concrete (air voids, aggregates, and cement paste). Additional thin sections and smaller polished sections can also be made for petrographic microscope and scanning electron microscope (SEM) evaluations of cement paste and including microscopic chemical analysis.

A powder mount is obtained by scratching off a very small portion of the paste and mounting the resultant powder on a glass microscope slide in immersion liquid. Examination of a powder mount allows the properties of the cement paste to be studied in detail. Petrographic examination will be carried out in accordance with ASTM C856 *Standard Practice for Petrographic Examination of the Hardened Concrete* and ASTM C1723 *Standard Guide for Examination of Hardened Concrete Using Scanning Electron Microscopy*.

All samples and epoxied discs shall be preserved for review by other experts if needed and access granted when requested. The petrographic report shall include descriptions of the method(s) of preparation of the samples and full documentation of the images and other aspects of the samples that the lab personnel have based their interpretations on. The preparation and cutting of the samples by the lab technicians shall be video recorded.

C6 Chloride Profile

Chloride ion content testing will be performed on selected core samples to determine whether chloride contamination from salts has accumulated to sufficient levels to promote corrosion of reinforcing steel. Selected core samples will be cut into slices at pre-determined depths, and the slices will be pulverized to facilitate chloride

content analysis in general conformance with ASTM C1152 *Test Method for Acid-Soluble Chloride in Mortar and Concrete* and ASTM C1218 *Standard Test Method for Water-Soluble Chloride in Mortar and Concrete*. The acid-soluble chloride content represents both water-soluble and chemically-bound chloride within the cement paste and any chloride that may be present in the aggregate. The water-soluble chloride content represents only the chlorides in the cement paste and aggregates that are soluble in water. Slices will be taken at depth prescribed by ASTM C1556 *Standard Test Method for Determining the Apparent Chloride Diffusion Coefficient of Cementitious Mixtures by Bulk Diffusion*, Table 1. Table 1 in C1556 describes the depth at which slices should be taken given a known water to cement ratio. Thus, the water to cement ratio should be determined from petrographic analysis and then used to determine the number of slices to be analyzed chloride and other ionic species.

C7 Ionic Species Profile

The same powder used for analysis of chloride content by ASTM C1218 and ASTM C1152 can be used to determine the content of sulfur, magnesium, sodium, and potassium found in the concrete by utilizing testing methods described in ASTM C114 *Standard Test Methods for Chemical Analysis of Hydraulic Cement*. In fact, C1152 and C1218 rely on the method for chloride analysis in C114. As these elements are part of the ocean salts or cement paste, changes in depth and concrete location can be useful in pinpointing deterioration mechanisms.

REINFORCING STEEL TESTING

Reinforcing Steel Sample Removal

All reinforcing steel samples shall be extracted such that the samples are straight and undamaged by chipping hammers or other removal equipment. Deep saw cutting along and perpendicular to the bar is envisioned. Samples shall be at least 36 inches long. Each extracted sample shall be measured and photographed. Repair of sample locations may be necessary to prevent water intrusion.

R1 Reinforcing Bar Location Survey Using Ground Penetrating Radar (GPR) – Non-Destructive

To determine location (depth and spacing) and concrete cover of the reinforcing steel, ground penetrating radar (GPR) will be used. In addition to the non-destructive testing, limited destructive measures (e.g., drilling small ~1/2 in. diameter holes) will be used for correlation and calibration. GPR involves the use of a high-frequency radar antenna, which transmits electromagnetic radar pulses along with a discrete longitudinal scan at the surface of a structural element. Electromagnetic signals reflected from material interfaces having different dielectric properties (i.e., reinforcing steel embedded in concrete) are collected by the antennae and displayed graphically for direct interpretation in the field. Guidelines for GPR considered during this work included ACI 228.2R-13 *Report on Nondestructive Test Methods for Evaluation of Concrete in Structures* and ASTM D6432 *Standard Guide for Using the Surface Ground Penetrating Radar Method for Subsurface Investigation*.

R2 Tensile Properties

Tensile properties of reinforcing bar specimens include yield strength, ultimate tensile strength, elongation, and elastic modulus. Tensile properties will be determined by testing as-rolled bar specimens in a load frame. Testing of tensile property shall be carried out in accordance with the applicable portions of ASTM A615 *Standard Specification for Deformed and Plain Carbon-Steel Bars for Concrete Reinforcement* and ASTM A370 *Standard Test Methods and Definitions for Mechanical Testing of Steel Products*.

R3 Rockwell Hardness

The Rockwell hardness test is an indentation hardness test that involves the use of a verified machine to force a diamond spheroconical indenter or tungsten carbide (or steel) ball indenter into the surface of a material. Hardness is determined based on the size of the indentation. Rockwell hardness testing will be carried out in accordance with ASTM E18 *Standard Test Methods for Rockwell Hardness of Metallic Materials*.

R4 Chemistry

A spectrometric analysis will be used to determine the quantity of carbon, iron, and residual elements in carbon steel (copper, nickel, arsenic, lead, tin, chromium, etc.) for the determination of compliance with compositional specifications (in this case, ASTM A615). Testing will be carried out in accordance with ASTM E415 *Standard Test Method for Analysis of Carbon and Low-Alloy Steel by Spark Atomic Emission Spectrometry*.

R5 Section Loss/Pitting

The extent of section loss and pitting on samples (particularly those tested in R2) will be determined according to ASTM G46 *Standard Guide for Examination and Evaluation of Pitting Corrosion* and ACI 364.14T-17 *Section Loss Determination of Damaged or Corroded Reinforcing Steel Bars*.

GEOTECHNICAL INVESTIGATION

G1 Soil/Rock Borings

The following geotechnical investigation is proposed:

- Forty (40) borings with approximate locations as annotated in the attached Appendix B are proposed. Borings SPT-8, SPT-9, SPT-10 and SPT 14, which lie outside the perimeter of the Champlain tower walls to the south, will be performed with a smaller rig that fits in the narrow passageway.
- All borings are generally expected to extend to a rock layer at about 80 feet deep unless determined otherwise by the Participants.
- Boreholes for sampling shall be kept full of drilling fluid at all times. If needed, bentonite or suitable alternative should be added to keep the borehole full of drilling fluid and avoid borehole squeeze.
- The driller will open the SPT sampler and allow Participants to visually inspect the contents. Samples from the SPT sampler will be collected at various depths within the boreholes as directed by the Consultant and sealed in watertight containers. The drilling contractor will perform SPT testing and obtain samples at a maximum interval of every 5 feet, or as otherwise directed by the Participants. SPT samples will be collected continuously as directed by the Participants.
- Samples of the interbedded limestone/sand stratum will be obtained using Pitcher samplers in addition to split-spoon samples during the initiation of the subsurface investigation. The Consultant, in agreement with the Participants, shall evaluate and determine the need for subsequent Pitcher sampling as the subsurface investigation continues; however, pitcher sampling shall not be eliminated and shall be incorporated as a supplement in select borings to provide a comparison between split-spoon samples and pitcher samples. An alternative to the Pitcher sampler may be the use of a triple-tube core barrel to increase sample recovery and quality.
- The drilling subcontractor shall use methods and procedures to maximize core recovery. Coring shall be in accordance with FDOT standards. In addition, coring should be continuous, whether 5-ft or 10-ft core runs are used.
- Samples of soil and rock cores will be retrieved and shipped for further laboratory testing at a mutually agreed upon geotechnical testing laboratory. The Consultant will develop a protocol for adequate sample preservation and shipping.
- All Participants will be allowed to visually observe the joint investigation. Participants will not direct the drilling contractor for the forty (40) joint borings. All soil and rock samples will be allowed to be visually evaluated by all Participants on-site during the investigation.

The forty (40) boring locations are to be drilled by the same mutually agreed upon drilling contractor who will work with the Consultant to observe, log samples, and provide investigation reports. The drilling contractor shall provide sample boring and drilling logs and list of items to be included in the visual description for review by Participants. The Consultant will provide a sample boring log also for review by Participants. The Consultant will provide reports including, but not limited to, visual-manual soil identification per ASTM D2487 and D2488, SPT blow counts, RQD

and water level observations. The same contractor shall remain on site after the completion of the 40 borings so each Participant can perform additional tests, if so requested, for an agreed upon duration of time. The testing performed by each Participant after the first 40 borings can be visually observed by all Participants; however, the samples extracted will belong to the specific Participant coordinating the testing and can be destructively tested as needed.

In the standard penetration test (SPT) procedure, a 2-inch outer diameter split-barrel sampler is driven into the soil by repeatedly dropping a 140-pound hammer from a height of 30 inches. The length of the samplers can be 18 or 24 inches. The number of blows required to advance the sampler every 6 inches are recorded. The sum of the second and third blows is recorded as the SPT N values for that depth. SPT N values are utilized to correlate strength parameters for the soils. Accompanying energy measurements will be reported. Soil and rock borings shall be carried out in accordance with ASTM D1586 *Standard Test Method for Standard Penetration Test (SPT) and Split-Barrel Sampling of Soils*.

- Soil borings will be advanced using the standard penetration test (SPT) procedure described above unless otherwise directed by the Consultant's geologist or engineer. The test provides samples for identification purposes and provides a measure of penetration resistance which can be used for geotechnical design purposes. The standard penetration test recovers a highly disturbed sample, which is generally not suitable for tests that measure properties of the in-situ soil structure, such as density, strength, and consolidation characteristics. The samples will be labeled, sealed, and transported to a laboratory for further index and classification testing. If fine-grained material is encountered, thin-walled tube samples should be used for obtaining intact specimens of fine-grained soils for laboratory tests to determine engineering properties. Similarly, if soils containing cohesive materials (clays) are encountered, thin-walled tube sampling should be performed to obtain minimally-disturbed samples. Drained and undrained triaxial testing will be performed on any samples of clay to establish strength parameters.
- The Consultant should provide draft investigation logs every week or sooner of all work completed the prior week.

G2 Observation Wells and Water Quality Samples

A standpipe will be installed after the completion of specified boreholes. Observation wells should be screened in separate strata. Certain observation wells should be screened in the sand stratum above the interbedded limestone/sand, and others should be screened in the interbedded limestone/sand stratum itself. These standpipes will be monitored for groundwater levels in the subsequent days of the investigation. An electronic transducer connected to a datalogger shall be installed in each well. The groundwater should be monitored at least six times daily until it is determined by Participants that it can be stopped, to develop a record of seasonal and other variations. A barometer shall be installed at the site. Sufficient dataloggers shall be installed at the site to read all installed sensors with provisions for the data to be provided to the Participants at regular intervals. Water quality samples will be collected and sent to an approved laboratory for testing. Field permeability testing in the sand and the interbedded limestone/sand strata in general shall be performed accordance with ASTM D5912. The Participants shall identify the location of each field permeability test. The Participants shall fully develop the requirements and details for the observation wells after the Consultant obtains subsurface information from the borings.

Monitoring wells will be installed at the locations shown on Appendix B and any other location identified by the Participants to determine long-term, stabilized groundwater readings. A slotted pipe will be installed within the borehole and packed with sand on the outside to ensure the slots in the pipe are not clogged. A bentonite seal shall be established above the top of the screened zone. In addition, grout will be placed near the top of the pipe to ensure surface water does not percolate into the observation well. The top of the pipe will be capped and sealed to ensure precipitation does not enter the observation well. Observation wells will be installed and monitored in accordance with ASTM D5092 *Standard Practice for Design and Installation of Groundwater Monitoring Wells*.

Samples at 10-ft intervals within the borehole shall be obtained for the purposes of water quality sample testing. Samples shall be obtained in accordance with the FDOT Soils and Foundation Handbook for investigations for Structures and shall be tested in accordance with Section 4.10 – Environmental Corrosion Test Guidelines.

G3 Test Pit Excavation

Two (2) test pit excavations (TP-5 and TP-17) and full-length cores at the piles identified as TP-5 and TP-17 on Appendix B shall be performed prior to commencement of any borings in order to establish the lengths of the borings. The remainder of the test pits shall be performed after the borings are completed.

The structural slab and remaining column shall be extracted labeled and set aside for inspection and additional sampling if necessary. Care should be taken to remove the column and structural slab as to minimize damage.

Test pits shall be sufficiently deep to expose the pile caps and the top 2 to 3 feet of the pile. At each test pit, core drill or saw cut through the pile cap and portion of the pile to extract samples and permit pile integrity testing. Test pits shall be dewatered as required to permit observation and pile integrity testing. Contractor shall cut through concrete, remove materials and provide needed dewatering under the direction of the Consultant. At the basement wall, test pits shall be sufficiently deep to expose or extend to the elevation of the bottom of the foundation wall.

Walkway test pits shall be sufficiently deep to expose or extend to the elevation of the bottom of the foundation wall. Test pits shall be dewatered as required to permit observation. Contractor shall cut through concrete, remove materials and provide needed dewatering under the direction of the designated engineer/geologist.

The Consultant will engineer each test pit excavation for safe access. The test pit designs will be shared with all parties. OSHA standards will be complied with in relation to design, execution and access.

G4 Pile Integrity Testing (PIT)

PIT Testing is performed by placing accelerometers at several locations on the pile and then hitting the pile or as necessary to obtain conclusive and accurate test results, with different size hammers and then measuring the response of the pile to the impact. The results are used to estimate the length of the pile. For higher quality of results, it is recommended to access the pile at the top and the side. Pile integrity testing will be performed in accordance with ASTM D5882 *Standard Test Method for Low Strain Impact Integrity Testing of Deep Foundations*.

PIT will be performed by an agreed upon subcontractor. The PIT subcontractor will work under the direction of the Consultant and, subsequent to the testing, will provide raw data and interpretive reports to all Participants. Depending on the data available for the design and as-built length of piles, PIT must be performed a few days prior to the geotechnical boring investigation for all Phase 1 test pits to determine the necessary depth of borings. Sufficient time will be provided to allow interpretation of PIT data and if inconclusive results are obtained, alternative testing techniques should be implemented. Refer to G19 Parallel Seismic logging.

G5 Crosshole Seismic Testing

Crosshole seismic testing will be performed between boreholes as directed during the field investigation as identified in Appendix B. Crosshole Seismic Testing will measure compression and shear waves between two adjacent boreholes at various depths according to ASTM D4428. The Consultant will determine the final holes to be tested based on findings of the borehole sampling and initial test results. Additional borings shall be made if necessary to satisfy the requirements of ASTM D4428. A crosshole testing plan will be provided by the consultant prior to execution.

G6 through G17, Laboratory Testing

Select samples from the field investigation of all joint borings and the test pits will be tested in a commonly agreed upon laboratory proposed by the Consultant. Specific tests and testing locations will be agreed upon on a rolling basis by all Participants as the field work is completed, and drilling logs have been provided. Reasonable efforts will

be made depending on available sample quantities to provide every requested test. Participants may request to use samples that were not selected for laboratory testing as part of this Protocol, for laboratory testing at their own expenses. The Consultant shall provide weekly drilling/investigation logs and testing will be agreed upon and ordered every week based on the field observations and the drill logs.

G6 Grain Size Analysis

Soils consist of particles of various shapes and sizes. The particle-size distribution (gradation) of soil can be determined from representative specimen following ASTM D6913 *Standard Test Methods for Particle-Size Distribution (Gradation) of Soils Using Sieve Analysis*. In cases where the gradation of particles smaller than No. 200 (75- μ m) sieve is needed, *Test Method D7928* may be used. All samples subjected to grain size evaluation with more than 10% fines should include hydrometer testing such that the D10 of the material can be determined for estimation of permeability.

G7 Moisture content

The moisture content of soil and rock can be determined in the laboratory by drying the material and measuring the reduction in mass of the material. The water content is the ratio of the mass of water contained in the pore spaces of soil or rock material to the solid mass of particles expressed as a percentage. The test is performed according to ASTM D2216 *Standard Test Methods for Laboratory Determination of Water (Moisture) Content of Soil and Rock by Mass*.

G8 Organic Content

Percentage of organic matter is important for classifying or evaluating peat or other organic soil, and for geotechnical and general soil classification purposes. This test can be used to determine the moisture content, ash content, and percent organic matter in the soil. The test is performed according to ASTM D2974 *Standard Test Methods for Moisture, Ash, and Organic Matter of Peat and Other Organic Soils*.

G9 Incremental Consolidation

The magnitude and rate of consolidation of fine-grained soil can be determined when it is restrained laterally and drained axially while subjected to incrementally applied controlled-stress loading. The test is performed on undisturbed soil samples in accordance with ASTM D2435 *Standard Test Methods for One-Dimensional Consolidation Properties of Soils Using Incremental Loading*. Loading increments specified for the lab will need to be agreed upon by all Participants.

At the request of any Participant, testing will be performed on reconstituted samples of sand to obtain stiffness parameters.

G10 Electrical Resistivity

Electrical resistivity for representative samples will be determined in situ and in the laboratory. Measurement of soil resistivity is used for the control of corrosion of buried structures and for the estimation of expected corrosion rates. The test will be performed according to ASTM G57 *Standard Test Method for Field Measurement of Soil Resistivity Using the Wenner Four-Electrode Method*.

G11 Chlorides, G12 pH, G13 Sulfates, G14 Sulfides

This group of tests is performed in a laboratory setting to determine chloride ion, sulfate ion, sulfide content, and pH level in the representative samples. The principal use of the pH test is to supplement soil resistivity measurements and thereby identify conditions under which the corrosion of metals in soil may be accentuated. The individual tests will be performed according to ASTM D512 *Standard Test Methods for Chloride Ion in Water*, ASTM D516 *Standard Test Method for Sulfate Ion in Water*, SM 4500 *Sulfide by Standard Methods Committee*, and G51 *Standard Test*

Method for Measuring pH of Soil for Use in Corrosion Testing. Water samples from bailed observation wells for groundwater chemistry testing shall be obtained.

G15 Unconfined Compression of Rock

The strength of intact rock core specimens in uniaxial compression will be determined following ASTM D7012C *Standard Test Methods for Compressive Strength and Elastic Moduli of Intact Rock Core Specimens under Varying States of Stress and Temperatures*. In the laboratory, a rock core specimen is cut to length and the ends are machined flat if possible and if not possible, as otherwise determined by the Participants. The specimen is placed in a loading frame, where the axial load on the specimen is increased and measured continuously until the sample fails. The uniaxial compressive strength is calculated as the maximum load at failure divided by the cross-sectional area of the specimen. Adequate instrumentation shall be included to determine Young's modulus.

G16 Unit weight of Rock Cores

Unit weight of rock cores will be determined in general accordance with ASTM D7263 *Standard Test Methods for Laboratory Determination of Density and Unit Weight of Soil Specimens*, method B (direct measurement). For a rock core specimen of cylindrical shape, the dimensions and mass of a specimen are measured. The density and unit weight are then calculated using these direct measurements.

G17 Specific Gravity

Specific gravity of soil samples will be determined in general accordance with ASTM D854, *Standard Test Methods for Specific Gravity of Soil Solids by Water Pycnometer*. These test methods cover the determination of the specific gravity of soil solids passing a sieve using a water pycnometer. These test methods are not viable for solids that can be altered by these methods, are contaminated with a substance that prohibits the use of these methods, or are highly organic (such as fibrous matter which floats in water).

G18 Seismic Cone Penetration Testing (CPT)

Seismic cone penetration testing estimates the small strain shear modulus of the soil by measuring shear wave velocities in the ground. The seismic CPT testing is performed as part of CPTU testing. The CPTU test will additionally measure continuous point and frictional resistance during penetration of the cone, pore pressures, and dissipation of pore pressures with time at constant depths. This testing will be carried out in general accordance with ASTM D5778 *Standard Test Method for Electronic Friction Cone and Piezocone Penetration Testing of Soils*.

The following geotechnical investigation is proposed:

- Seismic CPTs capped at 400-500 tsf with approximate locations as annotated in the attached Appendix B.
- CPTs will be pushed until refusal.
- Continuous measurements of point resistance, frictional resistance, pore pressure and shear wave velocity will be performed while the cone is pushed at a constant rate of 2 cm/sec. Three dissipation tests for each CPT location will be performed to establish groundwater and dissipation properties at those depths as directed by the Consultant.
- No sample recovery is performed with this testing.

The Seismic CPTs will be performed by the same mutually agreed upon contractor who will work with Consultant to observe and provide investigation reports. The Seismic CPT contractor will provide both raw data and reports including, but not limited to, cone tip resistance, frictional resistance, pore pressure measurement with depth, and pore pressure measurements with time during dissipation testing. Permeability values determined from pore pressure dissipation tests will be included.

G19 Parallel Seismic Logging

Parallel Seismic Logging (PSL) will be performed at indicated locations as annotated in the attached Appendix B. Boreholes will have casing and grout installed for the PSL testing that will be performed subsequent to the drilling activities. This testing involves sending a transceiver down the cased borehole to measure the depth of the buried sheet pile wall and will be carried out in general accordance with ASTM D8381 *Standard Test Methods for Measuring the Depth of Deep Foundations by Parallel Seismic Logging*.

Parallel seismic logging shall be accomplished as a supplemental means of determination of foundation lengths in the event that PIT interpretations are deemed inconclusive.

G20 Drained and Undrained Triaxial Testing of Fine-Grained Soil Samples

If fine-grained material is encountered and if thin-walled tube samples are used for obtaining intact specimens of fine-grained soils for laboratory tests to determine engineering properties, then triaxial testing will be performed on the specimens. This testing will be carried out in general accordance with ASTM D7181 *Standard Method for Consolidated Drained Triaxial Compression Test for Soils*, ASTM D4767 *Standard Test Method for Consolidated Undrained Triaxial Compression Test for Cohesive Soils*, and/or ASTM D2850 *Standard Test Method for Unconsolidated-Undrained Triaxial Compression Test on Cohesive Soils*.

At the request of any Participant, testing will be performed on reconstituted samples of sand to obtain shear strength parameters.

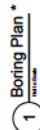
The Participants shall endeavor, but are not required, to agree on the Protocol of testing G20 including initial stress state before shearing, stress levels, strain rate, and drainage conditions during testing.

G21 Seismograph

Vibration monitoring shall be accomplished via a calibrated seismograph throughout the duration of all work accomplished on site at various locations, including those shown on Appendix B, with the monitors installed per location at different distances. The vibration monitoring shall start one week prior to the beginning of testing under the Protocol to establish a baseline at a trigger level of 0.05 in/s or lower, as required to record ambient/background levels; and shall continue 24- hours per day and 7-days per week for the duration of the work under the Protocol.

G22 MASW/ER

MASW and ER geophysical testing shall be accomplished along the entire length of the perimeter wall and on the extent of the basement slab.



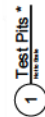
Champlain Towers South - Joint Testing Protocol

Proj. No.	2021.4155
Date	December 10, 2021
Drawn	SKA
Checked	MFF
Scale	As Noted

SK-01

Sheet No.

: BASE PLAN FROM DRAWINGS TITLED "CHAMPLAIN TOWERS SOUTH" PREPARED BY BREITERMAN JURADO & ASSOCIATES CONSULTING ENGINEERS, DATED 8/27/79



2/10/2021 5:55 PM by Andrews, Sandra File Name: CIVILE W/02021-4155 - Surface Sheets K-02 Test PLS.DWG












Proj. No.	20214155
Date	December 10, 2021
Drawn	SKA
Checked	MFF
Scale	As Noted

Project Champlain Towers South - Joint Testing Protocol
8777 Collins Ave., Surfside, Florida 33154

Sheet Title Test Pits

SK-02



- | | |
|---|---|
|  | CONCRETE CORE (3 CORES PER LOCATION U.N.O.) |
|  | STEEL SAMPLE (2 SAMPLES PER LOCATION U.N.O.) |
|  | EXPLORATORY OPENING AT TOP OF BASEMENT WALL, NOTE 1 |
|  | EXPLORATORY OPENING AT BOTTOM OF BASEMENT WALL, NOTE 2 |
|  | EXPLORATORY OPENING AT EXTERIOR FACE OF BASEMENT WALL, NOTE 3 |
|  | CONSTRUCTION JOINT |
|  | CONSTRUCTION JOINT |
|  | CONCRETE CHIPPING PER TESTING PROTOCOL |
|  | ADDITIONAL STRUCTURAL SAMPLING ITEM. SEE PROPOSED JOINT PROTOCOL FOR TESTING AND MATERIAL SAMPLING DOCUMENT |
|  | 1 FT x 1 FT STEEL PILING SAMPLE |
|  | APPROXIMATE EXTENT OF BUILDING |

NOTES:

- NOTES:
1. WALL OPENING, APPROX. 2 FT LONG BY FULL WALL WIDTH. PROCEED OPENING TO EXPOSE THE TOP OF THE SLAB-WALL CONNECTION. SEE TESTING PROTOCOL ADD. STRUCT. SAMPLING ITEM #41, #10, #11 AND #12.
 2. BOT. WALL OPENING. PROCEED OPENING TO EXPOSE THE BOTTOM OF THE SLAB-WALL CONNECTION. SEE TESTING PROTOCOL ADD. STRUCT. SAMPLING ITEM #13 AND #14.
 3. EXT. WALL OPENING, APPROX. 2 FT LONG BY 2 FT DEEP. PROCEED OPENING TO EXPOSE THE EXTERIOR FACE OF SLAB-WALL CONNECTION AND SHEET PILING. SEE TESTING PROTOCOL ADD. ITEM #41.
 4. ALL LOCATIONS ARE APPROXIMATE.

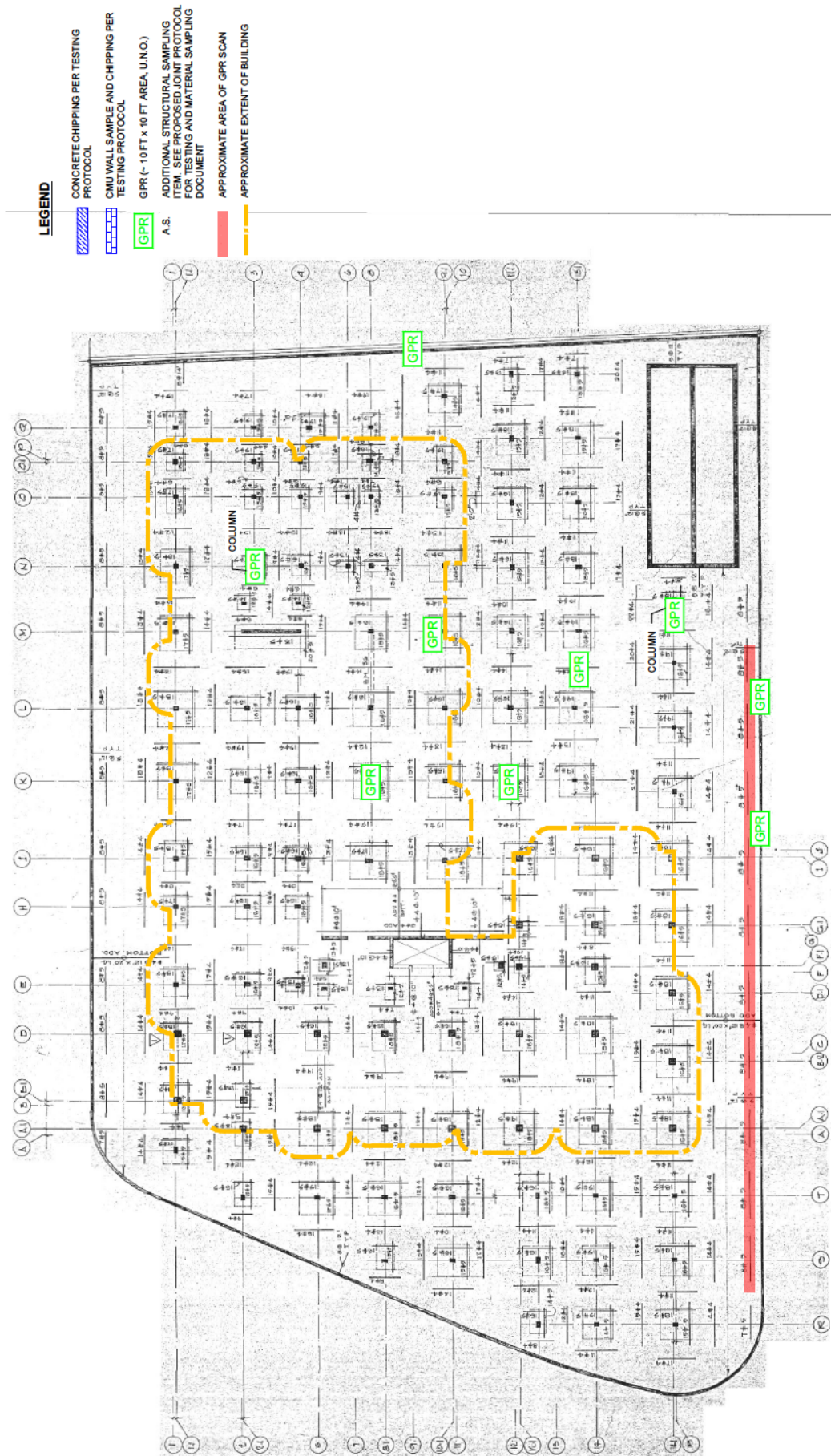
Material Samples and Expository Opening Plan *

Project	Champlain Towers South - Joint Testing Protocol		Proj. No.	2021.4155	SK-03
	8777 Collins Ave., Surfside, Florida 33154		Date	December 10, 2021	
	Sheet Title		Drawn	SKA	
	Material Samples and Expiratory Openings		Checked	MFF	
			Scale	As Noted	
				Sheet No.	

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Appendix B
Champlain Towers South Investigation - Joint Protocol for Testing and Material Sampling (Collapse Site)

5



1 Basement Plan

Revised: January 13, 2022

Project	Champlain Towers South - Joint Testing Protocol
Sheet Title	Basement NDT
Project No.	2021.4155
Date	December 10, 2021
Drawn	SKA
Checked	MFF
Scale	As Noted

SK-04

Sheet No.

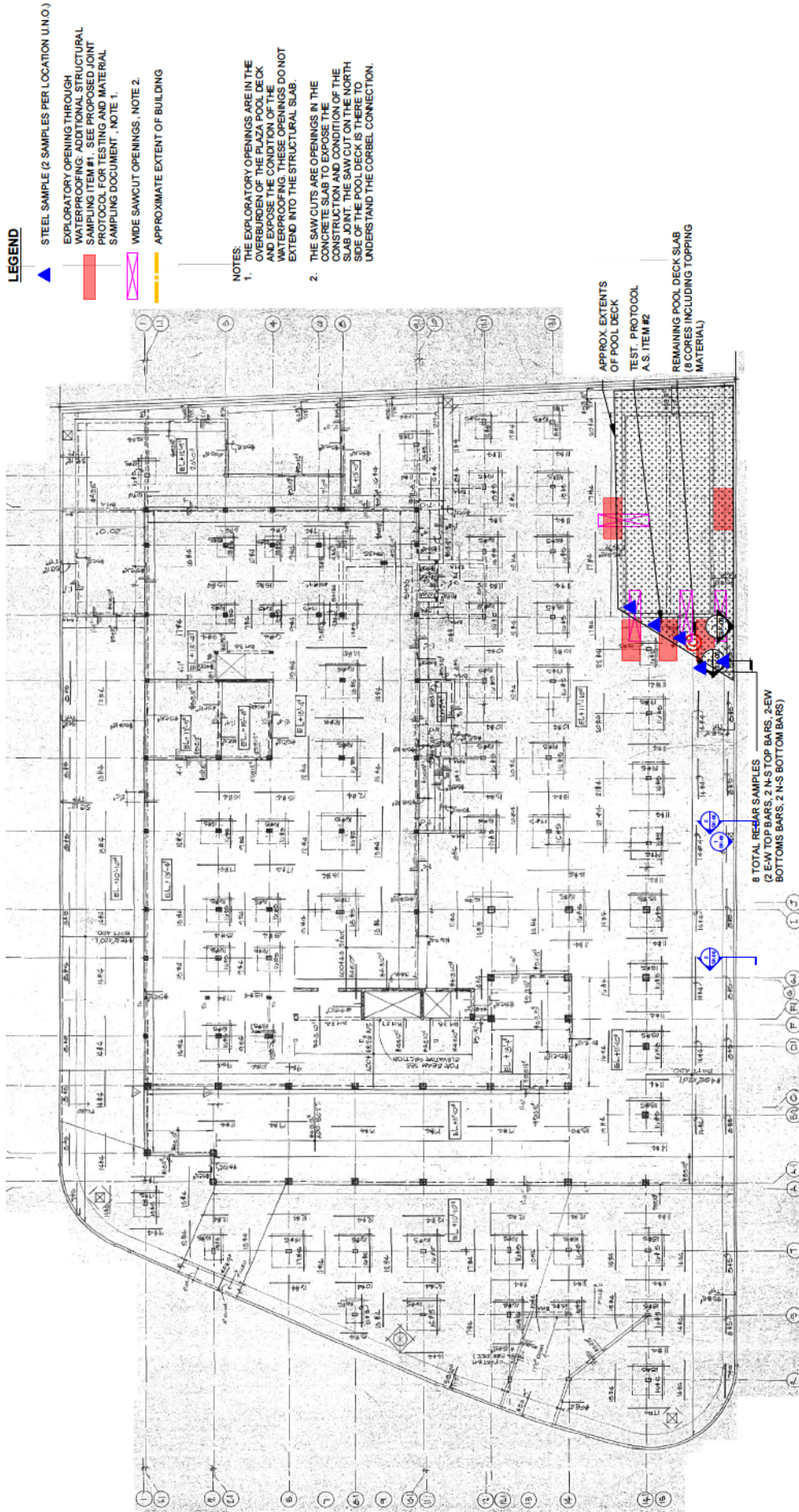
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BASE PLAN FROM DRAWINGS TITLED "CHAMPLAIN TOWERS SOUTH" PREPARED BY BREITHEIMAN JURADO & ASSOCIATES CONSULTING ENGINEERS, DATED 9/27/19

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Appendix B Champlain Towers South Investigation - Joint Protocol for Testing and Material Sampling (Collapse Site)



Pool Deck Level

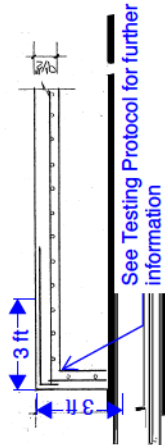
Revised: January 13, 2022

Project	Champlain Towers South - Joint Testing Protocol		Proj. No.	2021.4155
	8777 Colina Ave., Sunrise, Florida 33154		Date	December 10, 2021
Sheet Title	Pool Deck Level		Drawn	SKA
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SK-05

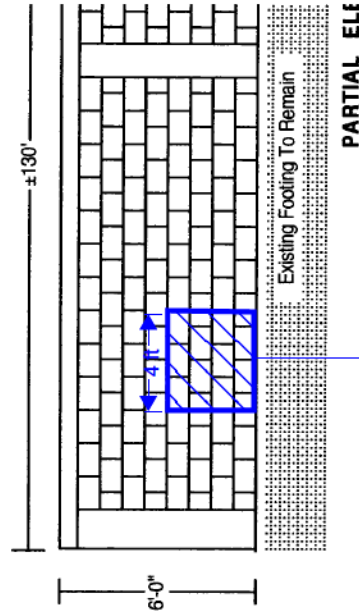
* BASE PLAN FROM DRAWINGS TITLED "CHAMPLAIN TOWERS SOUTH" PREPARED BY BREITERMAN JURADO & ASSOCIATES CONSULTING ENGINEERS, DATED 9/27/19

Files: 1/10/2021 1:59 PM Jay Anderson, Sunrise File Name: CWJE 190202.4155 - Sunrise Towers SK-05 Pool Deck Level.dwg



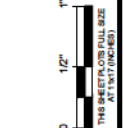
TESTING PROTOCOL ADDITIONAL
STRUCTURAL ITEM #4

NOTE:
AT GRIDLINE K, REMOVAL CONSISTS OF THE
PRIVACY WALL, FOUNDATION WALL, AND
RESIDUAL SLAB.
AT GRIDLINE H, REMOVAL CONSISTS OF THE
FOUNDATION WALL AND RESIDUAL SLAB.



TESTING PROTOCOL ADDITIONAL
STRUCTURAL ITEM #3

WJE ENGINEERS ARCHITECTS MATHEMATICAL SCIENTISTS Wiss, Janney, Elstner Associates, Inc. 330 Princeton Road Naperville, IL 60563 630.272.7400 (N) 847.490.0304 (F) www.wje.com	Project	Champlain Towers South - Joint Testing Protocol
	Sheet Title	8777 Colina Ave., Sunrise, Florida 33154
	Drawn	SKA
	Checked	MFF
Scale		As Noted
Sheet No.		SK-06



* BASE PLAN FROM UNDATED DRAWING SHEET NO. S1 TITLED CHAMPLAIN TOWERS SOUTH
PREPARED BY BREITERMAN JURADO & ASSOCIATES CONSULTING ENGINEERS
Plotted: 1/21/2021 1:59 PM by Andrew, Sandra File Name: C:\WJE\Work\2021\4155 - Sunrise\Structural\SK-06 Additional Items.dwg

IN THE CIRCUIT COURT OF THE 11TH
JUDICIAL CIRCUIT IN AND FOR
MIAMI-DADE COUNTY, FLORIDA

IN RE: CHAMPLAIN TOWERS SOUTH
COLLAPSE LITIGATION,

CLASS REPRESENTATION

CBL DIVISION

CASE NO: 2021-015089-CA-01

**EMERGENCY MOTION FOR COURT APPROVAL OF ACCESS TO PROPERTY
CONTROLLED BY DEFENDANT 8701 COLLINS AVENUE CONDOMINIUM
ASSOCIATION, INC. FOR THE PURPOSE OF PERFORMING JOINT TESTING
PROTOCOL TESTING AND, ALTERNATIVELY, MOTION TO SHORTEN TIME TO
RESPOND TO REQUEST FOR ENTRY UPON LAND FOR INSPECTION**

Defendant 8701 Collins Development, LLC (“8701 Collins Development”) moves (1) for Court approval under the Joint Testing Protocol for testing on property controlled by Defendant 8701 Collins Avenue Condominium Association, Inc. (the “8701 Condo Association”) and, alternatively, and in an abundance of caution, (2) to shorten the time for the 8701 Condo Association to respond to 8701 Collins Development’s Request for Entry Upon Land for Inspection (the “Request for Inspection”) to one day from the date of the Request for Inspection.

1. On January 21, 2022, this Court entered an Order Regarding Joint Protocol for Testing and Material Sampling – Collapse Site (the “January 21 Order”), which required the litigants to “promptly mobilize and begin invasive testing at the collapse site at the beginning of February” and required that all such testing be “performed pursuant to the agreed upon Champlain Towers South Collapse Investigation: Joint Protocol for Testing and Material Sampling – Collapse Site (the ‘Joint Testing Protocol’)” The January 21 Order also required that the parties “keep the Court informed as to the scheduling and progress of the testing being performed pursuant to the Joint Testing Protocol.” Copies of the January 21 Order and the Joint Testing Protocol are attached as Composite Exhibit A.

2. The Joint Testing Protocol states that “[t]here may be several boring, test pits and other tests anticipated under this Protocol that are not located on the CTS Property *and are subject to access authorized by the property owner and/or Court approval.*” Notice of Filing Joint Protocol for Testing and Material Sampling – Collapse Site, Ex. A. at 4 (emphasis added).

3. The 8701 Condo Association owns the property located at 8701 Collins Avenue, Miami Beach, Florida, which is immediately south of the site of the Champlain Towers South Condominium. The Joint Testing Protocol calls for testing to be performed in a beach access easement located over this property (the “Beach Access Area”).

4. The Beach Access Area is situated at the northern edge of the property owned by the 8701 Condo Association and stretches from Collins Avenue eastward to the beach. The Joint Testing Protocol identifies, in Appendix B, the nature and the specific locations of the testing.

5. According to Geosyntec Consultants, Inc., the independent consultant hired by this Court’s appointed Receiver, testing within the Beach Access Area must begin no later than **Monday, April 18, 2022**. If testing cannot commence by then, Geosyntec’s contractors will have to demobilize from the Champlain Towers South property, which will delay the completion of the testing required by the Joint Testing Protocol.

6. Because the Beach Access Area is not located on the CTS Property, either the 8701 Condo Association or the Court must authorize access for testing to occur.

7. Despite multiple attempts by 8701 Collins Development’s counsel to obtain such access, counsel for the 8701 Condo Association has advised counsel for 8701 Collins Development that it will not grant Geosyntec and its contractors access to the Beach Access Area for execution of the Joint Testing Protocol.

8. To avoid delays and complete the testing called for by the Joint Testing Protocol, 8701 Collins Development seeks an expedited order authorizing access to the Beach Access Area for the purposes of performing the required testing.

9. Alternatively, and in an abundance of caution, 8701 Collins Development has served a Request for Inspection on the 8701 Condo Association to obtain access to perform the testing called for by the Joint Testing Protocol. A copy of the Request for Inspection is attached as Exhibit B.

10. Pursuant to Fla. R. Civ. P. 1.350(b), the Court may shorten the time for a party to respond to a request for inspection. *See* Fla. R. Civ. P. 1.350(b) (“The party to whom the request is directed shall serve a written response within 30 days after service of the request, except that a defendant may serve a response within 45 days after service of the process and initial pleading on that defendant. *The court may allow a shorter or longer time.*”) (emphasis added).

11. To complete the required testing and to comply with the schedule established by the Court, 8701 Collins Development requests that the 8701 Condo Association respond to the Request for Inspection within one day, and that the Court expedite its consideration and adjudication of any objections that the 8701 Condo Association may raise so as to preserve the testing schedule.

12. Counsel for 8701 Collins Development has conferred with the 8701 Condo Association’s counsel regarding the subject matter of this motion and have been unable to reach an agreement.

WHEREFORE, 8701 Collins Development respectfully requests that the Court grant this Motion and approve access to property controlled by the 8701 Condo Association pursuant to the Joint Testing Protocol. Alternatively, 8701 Collins Development respectfully requests that the

Court shorten the time for the 8701 Condo Association to respond to the Request for Inspection, require that any objections be served within one day after service of the Request for Inspection, that consideration and adjudication of any objections be expedited so as to preserve the testing schedule, and that the Court grant such additional relief as it deems proper.

Dated: April 14, 2022

Respectfully submitted,

GREENBERG TRAURIG, P.A.
Counsel for 8701 Collins Development, LLC
Terra Group, LLC and
Terra World Investments, LLC
333 S.E. 2nd Avenue, Suite 4400
Miami, Florida 33131-3238
Telephone No.: (305) 579-0506
Facsimile No.: (305) 579-0717
Email: thomasmic@gtlaw.com

By: /s/ Michael J. Thomas
MICHAEL J. THOMAS
Florida Bar No.: 21309

CERTIFICATE OF SERVICE AND CONFERENCE

I certify that on April 13, 2022, I conferred with counsel for the 8701 Condo Association concerning the subject matter of this motion but was unable to resolve the motion by agreement. I further certify that a true and correct copy of the foregoing was furnished, either via transmission of Notices of Service of Court Document generated by the E-Portal or in some other authorized manner for those counsel or parties who are excused from e-mail service on this 14th day of April 2022.

/s/ Michael J. Thomas
MICHAEL J. THOMAS

COMPOSITE EXHIBIT “A”

**IN THE CIRCUIT COURT OF THE ELEVENTH JUDICIAL
CIRCUIT IN AND FOR MIAMI-DADE COUNTY, FLORIDA**

CASE NO: 2021-015089-CA-01

SECTION: CA43

JUDGE: Michael Hanzman

In Re: Champlain Towers South Collapse Litigation

Plaintiff(s)

vs.

N/A

Defendant(s)

_____ /

**ORDER REGARDING JOINT PROTOCOL FOR TESTING AND MATERIAL
SAMPLING – COLLAPSE SITE**

THIS CAUSE came before the Court upon a series of conferences and hearings, including the December 22, 2021 evidentiary hearing, the Court’s December 30, 2021 *sua sponte* Order, the Court’s January 3, 2022 Order, and hearings on January 14, 2022 and January 21, 2022. The Court having considered the issues and positions of the parties, it is **ORDERED AND ADJUDGED** as follows:

1. The parties and their respective experts, consultants, and subcontractors shall promptly mobilize and begin invasive testing at the collapse site at the beginning of February. The invasive testing shall be performed pursuant to the agreed upon Champlain Towers South Collapse Investigation: Joint Protocol for Testing and Material Sampling – Collapse Site (the “Joint Testing Protocol”), which was submitted to the Court on January 21, 2022.
2. Unless otherwise provided in the Joint Testing Protocol, the testing performed on site will be coordinated and overseen by the independent Consultant, Geosyntec Consultants, Inc. (“Consultant”), which was chosen by the Receiver with the

consent of the Parties. The Consultant, subject to review by all parties, will employ testing agencies and contractors required to carry out the goals of the Joint Testing Protocol.

3. The Joint Testing Protocol shall allow all parties adequate opportunity to collect and test evidence so as to allow the parties to properly investigate the cause of this collapse as well as the claims advanced by Plaintiffs and any potential defenses.
4. The Parties shall keep the Court informed as to the scheduling and progress of the testing being performed pursuant to the Joint Testing Protocol.
5. Except for the costs attributable to additional or specialized testing, which costs are the sole responsibility of the Participant seeking such additional or specialized testing, each Participant's or other third-party's allocation of the costs incurred through performance of this Protocol shall be determined pursuant to an order from the Court to be issued subsequently.

DONE and ORDERED in Chambers at Miami-Dade County, Florida on this 21st day of January, 2022.



2021-015089-CA-01 01-21-2022 1:47 PM

Hon. Michael Hanzman

CIRCUIT COURT JUDGE

Electronically Signed

No Further Judicial Action Required on **THIS MOTION**

CLERK TO **RECLOSE** CASE IF POST JUDGMENT

Electronically Served:

Aaron Podhurst, apodhurst@podhurst.com

Aaron Podhurst, dricker@podhurst.com

Adam A Schwartzbaum, adams@moskowitz-law.com

Adam A Schwartzbaum, service@moskowitz-law.com
Adam A Schwartzbaum, dione@moskowitz-law.com
Adam Moskowitz, adam@moskowitz-law.com
Adam Moskowitz, dione@moskowitz-law.com
Adam Moskowitz, service@moskowitz-law.com
Alfred Armas, alfred@armaslaw.com
Alison E Patino, apatino@patinolaw.com
Amanda Anderson, AAnderson@insurance-counsel.com
Amanda K Anderson, eservice@insurance-counsel.com
Amanda K Anderson, aanderson@insurance-counsel.com
Amanda K Anderson, InsuranceCounsel2050@gmail.com
Andrew B. Yaffa, aby@grossmanroth.com
Andrew B. Yaffa, omb@grossmanroth.com
Andrew M Feldman, feldmana@kleinpark.com
Andrew M Feldman, montanem@kleinpark.com
Andrew M Feldman, piersonj@kleinpark.com
Andrew P. Gold, Esq., andrew.gold@akerman.com
Andrew P. Gold, Esq., jill.parnes@akerman.com
Andrew Paul Gold, andrew.gold@akerman.com
Andrew Paul Gold, jill.parnes@akerman.com
Anthony J Carriuolo, acarriuolo@bergersingerman.com
Anthony J Carriuolo, mnewland@bergersingerman.com
Anthony J Carriuolo, drt@bergersingerman.com
Anthony Perez, aperez@hsmipa.com
Aron Raskas, araskas@gunster.com
Aron Raskas, avalido@gunster.com
Aron Raskas, eservice@gunster.com
Benjamin Fernandez, IV, bfernandez@silvasilva.com
Benjamin Fernandez, IV, tgomez@silvasilva.com
Bernadette Vazquez, bvazquez@klugerkaplan.com
Bernadette Vazquez, clong@klugerkaplan.com
Bradford R. Sohn, brad@bradsohnlaw.com
Bradford R. Sohn, paralegal@bradsohnlaw.com
Bradford R. Sohn, service.bradsohnlaw@gmail.com
Bradley J Edwards, staff.efile@epllc.com
Bradley J Edwards, brad@epllc.com
Bradley J Edwards, maria@epllc.com
Brenda Radmacher, brenda.radmacher@akerman.com

Bret M Feldman, feldmanb@phelps.com
Bret M Feldman, yolanda.vazquez@phelps.com
Brian S Dervishi, bdervishi@wdpalaw.com
Brian S Dervishi, service@wdpalaw.com
Bruce Alan Katzen, bkatzen@klugerkaplan.com
Bruce Alan Katzen, cfalla@klugerkaplan.com
Bruce Alan Katzen, probate@klugerkaplan.com
Carlos A. Velasquez, CVelasquez@VDLawyers.com
Carlos A. Velasquez, Andrea@VDLawyers.com
Carlos E Silva, csilva@silvasilva.com
Carlos E Silva, mromera@silvasilva.com
Carlos M. Macias, macias@leesfield.com
Carlos M. Macias, becerra@leesfield.com
Carlos M. Macias, abreu@leesfield.com
Caroline Catchpole Spradlin, caroline.spradlin@phelps.com
Caroline Catchpole Spradlin, samantha.powell@phelps.com
Carolyn M. Luna, cluna@patinolaw.com
Christine L. Welstead, christine.welstead@bowmanandbrooke.com
Christine L. Welstead, ashleigh.carroll@bowmanandbrooke.com
Christine L. Welstead, lisa.morales@bowmanandbrooke.com
Christopher L Barnett, barnettch@gtlaw.com
Christopher L Barnett, nicauda@gtlaw.com
Christopher S Carver, christopher.carver@akerman.com
Christopher S Carver, cary.gonzalez@akerman.com
Cole Scott &Kissane PA, Ryan.Charlson@csklegal.com
Cole Scott &Kissane PA, Ryan.Sooahoo@csklegal.com
Cole Scott &Kissane PA, Construction.FTLW@csklegal.com
Cosme Caballero, ccaballero@deutschblumberg.com
Cosme Caballero, kdominguez@deutschblumberg.com
Cosme Caballero, rvargas@deutschblumberg.com
Cosme Caballero, Esq., ccaballero@deutschblumberg.com
Cosme Caballero, Esq., bblumberg@deutschblumberg.com
Curtis Miner, curt@colson.com
Curtis Miner, claudiav@colson.com
Curtis Miner, eservice@colson.com
Daniel Y Gielchinsky, dan@dgimlaw.com
Daniel Y Gielchinsky, tatiana@dgimlaw.com
David B. Weinstein, weinsteind@gtlaw.com

David B. Weinstein, thomasm@gtlaw.com
David L Rosinsky Esq., LUKSFLL-Pleadings@LS-LAW.COM
David L Rosinsky Esq., Drosinsky@insurancedefense.net
David L Rosinsky Esq., drosenbaum@insurancedefense.net
David M Wells, dwells@gunster.com
David M Wells, dculmer@gunster.com
David M. Murray, dmmurray@miami-airport.com
David M. Murray, dmmurray@miami-airport.com
David M. Murray, rmartin@miami-airport.com
David M. Wells, dwells@gunster.com
David M. Wells, dculmer@gunster.com
David M. Wells, eservice@gunster.com
David Stanoch, david@honiklaw.com
Dorian N. Daggs, ddaggs@hsmmpa.com
Douglas J. Kress, dkress@schwedpa.com
Douglas J. Kress, lrandell@schwedpa.com
Douglas J. Kress, amarcus@schwedpa.com
Dustin C. Blumenthal, dblumenthal@goldbergsegalla.com
Dustin C. Blumenthal, ppowers@goldbergsegalla.com
E. Bruce Johnson, johnson@jambg.com
E. Bruce Johnson, young@jambg.com
Edward Marod, emarod@gunster.com
Edward Marod, dpeterson@gunster.com
Edward R. Blumberg, erb@deutschblumberg.com
Edward R. Blumberg, rmitchell@deutschblumberg.com
Edward R. Blumberg, kdominguez@deutschblumberg.com
Elliot H. Scherker, scherkere@gtlaw.com
Elliot H. Scherker, miamiappellateservice@gtlaw.com
Eric P. Hockman, ehockman@wsh-law.com
Eric P. Hockman, lmartinez@wsh-law.com
Eric Page Hockman, ehockman@wsh-law.com
Eric Page Hockman, lmartinez@wsh-law.com
Eric S Kay, ekay@kttlaw.com
Eric S Kay, ga@kttlaw.com
Eric S Kay, agarcia@kttlaw.com
Felipe Gonzalez, fgonzalez@switkeslaw.com
Felipe Gonzalez, paralegal@switkeslaw.com
Felipe Gonzalez, paralegal@switkeslaw.com

Frank Florio, FFlorio@gunster.com
George R Truitt Jr., george.truitt@csklegal.com
George R Truitt Jr., construction.miami@csklegal.com
Gonzalo Barr, gbarr@dldlawyers.com
Gonzalo Barr, viviane@dldlawyers.com
Gonzalo Dorta, grd@dortalaw.com
Gonzalo R Dorta, grd@dortalaw.com
Gonzalo R Dorta, jpedraza@dortalaw.com
Gonzalo R Dorta, jgonzalez@dortalaw.com
Graham LippSmith, g@lippsmith.com
Graham LippSmith, mb@lippsmith.com
Graham LippSmith, cca@lippsmith.com
H. Clay Roberts, gloria@robertspa.com
H. Clay Roberts, roberts@robertspa.com
H. Clay Roberts, jennifer@robertspa.com
Hali E Marsocci, Hali@RomanoLawGroup.com
Hali E Marsocci, Becky@RomanoLawGroup.com
Hali E Marsocci, Service@RomanoLawGroup.com
Harley S. Tropin Esq., hst@kttlawn.com
Harley S. Tropin Esq., sf@kttlawn.com
Harley Tropin, hst@kttlawn.com
Henry Lawrence Perry, lperry@perry-young.com
Henry Lawrence Perry, kroberson@perry-young.com
Henry Lawrence Perry, frichard@perry-young.com
Henry N. Wixon, henry.wixon@nist.gov
Houston S. Park III, parkh@kleinpark.com
Houston S. Park III, harriss@kleinpark.com
Howard Kraft Pita, spita@pwndlawfirm.com
Howard Kraft Pita, lalvarez@pwndlawfirm.com
Howard Kraft Pita, pitaservice@pwndlawfirm.com
Howard M Bushman, howard@moskowitz-law.com
Howard M Bushman, dione@moskowitz-law.com
JOHN H RUIZ, jruiz@msprecoverylawfirm.com
JOHN H RUIZ, serve@msprecoverylawfirm.com
JOHN H RUIZ, jruiz@msprecovery.com
Janel C. Diamond, jdiamond@gunster.com
Javier A. Lopez Esq., jal@kttlawn.com
Javier A. Lopez Esq., ya@kttlawn.com

Javier A. Lopez Esq., fsr@kttlaw.com
Javier Zapata, jzapata@miamidade.gov
Jeffrey Lee Newsome II, jeffrey.newsome@phelps.com
Jeffrey Lee Newsome II, samantha.powell@phelps.com
Jeffrey Michael Cohen, jmcohen@cfjblaw.com
Jeffrey Michael Cohen, schacon@carltonfields.com
Jeffrey Michael Cohen, miaecf@cfdom.net
Jillian M Askren, askrenj@gtlaw.com
Jillian M Askren, thomasm@gtlaw.com
Joel L McNabney, joel.mcnabney@clydeco.us
Joel L McNabney, Audrie.Finney@clydeco.us
Joel L McNabney, miainsfilings@clydeco.us
John B. Morgan, jmorgan@forthepeople.com
John B. Morgan, kmitnik@forthepeople.com
John B. Morgan, andrew@forthepeople.com
John Davis, jdavis@slackdavis.com
John H. Ruiz, jruiz@msprecoverylawfirm.com
John H. Ruiz, serve@msprecoverylawfirm.com
John H. Ruiz, afernandez@msprecoverylawfirm.com
John Romano, John@RomanoLawGroup.com
John Scarola, _scarolteam@searcylaw.com
John Scarola, mmccann@searcylaw.com
John Scarola, _scarolteam@searcylaw.com
Jonathan E Kanov, jekanov@mdwgc.com
Jonathan E Kanov, kafriday@mdwgc.com
Jordi Gusó, jguso@bergersingerman.com
Jordi Gusó, drt@bergersingerman.com
Jordi Gusó, fsellers@bergersingerman.com
Jorge A Calil Esq., jorge@jcalillaw.com
Jorge A Calil Esq., jeannie.calillaw@gmail.com
Jorge L. Piedra, jpiedra@kttlaw.com
Jorge L. Piedra, ga@kttlaw.com
Jorge Silva, jsilva@silvasilva.com
Jorge Silva, csilva@silvasilva.com
Jorge Silva, hsoto@silvasilva.com
Joseph H. Serota, jserota@wsh-law.com
Joseph H. Serota, lmartinez@wsh-law.com
Joseph M. Kaye, joseph@moskowitz-law.com

Joseph M. Kaye, dione@moskowitz-law.com
Josh M Rubens, jrubens@klugerkaplan.com
Josh M Rubens, cfalla@klugerkaplan.com
Josh M Rubens, cfernandez@klugerkaplan.com
Joshua Talcovitz, TalcovitzJ@kleinpark.com
Jourdan Leslie Weltman, jw0326@universalproperty.com
Jourdan Leslie Weltman, subrogationservice@universalproperty.com
Jourdan Leslie Weltman, vm1217@universalproperty.com
Judd G. Rosen, pleadings@goldbergandrosen.com
Judd G. Rosen, jgrsecy@goldbergandrosen.com
Julia Holden-Davis, JHoldenDavis@gunster.com
Julia Holden-Davis, dholland@gunster.com
Julian S. Geraci Esq., jgeraci@pbcgov.org
Julian S. Geraci Esq., aairey@pbcgov.org
Karen B Parker, kparker@kbparkerlaw.com
Karen B Parker, fmartinez@kbparkerlaw.com
Karen B Parker, ebacker@kbparkerlaw.com
Kenneth R Drake, kendrake@dldlawyers.com
Kenneth R Drake, viviane@dldlawyers.com
Kerry L. Burns, kburns@bergersingerman.com
Kerry L. Burns, efile@bergersingerman.com
Laritza Orozco, Orozcol@kleinpark.com
Laura Adams, lauraadams@miamisao.com
Lauren E. Morse, laurenm@miamidade.gov
Lauren E. Morse, olga1@miamidade.gov
Lauren E. Morse, hern@miamidade.gov
Lauren Morse, lauren.morse@miamidade.gov
Luis Eduardo Suarez, lsuarez@hsmpa.com
Luis Eduardo Suarez, filings@hsmpa.com
Manual Arteaga-Gomez, aag@grossmanroth.com
Marc J. Gottlieb, marc.gottlieb@akerman.com
Marc J. Gottlieb, joyce.gutierrez@akerman.com
Marc J. Gottlieb, Esq., marc.gottlieb@akerman.com
Marc J. Gottlieb, Esq., joyce.gutierrez@akerman.com
Mark A Boyle, Eservice@Insurance-Counsel.com
Mark A Boyle, MBoyle@Insurance-Counsel.com
Mark A Boyle, InsuranceCounsel2050@gmail.com
Mark J. Heise, mheise@hsmpa.com

Mark J. Sullivan, sullivanm@kleinpark.com
Mark J. Sullivan, piersonj@kleinpark.com
Mark J. Sullivan, montanem@kleinpark.com
Mark R. Antonelli, mantonelli@gaebemullen.com
Mark R. Antonelli, cgreer@gaebemullen.com
Mark R. Antonelli, lbeggs@gaebemullen.com
Matthew Wildner, mjwildner@mdwgcg.com
Matthew Wildner, kafriday@mdwgcg.com
Meghan C Moore Moore, meghan.moore@flastergreenberg.com
Meghan C Moore Moore, betty.plasencia@flastergreenberg.com
Michael Caballero, mcaballero@hsmpa.com
Michael F. Suarez, MFS-KD@kubickidraper.com
Michael F. Suarez, mfs@kubickidraper.com
Michael I. Goldberg, michael.goldberg@akerman.com
Michael I. Goldberg, charlene.cerda@akerman.com
Michael I. Goldberg, kimberly.smiley@akerman.com
Michael J Thomas, thomasmic@gtlaw.com
Michael J Thomas, cordovam@gtlaw.com
Michael J Thomas, flservice@gtlaw.com
Michael S Hooker, michael.hooker@phelps.com
Michael S Hooker, guy.mcconnell@phelps.com
Michael S Hooker, renee.hogue@phelps.com
Mitchel Chusid, mchusid@ritterchusid.com
Mitchel Chusid, karenz@ritterchusid.com
Mitchel Chusid, ifeliciano@ritterchusid.com
Mustafa Hasan Dandashly, pleadings@goldbergandrosen.com
Mustafa Hasan Dandashly, mdandashly@goldbergandrosen.com
Mustafa Hasan Dandashly, evelyn@goldbergandrosen.com
Nicole Juarez, njuarez@hsmpa.com
Oscar E. Marrero, oem@marrerolegal.com
Patricia Melville, pmelville@hsmpa.com
Paul A. Avron, pavron@bergersingerman.com
Paul J. Schwiep, pschwiep@coffeyburlington.com
Paul J. Schwiep, yvb@coffeyburlington.com
Paul J. Schwiep, service@coffeyburlington.com
Paul Jon Layne, playne@silvasilva.com
Paul Jon Layne, mromera@silvasilva.com
Paul Jon Layne, vramos@silvasilva.com

Paul S Singerman, singerman@bergersingerman.com
Paul S Singerman, mdiaz@bergersingerman.com
Paul S Singerman, efile@bergersingerman.com
Paul Steven Labiner, Pleadings@PIALawCenter.com
Rachel W. Furst, rwf@grossmanroth.com
Rachel Wagner Furst, rwf@grossmanroth.com
Ralph George Patino, service@patinolaw.com
Ralph George Patino, rpatino@patinolaw.com
Ralph George Patino, cluna@patinolaw.com
Rami Shmuely, rshmuely@cmslawgroup.com
Randy M Weber, rmweber@pwndlawfirm.com
Randy M Weber, ngarcia@pwndlawfirm.com
Ricardo Manoel Martinez-Cid, rmcid@podhurst.com
Ricardo Manoel Martinez-Cid, RMCTeam@podhurst.com
Ricardo Manoel Martinez-Cid, lbarrington@podhurst.com
Robert J. Mongeluzzi, rmongeluzzi@smbb.com
Robert J. Mongeluzzi, jgoodman@smbb.com
Robert J. Mongeluzzi, sdordick@smbb.com
Robert L Switkes, Rswitkes@switkeslaw.com
Robert L Switkes, paralegal@switkeslaw.com
Robert L Switkes, bzappala@switkeslaw.com
Robert M Klein, kleinr@kleinpark.com
Robert M Klein, piersonj@kleinpark.com
Robert M Klein, carrillom@kleinpark.com
Roselyn Lax, rlax@hsmipa.com
Ruben Honik, ruben@honiklaw.com
Ryan A Waton, rwaton@zdlaw.com
Ryan J. Yaffa, rjy@grossmanroth.com
Ryan M. Charlson, ryan.charlson@csklegal.com
Ryan M. Charlson, nicole.kaufman@csklegal.com
Ryan Thomas Hopper, hopperr@gtlaw.com
Ryan Thomas Hopper, ryan.t.hopper@gmail.com
Ryan Thomas Hopper, thomasm@gtlaw.com
STEVEN JEFFREY BRODIE, sbrodie@carltonfields.com
STEVEN JEFFREY BRODIE, ldelpino@carltonfields.com
Scott Andrew Hiaasen, shiaasen@coffeyburlington.com
Scott Andrew Hiaasen, lperez@coffeyburlington.com
Scott Andrew Hiaasen, service@coffeyburlington.com

Sergio Bueno, sergio.bueno@clydeco.us
Sergio Bueno, Audrie.Finney@clydeco.us
Sergio Bueno, miainsfilings@clydeco.us
Sergio L Mendez, sergio@mendezandmendezlaw.com
Sergio L Mendez, service@mendezandmendezlaw.com
Sergio L Mendez, lorena@mendezandmendezlaw.com
Seth M Lehrman, staff.efile@epllc.com
Seth M Lehrman, seth@epllc.com
Seth M Lehrman, iris@epllc.com
Seth M Schimmel, seth.schimmel@phelps.com
Seth M Schimmel, yolanda.vazquez@phelps.com
Shannon Lyn Nunez del Prado, sdelprado@pwwlawfirm.com
Stuart Z Grossman, szg@grossmanroth.com
Stuart Z Grossman, lka@grossmanroth.com
Tal J Lifshitz, tj1@kttlaw.com
Tal J Lifshitz, ya@kttlaw.com
Tal J Lifshitz, fsr@kttlaw.com
Tal J. Lifshitz, tj1@kttlaw.com
Tal J. Lifshitz, ya@kttlaw.com
Thomas A. Robes, Trobes@robeslawgroup.com
Thomas A. Robes, jbuchko@robeslawgroup.com
Thomas A. Robes, dmarie@robeslawgroup.com
Todd Romano, Todd@RomanolawGroup.com
Valerie Shea, vshea@goldbergsegalla.com
Wanda Monteverde, monteverdew@kleinpark.com
William F. Merlin Jr, cmerlin@merlinlawgroup.com
William F. Merlin Jr, ssmith@merlinlawgroup.com
William F. Merlin Jr, trodriguez@merlinlawgroup.com
William J Tinsley Jr., william.tinsley@phelps.com
William J Tinsley Jr., yolanda.vazquez@phelps.com
William P. Mulligan, wpm@grossmanroth.com
William P. Mulligan, lka@grossmanroth.com
Yechezkel Rodal, chezky@forthepeople.com
Yechezkel Rodal, rmongeluzzi@smbb.com
Yechezkel Rodal, jgoodman@smbb.com
Yechezkel Rodal, chezky@forthepeople.com
Yitzhak Levin, ylevin@levinlitigation.com
Yitzhak Levin, service@levinlitigation.com

Yummy Marrero, Ymarrero@gaebemullen.com

Physically Served:

CHAMPLAIN TOWERS SOUTH COLLAPSE INVESTIGATION:
Joint Protocol for Testing and Material Sampling – Collapse Site

In accordance with the Court Order dated September 1, 2021, this document presents the protocol for testing and material sampling at the collapse site (the “Protocol”) and has been developed considering input from involved Participants.

DEFINITIONS

- **Consultant** – Geosyntec Consultants, Inc., an independent consultant approved by the Receiver to coordinate, plan, oversee, and provide factual reporting as defined in this Protocol to the Experts/Participants. The Consultant will employ Testing Agencies and Contractors to meet the goals of the Protocol. Payment to Consultant for its fees and costs shall be paid by all Participants as defined in the "Payment of Costs" section below.
- **Contractor** – Company, or companies, approved by the Receiver to engage in providing construction, samples, borings, security, or other support for execution of the Protocol. The Contractors shall be engaged by the Consultant. Payment for services shall be remitted to Consultant for payment to Contractors. Payment for Contractors' services shall be paid by all Participants as defined in the "Payment of Costs" section below.
- **Court** – Circuit Court of the 11th Judicial Circuit in and for Miami-Dade County, Florida.
- **Expert** – Engineers (or other disciplines) investigating the collapse on behalf of Participants.
- **Litigation** -- *In Re: Champlain Towers South Collapse Litigation*, Case No. 2021-015089-CA-01, pending in the Eleventh Judicial Circuit Court in and for Miami-Dade County, Florida
- **Participant** – A party or entity authorized or defined by the *Order Approving Protocol for Inspection, Documentation, and Storage of Components, Remnants, and Debris of The Champlain Towers South Collapse*. Notwithstanding anything to contrary herein, non-parties to the Litigation are not Participants; *however*, such non-parties may participate in the testing and investigations set forth in this Protocol if authorized to do so by the Court.
- **Protocol** – This document, titled *CHAMPLAIN TOWERS SOUTH COLLAPSE INVESTIGATION: Proposed Joint Protocol for Testing and Material Sampling – Collapse Site*.
- **Receiver** – Michael I. Goldberg, Esq., on behalf of the Champlain Towers South Condominium Association, Inc.
- **Testing Agency** – An independent company, approved by the Receiver and working under the direction of the Consultant and/or Receiver to facilitate surveying, sampling, borings, materials testing, and report factual results. The Testing Agency will use Contractors as necessary to carry out the Protocol. The Testing Agencies shall be engaged by the Consultant. Payment to Testing Agencies for their services shall be paid by all Participants as defined in the "Payment of Costs" section below.

GENERAL

Scope. The Protocol applies to the Champlain Towers South site. A separate or amended protocol for testing and materials sampling at the primary evidence facility will be developed later.

Objective. The Protocol is intended to provide reliable *factual* information needed to support the collapse investigations by the Experts, including but not necessarily limited to the following:

- Mechanical, physical, and chemical characteristics of construction materials
- Subsurface conditions, including soils, rocks, water, pile caps, and piles

- Physical configuration of structural and waterproofing components, including the plaza deck, perimeter foundation walls, shear walls, columns, and basement slab.

Reporting. It is envisioned that the Consultant, who does not represent Participants, will obtain this factual information. The factual information will be shared among Participants. Except as noted below, reporting to Participants shall include factual information only (without interpretation or opinion), including the following:

- Testing standards and methodology
- Documentation of the locations and extraction of samples for materials testing
- Test results in a format usable for engineering analysis and interpretation
- Given that geophysical testing requires interpretation to prepare result reports, one report for each geophysical test shall be prepared for the benefit of all parties. Geophysical consultants shall prepare a written narrative that shows the basis for any interpretation in each report.

Project Updates. The Consultant will conduct daily meetings at the end of each day at the site to discuss the work completed on that day and the work anticipated for the following day. Additionally, the Consultant will conduct weekly virtual meetings to discuss the work completed during that week and the multi-week look-ahead planned work. The Consultant will issue project updates weekly. The project updates will include:

- Field testing/sampling
 - Anticipated start dates and testing duration
 - At least one week of notice prior to commencement of testing
- Laboratory testing
 - Anticipated start dates and testing duration
 - At least one week of notice prior to commencement of testing
- Issues/deviations encountered at site
- Daily site visit reports (to be uploaded to a secure Extranet)

Photos and videotaping of site sampling operations; videotaping of laboratory testing shall be conducted and agreed upon for each laboratory test.

Extranet. The Consultant will provide data, photos, videos, and other factual information on a secure FTP/Sharepoint site (or equivalent).

Access to Site. The site is located at 8777 Collins Ave. in Surfside, Florida. The entrance and security check-in trailer are in the northeast corner of the site. Participants, Experts, Testing Agencies, Contractors, and other site visitors seeking access shall comply with the following:

- Submit all requests for access to the site through the Receiver.
- Notify the Receiver at least two (2) business days before the planned site visit.
- Sign a waiver releasing the Receiver of liability and provide proof of liability insurance coverage, naming the Receiver as additional insured.
- Comply with established procedures for signing in and signing out. These procedures include but are not limited to surrendering driver's licenses and wearing wristbands during each visit.

Observation of Site Sampling and Testing. Participants and Experts shall be permitted to observe sampling and testing conducted at the site and laboratories as agreed upon by all Participants prior to testing. For laboratory testing, virtual observation of testing will be provided and the ability to comment on test procedure for the first of any certain type of test, Participants shall be permitted to visit a lab and observe the first test. The first test of each test type shall also be videorecorded and promptly shared with the Participants for review and comments. The entirety of any test performed at a laboratory on a sample or material must be recorded by video if the sample or material either (i) cannot be reconstituted or retested, or (ii) will be altered by reason of the performance of the

test. Site testing can be observed in person, and any objections to the methodology can be raised on site, during daily meetings, and/or weekly virtual meetings.

Safety. All site visitors shall comply with state and federal safety regulations and the site safety protocol established by the Consultant. The Receiver may prohibit an individual from accessing the site in the event such individual fails to adhere to state and federal safety regulations and the site safety protocol established by the Receiver.

Samples. Except as permitted herein or expressly approved by the Receiver for additional testing, samples of any materials shall not be taken or removed from the site.

Reserved/Backup Samples. In addition to samples designated for testing under the Protocol, additional samples shall be taken and reserved for additional testing to address failed tests, ambiguous results, or disputed results. These samples shall be identified in the Testing Plans.

Testing Plans. Consultant and its Contractors and testing agencies shall prepare a detailed testing plan of each field and laboratory test to be performed meeting the requirements of this Protocol. The testing plans shall be shared for review by all Participants prior to executing the work.

If consensus is not reached, any dissenting Participant may bring the disagreement to the Receiver's attention for resolution. If a resolution is not achieved, counsel for the Participant may choose to bring the disagreement to the Court for resolution.

Chain of Custody. All sample extraction and preparation in the field will be documented by photo and videotaping as agreed upon by the Participants. Samples will be extracted, protected per appropriate testing standards, and labeled. A chain of custody form will be issued for each sample/specimen. Samples will be placed in appropriate containers for the intended laboratory testing. After testing, the samples will be stored at a secure location (TBD) and preserved for future testing or observation.

Additional or Specialized Testing. Each Participant may, if they choose, spend additional days on site after the joint testing is complete to take additional samples for private testing and analysis by a laboratory of their choosing or to conduct additional testing on site. The Consultant will notify the Participants when the sampling for the Protocol is complete and such additional testing can begin. If the services of the Consultant are needed for additional or specialized testing, Participants will submit additional or specialized testing requests in writing to the Consultant. The Consultant will coordinate sampling and scheduling of samples. Any such additional samples taken or testing conducted over the additional days on site shall occur immediately after work performed under this Protocol. The Consultant/Contractor will remain on site following the conclusion of this Protocol for any Participant that wishes to take additional samples. Any additional or specialized testing, or any portion of it, may be completed by the Participant's own consultants and contractors without the participation of the Consultant (1) should the Participant so choose at its discretion, or (2) should the Consultant and its contractors lack the capacity, expertise or preparation to accomplish the additional testing. The Receiver shall be consulted for the scheduling of any additional or specialized testing and the Receiver shall make the site available for such additional or specialized testing coordinated with the Consultant and the Receiver. If any non-party Participant wishes to conduct additional testing, leave of Court is required. All Participants may observe additional specialized testing at the site.

For geotechnical testing using the Consultant's services, any Participant that chooses to remain on site following the conclusion of this Protocol for the purpose of collecting additional samples will be limited to additional borings at locations to be identified and provided in advance to the Consultant to be conducted within the agreed upon period for additional testing. The location(s) where any additional samples are taken from will be appropriately documented by the Consultant. For concrete and rebar sampling, the Participants shall be authorized to take companion samples of material taken under this Protocol during the agreed upon period for additional testing or as required by the Consultant or, depending upon the sequencing of work, while joint geotechnical testing is underway provided that the taking of additional concrete and rebar sampling will not interfere with other work on site.

The Receiver will permit Non-Destructive Testing to be coordinated and proceed before sampling.

Payment of Costs. Except for the costs attributable to additional or specialized testing, which costs are the sole responsibility of the Participant seeking such additional or specialized testing, each Participant's or other third-party's allocation of the costs incurred through performance of this Protocol shall be determined pursuant to an order from the Court.

Audio and Video Recording. Some or all of the work performed pursuant to this Protocol will be recorded via video and/or audio. Although the purpose of the video and/or audio recordings is to document and record the work performed pursuant to this Protocol, it is understood that said video and/or audio recordings may also capture conversations or statements made by on-site Participants and/or the Consultant. It is expressly agreed between the Participants that any conversations or statements that are captured and/or recorded in the video and/or audio recordings of the work performed pursuant to this Protocol shall not be admissible at trial for any purpose and may not be used by any Participant in this litigation or any other legal or administrative proceeding, including but not limited to at depositions, in hearings, or in any filings.

Water Control. Until sampling and site testing is complete, the Receiver will continue to arrange for removal of excess water that accumulates on the lower-level slab from leakage and rain. The Consultant shall implement a comprehensive dewatering program throughout the entirety of the slab and as otherwise as necessary throughout the site.

General. Nothing in this Protocol shall serve to confirm authorization or access to conduct inspections or testing on any property or portion thereof that is not under the ownership and control of the Champlain Towers South Condominium Association (in the "CTS Property"). There may be several boring, test pits and other tests anticipated under this Protocol that are not located on the CTS Property and are subject to access authorized by the property owner and/or Court approval.

TESTING AGENCIES AND CONTRACTORS

Testing Agencies and Contractors extracting samples shall be named (including credentials) by the Consultant and selected by consensus among the Participants for the testing scope listed in Table 2. Laboratories performing the work shall be ISO 17025, AASHTO R 18 or USACE accredited (or acceptable equivalent) for the tests to be performed. Persons performing the work shall have documentation/training to perform the specific tests and/or tasks, (e.g., ACI certified laboratory technician performing compressive strength tests on concrete). All testing shall, at a minimum, meet the industry standard of care. The Consultant will coordinate the extraction and delivery of samples to Testing Agencies and collect and deliver testing reports to the Receiver. A Participant may request permission from the Receiver to visit (in person or virtually) one or more testing agency to examine samples and view testing procedures. Such visits will be coordinated by the Receiver and Consultant.

Table 1. Testing Agencies and Contractors

Specialty	Testing Scope	Testing Agency/Contractor [website]
Structural	Demolition	To be determined
	Coring	To be determined
	Surveying	To be determined
Surveyor	Surveying	To be determined
Laboratory Testing	Concrete	To be determined
	Metals	To be determined
Petrography	Petrography	To be determined
Non-Destructive Testing	Ground penetrating radar	To be determined
Geophysics	MASW/ER	To be determined
	Seismograph	To be determined

Specialty	Testing Scope	Testing Agency/Contractor [website]
Geotechnical	Borings and observation wells	To be determined
	Test pits	To be determined
	Pile integrity testing (PIT)	To be determined
	Crosshole seismic testing	To be determined
	Laboratory testing	To be determined
	Seismic CPT testing	To be determined
	Parallel seismic testing for deep foundations and sheet piles	To be determined

TESTING METHODS AND STANDARDS

The Consultant, Testing Agencies, and Contractors shall adhere to the following standards for tests carried out. Proposed deviations from the standards shall be brought to the attention of all Participants and Experts through the Receiver. In general, non-destructive testing will occur before sampling. In addition, the following should be noted:

- General descriptions of the test methods are provided in Appendix A.
- Proposed testing locations are provided in Appendix B.

Table 2. Testing Standards

Category	Test	Destructive	Standard/Guide
Concrete Materials	C1 Compressive strength	Yes	ASTM C39 and C42
	C2 Elastic modulus and Poisson's ratio	Yes	ASTM C469
	C3 Splitting tensile strength	Yes	ASTM C496
	C4 Air void parameters	Yes	ASTM C457
	C5 Petrography*	Yes	ASTM C856 and C1723
	C6 Chloride profile	Yes	ASTM C1218 and C1152
	C7 Ionic Species (e.g., sulphur, magnesium, sodium, and potassium) profile	Yes	ASTM C114
Reinforcing Steel	R1 Reinforcing bar location survey using ground penetrating radar (GPR)	No	ASTM D6432 and ACI 228.2R-13
	R2 Tensile properties	Yes	ASTM A615 and ASTM A370
	R3 Rockwell hardness	Yes	ASTM E18
	R4 Chemistry	Yes	ASTM E415, ASTM A751
	R5 Section Loss / Pitting	Yes	ASTM G46, ACI 364.14T-17
Geotechnical	G1 Soil/rock borings**	Yes	ASTM D1586, FDOT Soils and Foundation Handbook (as applicable) Participants request including pitcher samplers (ASTM D7015, ASTM D1587), X-ray of undisturbed samples (ASTM D4452), rock coring with inner spring barrel (ASTM D2113), RQD (D6032). Soil/rock characterization (ASTM D2487, D2488, and D5878). Permeability tests (ASTM D5912)
	G2 Observation well	Yes	ASTM D5092; ASTM D5912.
	G3 Test pit excavation	Yes	N/A
	G4 Pile integrity testing (PIT)	Yes	ASTM D5882
	G5 Crosshole seismic testing	No***	ASTM D4428
	G6 Grain size analysis	Yes	ASTM D6913, ASTM D7928
	G7 Moisture content	Yes	ASTM D2216
	G8 Organic content	Yes	ASTM D2974
	G9 Incremental consolidation	Yes	ASTM D2435
	G10 Chlorides	Yes	AASHTO T291
	G11 Electrical resistivity	Yes	ASTM G57
	G12 pH	Yes	ASTM D4972 or G51
	G13 Sulfates	Yes	ASTM D516
	G14 Sulfides	Yes	SM4500
	G15 Unconfined compression of rock	Yes	ASTM D7012C
	G16 Unit weight of Rock Cores	Yes	ASTM D7263
	G17 Specific Gravity	Yes	ASTM D854, ASTM D5778
	G18 Seismic CPT	Yes	ASTM D5778
	G19 Parallel Seismic Testing	Yes	ACI 228.2R and ASTM D8381
	G20 Drained and Undrained Triaxial Testing of Fine-Grained Soil Samples	Yes	ASTM D7181, ASTM D4767, ASTM D2850
	G21 Seismograph	No	N/A

Category	Test	Destructive	Standard/Guide
	G22 MASW/ER	No	NA
* Due to the nature of Petrographic testing samples will be provided to labs as requested by Participants and available			
**Includes SPT, and obtaining and testing of soil and rock samples			
***Uses borings; however, the test is non-destructive			

CONCRETE SAMPLING AND TESTING PLAN

The table below lists tests of concrete core samples. Final locations are to be determined by the consensus of the Experts and depend on site conditions and locations of interest and shall be provided to the Receiver at least seven (7) days prior to the commencement of the concrete core sampling.

Table 3. Concrete Sampling and Testing Plan

Core Sample Phase/Location	Element	Original Element Location	Number of Cores***	Test Type and Number (See Table 2)				
				C1 Comp.	C2 E & v	C3 Tens.	C4 & C5* Voids/Petro	C6 & C7 Cl & Ionic Species
Site	Slab	Basement	33	13	3**	3	3	3
Site	Perimeter Wall	Basement	33	13	3**	3	3	3
Site	Pile Cap	Below Basement Slab	22	6	4	4	4	0
Site	Pile	Below Pile Cap	22	6	4	4	4	0
Site	Pool Deck Slab (remaining area at SE corner of site, including all layers (i.e., structural slab, topping, waterproofing, pavers)	Pool Deck at Lobby Floor	8	3	1**	2	1	1
Site	Shear Walls	Gridline M at Basement and West Core	12	6	2**	0	2	0
Site	Columns (rubble within extant rebar)	Various columns on gridlines E, L, M, O, and Q	15	0	0	0	10	0
Site	Beams	Between Gridlines N-O and 8-10	9	6	2	0	2	0
Total***			154	53	19	16	29	7

*Tests C4 and C5 will be conducted on the same core sample; separate core samples are needed for all other tests.

**One of the three cores used for compressive strength will also be used to determine modulus of elasticity and Poisson's ratio.

***Collect spare samples at each location for backup/additional testing (Consultant/Testing Agency to provide plan)

REINFORCING STEEL SAMPLING AND TESTING PLAN

Table 4 lists the general location and number of samples. Final locations are to be determined by the consensus of the Experts and depend on site conditions and locations of interest and shall be provided to the Receiver at least seven (7) days prior to the commencement of the steel sampling.

Table 4. Reinforcement Sampling and Testing Plan

Bar Sample Location	Element	Original Element Location	Bar Size	Number of Bars*****	Test Type and Number 8 (See Table 2)				
					R1* GPR	R2** Tens.	R3** Hard.	R4** Chem.	R5 Sect. Loss
Site	Slab	Basement	#5	6	4	6	3	1	6
Site	Wall	Basement	#5	23	4	23	23	4	23
Site	Column	Basement	N/A	N/A	2	0	0	0	0
Site	East Shear Wall	Basement	#10/#11	0	N/A	0	0	0	0
Site	Slab	Pool Deck	#4/#5****	16	0	16	16	4	16
Site	South Wall	Pool Deck	#5*****	10	0	0	0	0	10
Site	Columns (rebar protruding through slab)	Type N Columns***	#7	3	N/A	3	3	1	3
Site	Columns (rebar protruding through slab)	Various Type A and L Columns	#10	3	N/A	3	3	1	3
Site	Columns (rebar protruding through slab)	Various Type C, D, and G Columns	#11	3	N/A	3	3	1	3
Site	Column	Various Ties from Columns	#3/#4/#5	3	N/A	3	3	1	3
Total				67	10	57	54	13	67

*Non-Destructive Testing (approximately 10 ft x 10 ft area)

**Tests R1, R2 and R3 to be conducted on the same bar sample

***No. 7 rebar remaining in the slab at columns that supported the pool deck will be over-cored and removed for inspection only.

****Two bars top and bottom bars in each direction

*****Exposed Vertical Bars, perform fractography examination of exposed bar ends.

*****Collect spare samples at each location for backup/additional testing (Consultant/Testing Agency to provide plan)

ADDITIONAL STRUCTURAL INSPECTION/SAMPLING PLAN

The following inspections and samples shall be taken by the Consultant and examined on site or in the lab as applicable:

1. Plaza pool deck investigative work, including five (5) exploratory borings in the overburden, examination, field testing, and sampling of the waterproofing for laboratory testing. NOTE: This waterproofing sampling, field testing, and exploratory work must happen before destructive work on the underlying structure.
2. Pool deck slab N/S construction joint probe:
 - a. Provide additional pool deck slab shoring as required;
 - b. Shore top edge of adjoining privacy wall against lateral out-of-plane movement as required;
 - c. Locate all bars passing through construction joint along a 4 ft length of construction joint;
 - d. In the presence of all interested parties and under video observation, sawcut and remove a 4 ft (north-south) x 3 ft (east-west) slab of the existing pool deck slab and topping slab along the joint, taking care not to damage or drop the slab during sawcutting and removal;
 - e. Two (2) sawcut openings in the west edge of the remaining pool deck plaza to expose the cold joint assembly;
 - f. One (1) sawcut opening in the west edge of the remaining pool deck plaza to expose the slab-to-hot tub wall connection;
 - g. One (1) sawcut in the north edge of the remaining pool deck plaza to expose the slab-to-corbel connection;
 - h. Seal the preserved slab in heavy plastic and set aside for retrieval by laboratory for controlled and video-documented deconstruction; and
 - i. Measure and photo document the remaining sawcut cross section of the construction joint.
3. Privacy wall probe:
 - a. Provide additional pool deck slab shoring as required;
 - b. Shore top edge of adjoining privacy wall against lateral out-of-plan movement as required;
 - c. In the location where the pool deck slab is remaining west of the pool, GPR a 4 ft wide section (wider if necessary) of the CMU wall to locate the wall vertical bar(s) and dowels,
 - d. Make two vertical sawcuts, 4 ft apart and capturing the location of the vertical bar(s), to isolate CMU wall to be inspected/demolished from remainder of privacy wall;
 - e. In the presence of all interested parties and under video observation, sawcut and remove a 3 ft (east-west) x 3 ft (tall) portion of the existing wall, taking care not to damage or drop the wall during sawcutting and removal;
 - f. Seal the preserved wall portion in heavy plastic and set aside for retrieval by laboratory for controlled and video-documented deconstruction; and
 - g. Measure and photo document the remaining sawcut cross section of the wall.
4. South foundation wall to pool deck slab joint probe:
 - a. Provide additional pool deck slab shoring as required;
 - b. Shore top edge of adjoining privacy wall against lateral out-of-plan movement as required;
 - c. Within the 4 ft wide privacy wall probe, GPR a 3 ft x 3 ft area of the pool deck slab and a 3 ft x 3 ft area of the supporting wall to locate the rebar;
 - d. In the presence of all interested parties and under video surveillance, sawcut a 3 ft (north-south) x 3 ft (east-west) slab of the existing pool deck slab and topping slab along the joint, taking care not to damage or drop the slab during sawcutting;
 - e. After sawcutting through the slab, in the presence of all interested parties and under video observation, continue to sawcut the supporting wall and remove the L-shaped portion of the slab-to-wall connection as shown in page 6 of the attached figures;
 - f. Seal the preserved wall portion in heavy plastic and set aside for retrieval by laboratory for controlled and video-documented deconstruction;
 - g. Measure and photo document the remaining sawcut cross section of the slab and wall;
 - h. Provide six (6) test pits (as shown on SK-03) to expose the beach access walkway assembly and the top of the south perimeter sheet pile wall driven for the construction of 8777 Collins Ave. Note,

- these are not test pits to the bottom of the foundation wall;
- i. Provide six (6) exploratory openings in the exposed interior face of the southern perimeter basement wall that align with the test pits in item (h). Exploratory openings shall expose the sheet pile wall and embedded steel reinforcement; and
 - j. Visually survey and use a carpenter/mason's hammer sound the exposed interior face of the wall to map cracks and detect delamination as described in ASTM D4580. At delaminated locations perform selective demolition of the concrete perimeter wall from the basement interior to expose the sheet pile wall and embedded steel reinforcement. Remove one (1) sample of embedded reinforcement steel and sheet pile wall at each opening location.
5. Measure the depth of the foundation wall sheet piling at two locations along the south wall using parallel seismic testing (cross-reference test G19);
 6. Survey top edge geometry of sheet piling at 20 ft on center maximum, capturing all abrupt elevation changes (absolute elevation or elevation relative to bottom of pool deck slab):
 - a. Where required, chip the concrete to expose the top of steel sheet pile;
 - b. Optically survey the top of the sheet piles at 20 ft on center maximum, capturing all abrupt elevation changes;
 - c. Optically survey the elevation of the bottom of pool deck slab near the pool;
 - d. Reference the surveyed elevations to a datum.
 7. Take 1 ft x 1 ft samples of sheet pile at two locations along the south wall for measuring thickness, material properties, and corrosion quantity and quality (uniform loss or pitting);
 8. Chip concrete at each shear wall boundary zone to expose boundary element ties for observation and documentation by all interested parties;
 9. Provide test pit at undocumented column at grid O/8.5;
 10. Provide two (2) exploratory openings to expose the top of the west perimeter sheet pile wall driven for the construction of 8777 Collins Ave;
 11. Provide two (2) exploratory openings to expose the top of the north perimeter sheet pile wall driven for the construction of 8777 Collins Ave;
 12. Provide two (2) exploratory openings to expose the top of the east perimeter sheet pile wall driven for the construction of 8777 Collins Ave;
 13. Provide two (2) exploratory openings to expose the slab-to-wall connection at the bottom of the south perimeter basement wall.
 14. Provide one (1) exploratory opening to expose the slab-to-wall connection at the bottom of the north, west and east perimeter basement wall; and
 15. Perform level survey of the basement slab on grade capturing slab elevation at all column locations, adjoining location at the perimeter wall as applicable, shear walls, and the midspans of bays. In addition, the surveyed elevations to a datum must be referenced.

GEOTECHNICAL INVESTIGATION PLAN

The following geotechnical plan is developed as a basis for understanding site conditions and the foundation system. The test details and standards are to be developed based on the Experts' input and the Consultant engaged in carrying out the plan. The table lists general locations of borings and tests/samples. Final locations are to be determined by consensus of the Participants and will depend on site conditions and locations of interest. Details on sampling within the borings are shown in the attached plan (Appendix B) and described in section *G1 Soil/Rock Borings* (Appendix A).

The locations and numbers of tests for G6-G17 will be recommended by the Experts in consultation with the Consultant and approved by the Receiver and/or the Court. Note: Nothing in this Protocol shall serve to confirm authorization or access to conduct inspections or testing on any property or portion thereof that is not CTS Property. There may be several boring and test pits anticipated under this Protocol that are not located on the CTS Property and are subject to access authorized by the property owner and/or Court approval. The Protocol schedule shall be adjusted if circumstances outside of the Receiver's, the Consultant's, or the Participants' control, such as, but not limited to, permitting delays, access to adjoining properties, contractors' availability, equipment and material shortages, damage to samples collected, weather events, or COVID-related issues, materially impact the schedule.

Table 5. Geotechnical Sampling and Testing Plan – Receiver to coordinate any request for additional testing by site purchaser

Sample/Test Location	Test Type and Number*** (See Table 2)						
	G1 Borings	G2 Obs. Wells	G3* Test Pits	G4 PIT	G5 Crosshole Seis.	G18 CPT	G19 PSL
Slab Corners	0	0	0	0	0	0	0
Within Building Footprint	33	6	23	22	5	12	0
Outside Building Footprint	7	1	5	5	0	2	4
Total	40	7	28	27	5	14	4

*Core drill through pile cap and full length of pile

**Non-Destructive Testing

SCHEDULING AND ESTIMATED TIME REQUIRED ONSITE

The table below shows the estimated start date and time on site required for sampling and testing.

Table 6. Estimated testing start date and time required onsite

Sampling/Test	Estimated Start Date	Estimated Time Required on Site
Concrete Sampling	TBD	TBD
Steel Sampling	TBD	TBD
Concrete/Steel Laboratory Testing	TBD	TBD
G1 Soil Borings, G2 Observation Wells, G5 Crosshole Seismic, G18 Seismic CPT, G20	TBD	TBD

G3 Test Pits, G4 Pile Integrity Testing, G19 Parallel Seismic	TBD	TBD
G6-G17,G20 Laboratory Testing	TBD	TBD
G21, G22 MASW/ER, Seismographs	TBD	TBD

APPENDIX A

This appendix is intended to provide a brief description and outline requirements for each testing method in the Protocol. These descriptions should not be considered comprehensive and Participants should consult with their Experts for further information. Note: for all testing, the most recent and applicable testing standards and guides are to be used.

SITE SAFETY

The safety and wellbeing of all individuals on site, as well as the safety and wellbeing of those that may be impacted by the work performed under this Protocol shall at all times be the first and foremost priority of all individuals and companies involved in the performance of work. All work shall be performed in accordance with all local, state and federal rules and safety regulations and guidelines, including OSHA. Consultant will develop a site safety plan for the work which anyone entering the site must accept and adhere to. If anything in this Protocol calls for work to be done in a manner deemed unsafe by the Consultant or any Contractor, then said Consultant or Contractor shall not proceed with the work and all parties shall discuss how to proceed more safely. The Participants shall not be responsible for the implementation for any safety program nor have any responsibility for site safety, with the exception of making sure that they themselves comply with the safety program implemented by the Consultant. The Consultant may request that Receiver expel a Contractor or Participant that violates the safety plan.

CONCRETE TESTING

Concrete Core Removal

All core samples shall be taken using a 4-inch diameter core barrel (core diameter = \pm 3.7-inch) or largest barrel suitable to fit between bars at a specific location. Each sample shall be marked, measured (diameter and overall length), described, and photographed, including the size and condition of any embedded reinforcement.

Core samples shall be taken and conditioned per ASTM C42. Before coring, the surface shall be sounded with a hammer as described by ASTM D4580, and reinforcing bars in the vicinity shall be located using ground penetrating radar (GPR) or pachymeter. Cores for petrographic examination will be taken through near-surface reinforcing bars, especially where delamination planes have been identified by sounding. Otherwise, coring operations should avoid reinforcement. Core holes shall be inspected to determine if any cracks in the core samples are also present in the concrete. Cores intended for compression testing or splitting tensile testing shall be retaken if the cores include reinforcement, large voids, or cracks that preclude obtaining a 6-inch long sawn sample without such characteristics. After cores have been drilled, wipe off surface drill water and allow remaining surface moisture to evaporate. When surfaces appear dry, but not later than 1 hour after drilling, place cores in separate plastic bags or nonabsorbent containers, seal to prevent moisture loss, and label for identification.

Core holes through the Level 1 slab and basement wall shall be patched as required to prevent groundwater entry. Table 4 lists the general location and number of cores. Concrete core samples will be extracted with a diamond impregnated, water-cooled core drill. After removal, the core holes will be photographed and then patched with a rapid-setting, cementitious repair material. The condition and location of each core will be documented with photographs and notes.

C1 Compressive Strength

To determine concrete compressive strength, concrete cores shall be removed using a coring drill, trimmed, prepared for testing using sulfur capping, and loaded in a loading frame at a rate within a specified range. The compressive strength of the specimen will be calculated by dividing the maximum axial load sustained by the sample by the cross-sectional area of the specimen. Concrete core samples will be prepared and tested according to ASTM

C42 *Standard Test Method for Obtaining and Testing Drilled Cores and Sawed Beams of Concrete* and ASTM C39 *Standard Test Method for Compressive Strength of Cylindrical Concrete Specimens*

C2 Elastic Modulus and Poisson's Ratio

These parameters will be measured using appropriate instrumentation in a loading frame during compressive loading of the specimens. Elastic modulus and Poisson's ratio will be measured according to ASTM C469 *Standard Test Method for Static Modulus of Elasticity and Poisson's Ratio of Concrete in Compression*.

C3 Splitting Tensile Strength

Splitting tensile strength testing provides measurements to calculate the concrete tensile strength of the tested specimen. This test method consists of applying a diametral compressive force at a rate within a prescribed range along the length of a concrete core specimen in a load frame until failure occurs. The maximum load sustained by the specimen is divided by appropriate geometrical factors to obtain the splitting tensile strength. Splitting tensile strength will be measured in accordance with ASTM C496 *Standard Test Method for Splitting Tensile Strength of Cylindrical Concrete Specimens*.

C4 Air Void Parameters

Determining air void parameters involves examining a concrete cross-section under a microscope. One of three procedures will be used to determine air void parameters, including specific surface, void frequency, spacing factor, and paste air ratio of the air-void system in hardened concrete:

1. Linear-traverse method
2. Modified point-count method
3. Contrast-enhanced method

Air void parameters will be determined in accordance with ASTM C457 *Standard Test Method for Microscopical Determination of Parameters of the Air-Void System in Hardened Concrete*.

C5 Petrography

A petrographic examination consists of a series of observations of hardened samples that are interpreted to draw conclusions about the composition, quality, water to cement ratio, and probable cause(s) of potential problems associated with concrete or other construction materials. To conduct a petrographic examination, a core sample will generally be sliced in half lengthwise. One of the resultant halves will be lapped (polished) to accentuate the appearance of the components of the concrete (air voids, aggregates, and cement paste). Additional thin sections and smaller polished sections can also be made for petrographic microscope and scanning electron microscope (SEM) evaluations of cement paste and including microscopic chemical analysis.

A powder mount is obtained by scratching off a very small portion of the paste and mounting the resultant powder on a glass microscope slide in immersion liquid. Examination of a powder mount allows the properties of the cement paste to be studied in detail. Petrographic examination will be carried out in accordance with ASTM C856 *Standard Practice for Petrographic Examination of the Hardened Concrete* and ASTM C1723 *Standard Guide for Examination of Hardened Concrete Using Scanning Electron Microscopy*.

All samples and epoxied discs shall be preserved for review by other experts if needed and access granted when requested. The petrographic report shall include descriptions of the method(s) of preparation of the samples and full documentation of the images and other aspects of the samples that the lab personnel have based their interpretations on. The preparation and cutting of the samples by the lab technicians shall be video recorded.

C6 Chloride Profile

Chloride ion content testing will be performed on selected core samples to determine whether chloride contamination from salts has accumulated to sufficient levels to promote corrosion of reinforcing steel. Selected core samples will be cut into slices at pre-determined depths, and the slices will be pulverized to facilitate chloride

content analysis in general conformance with ASTM C1152 *Test Method for Acid-Soluble Chloride in Mortar and Concrete* and ASTM C1218 *Standard Test Method for Water-Soluble Chloride in Mortar and Concrete*. The acid-soluble chloride content represents both water-soluble and chemically-bound chloride within the cement paste and any chloride that may be present in the aggregate. The water-soluble chloride content represents only the chlorides in the cement paste and aggregates that are soluble in water. Slices will be taken at depth prescribed by ASTM C1556 *Standard Test Method for Determining the Apparent Chloride Diffusion Coefficient of Cementitious Mixtures by Bulk Diffusion*, Table 1. Table 1 in C1556 describes the depth at which slices should be taken given a known water to cement ratio. Thus, the water to cement ratio should be determined from petrographic analysis and then used to determine the number of slices to be analyzed chloride and other ionic species.

C7 Ionic Species Profile

The same powder used for analysis of chloride content by ASTM C1218 and ASTM C1152 can be used to determine the content of sulfur, magnesium, sodium, and potassium found in the concrete by utilizing testing methods described in ASTM C114 *Standard Test Methods for Chemical Analysis of Hydraulic Cement*. In fact, C1152 and C1218 rely on the method for chloride analysis in C114. As these elements are part of the ocean salts or cement paste, changes in depth and concrete location can be useful in pinpointing deterioration mechanisms.

REINFORCING STEEL TESTING

Reinforcing Steel Sample Removal

All reinforcing steel samples shall be extracted such that the samples are straight and undamaged by chipping hammers or other removal equipment. Deep saw cutting along and perpendicular to the bar is envisioned. Samples shall be at least 36 inches long. Each extracted sample shall be measured and photographed. Repair of sample locations may be necessary to prevent water intrusion.

R1 Reinforcing Bar Location Survey Using Ground Penetrating Radar (GPR) – Non-Destructive

To determine location (depth and spacing) and concrete cover of the reinforcing steel, ground penetrating radar (GPR) will be used. In addition to the non-destructive testing, limited destructive measures (e.g., drilling small ~1/2 in. diameter holes) will be used for correlation and calibration. GPR involves the use of a high-frequency radar antenna, which transmits electromagnetic radar pulses along with a discrete longitudinal scan at the surface of a structural element. Electromagnetic signals reflected from material interfaces having different dielectric properties (i.e., reinforcing steel embedded in concrete) are collected by the antennae and displayed graphically for direct interpretation in the field. Guidelines for GPR considered during this work included ACI 228.2R-13 *Report on Nondestructive Test Methods for Evaluation of Concrete in Structures* and ASTM D6432 *Standard Guide for Using the Surface Ground Penetrating Radar Method for Subsurface Investigation*.

R2 Tensile Properties

Tensile properties of reinforcing bar specimens include yield strength, ultimate tensile strength, elongation, and elastic modulus. Tensile properties will be determined by testing as-rolled bar specimens in a load frame. Testing of tensile property shall be carried out in accordance with the applicable portions of ASTM A615 *Standard Specification for Deformed and Plain Carbon-Steel Bars for Concrete Reinforcement* and ASTM A370 *Standard Test Methods and Definitions for Mechanical Testing of Steel Products*.

R3 Rockwell Hardness

The Rockwell hardness test is an indentation hardness test that involves the use of a verified machine to force a diamond spheroconical indenter or tungsten carbide (or steel) ball indenter into the surface of a material. Hardness is determined based on the size of the indentation. Rockwell hardness testing will be carried out in accordance with ASTM E18 *Standard Test Methods for Rockwell Hardness of Metallic Materials*.

R4 Chemistry

A spectrometric analysis will be used to determine the quantity of carbon, iron, and residual elements in carbon steel (copper, nickel, arsenic, lead, tin, chromium, etc.) for the determination of compliance with compositional specifications (in this case, ASTM A615). Testing will be carried out in accordance with ASTM E415 *Standard Test Method for Analysis of Carbon and Low-Alloy Steel by Spark Atomic Emission Spectrometry*.

R5 Section Loss/Pitting

The extent of section loss and pitting on samples (particularly those tested in R2) will be determined according to ASTM G46 *Standard Guide for Examination and Evaluation of Pitting Corrosion* and ACI 364.14T-17 *Section Loss Determination of Damaged or Corroded Reinforcing Steel Bars*.

GEOTECHNICAL INVESTIGATION

G1 Soil/Rock Borings

The following geotechnical investigation is proposed:

- Forty (40) borings with approximate locations as annotated in the attached Appendix B are proposed. Borings SPT-8, SPT-9, SPT-10 and SPT 14, which lie outside the perimeter of the Champlain tower walls to the south, will be performed with a smaller rig that fits in the narrow passageway.
- All borings are generally expected to extend to a rock layer at about 80 feet deep unless determined otherwise by the Participants.
- Boreholes for sampling shall be kept full of drilling fluid at all times. If needed, bentonite or suitable alternative should be added to keep the borehole full of drilling fluid and avoid borehole squeeze.
- The driller will open the SPT sampler and allow Participants to visually inspect the contents. Samples from the SPT sampler will be collected at various depths within the boreholes as directed by the Consultant and sealed in watertight containers. The drilling contractor will perform SPT testing and obtain samples at a maximum interval of every 5 feet, or as otherwise directed by the Participants. SPT samples will be collected continuously as directed by the Participants.
- Samples of the interbedded limestone/sand stratum will be obtained using Pitcher samplers in addition to split-spoon samples during the initiation of the subsurface investigation. The Consultant, in agreement with the Participants, shall evaluate and determine the need for subsequent Pitcher sampling as the subsurface investigation continues; however, pitcher sampling shall not be eliminated and shall be incorporated as a supplement in select borings to provide a comparison between split-spoon samples and pitcher samples. An alternative to the Pitcher sampler may be the use of a triple-tube core barrel to increase sample recovery and quality.
- The drilling subcontractor shall use methods and procedures to maximize core recovery. Coring shall be in accordance with FDOT standards. In addition, coring should be continuous, whether 5-ft or 10-ft core runs are used.
- Samples of soil and rock cores will be retrieved and shipped for further laboratory testing at a mutually agreed upon geotechnical testing laboratory. The Consultant will develop a protocol for adequate sample preservation and shipping.
- All Participants will be allowed to visually observe the joint investigation. Participants will not direct the drilling contractor for the forty (40) joint borings. All soil and rock samples will be allowed to be visually evaluated by all Participants on-site during the investigation.

The forty (40) boring locations are to be drilled by the same mutually agreed upon drilling contractor who will work with the Consultant to observe, log samples, and provide investigation reports. The drilling contractor shall provide sample boring and drilling logs and list of items to be included in the visual description for review by Participants. The Consultant will provide a sample boring log also for review by Participants. The Consultant will provide reports including, but not limited to, visual-manual soil identification per ASTM D2487 and D2488, SPT blow counts, RQD

and water level observations. The same contractor shall remain on site after the completion of the 40 borings so each Participant can perform additional tests, if so requested, for an agreed upon duration of time. The testing performed by each Participant after the first 40 borings can be visually observed by all Participants; however, the samples extracted will belong to the specific Participant coordinating the testing and can be destructively tested as needed.

In the standard penetration test (SPT) procedure, a 2-inch outer diameter split-barrel sampler is driven into the soil by repeatedly dropping a 140-pound hammer from a height of 30 inches. The length of the samplers can be 18 or 24 inches. The number of blows required to advance the sampler every 6 inches are recorded. The sum of the second and third blows is recorded as the SPT N values for that depth. SPT N values are utilized to correlate strength parameters for the soils. Accompanying energy measurements will be reported. Soil and rock borings shall be carried out in accordance with ASTM D1586 *Standard Test Method for Standard Penetration Test (SPT) and Split-Barrel Sampling of Soils*.

- Soil borings will be advanced using the standard penetration test (SPT) procedure described above unless otherwise directed by the Consultant's geologist or engineer. The test provides samples for identification purposes and provides a measure of penetration resistance which can be used for geotechnical design purposes. The standard penetration test recovers a highly disturbed sample, which is generally not suitable for tests that measure properties of the in-situ soil structure, such as density, strength, and consolidation characteristics. The samples will be labeled, sealed, and transported to a laboratory for further index and classification testing. If fine-grained material is encountered, thin-walled tube samples should be used for obtaining intact specimens of fine-grained soils for laboratory tests to determine engineering properties. Similarly, if soils containing cohesive materials (clays) are encountered, thin-walled tube sampling should be performed to obtain minimally-disturbed samples. Drained and undrained triaxial testing will be performed on any samples of clay to establish strength parameters.
- The Consultant should provide draft investigation logs every week or sooner of all work completed the prior week.

G2 Observation Wells and Water Quality Samples

A standpipe will be installed after the completion of specified boreholes. Observation wells should be screened in separate strata. Certain observation wells should be screened in the sand stratum above the interbedded limestone/sand, and others should be screened in the interbedded limestone/sand stratum itself. These standpipes will be monitored for groundwater levels in the subsequent days of the investigation. An electronic transducer connected to a datalogger shall be installed in each well. The groundwater should be monitored at least six times daily until it is determined by Participants that it can be stopped, to develop a record of seasonal and other variations. A barometer shall be installed at the site. Sufficient dataloggers shall be installed at the site to read all installed sensors with provisions for the data to be provided to the Participants at regular intervals. Water quality samples will be collected and sent to an approved laboratory for testing. Field permeability testing in the sand and the interbedded limestone/sand strata in general shall be performed accordance with ASTM D5912. The Participants shall identify the location of each field permeability test. The Participants shall fully develop the requirements and details for the observation wells after the Consultant obtains subsurface information from the borings.

Monitoring wells will be installed at the locations shown on Appendix B and any other location identified by the Participants to determine long-term, stabilized groundwater readings. A slotted pipe will be installed within the borehole and packed with sand on the outside to ensure the slots in the pipe are not clogged. A bentonite seal shall be established above the top of the screened zone. In addition, grout will be placed near the top of the pipe to ensure surface water does not percolate into the observation well. The top of the pipe will be capped and sealed to ensure precipitation does not enter the observation well. Observation wells will be installed and monitored in accordance with ASTM D5092 *Standard Practice for Design and Installation of Groundwater Monitoring Wells*.

Samples at 10-ft intervals within the borehole shall be obtained for the purposes of water quality sample testing. Samples shall be obtained in accordance with the FDOT Soils and Foundation Handbook for investigations for Structures and shall be tested in accordance with Section 4.10 – Environmental Corrosion Test Guidelines.

G3 Test Pit Excavation

Two (2) test pit excavations (TP-5 and TP-17) and full-length cores at the piles identified as TP-5 and TP-17 on Appendix B shall be performed prior to commencement of any borings in order to establish the lengths of the borings. The remainder of the test pits shall be performed after the borings are completed.

The structural slab and remaining column shall be extracted labeled and set aside for inspection and additional sampling if necessary. Care should be taken to remove the column and structural slab as to minimize damage.

Test pits shall be sufficiently deep to expose the pile caps and the top 2 to 3 feet of the pile. At each test pit, core drill or saw cut through the pile cap and portion of the pile to extract samples and permit pile integrity testing. Test pits shall be dewatered as required to permit observation and pile integrity testing. Contractor shall cut through concrete, remove materials and provide needed dewatering under the direction of the Consultant. At the basement wall, test pits shall be sufficiently deep to expose or extend to the elevation of the bottom of the foundation wall.

Walkway test pits shall be sufficiently deep to expose or extend to the elevation of the bottom of the foundation wall. Test pits shall be dewatered as required to permit observation. Contractor shall cut through concrete, remove materials and provide needed dewatering under the direction of the designated engineer/geologist.

The Consultant will engineer each test pit excavation for safe access. The test pit designs will be shared with all parties. OSHA standards will be complied with in relation to design, execution and access.

G4 Pile Integrity Testing (PIT)

PIT Testing is performed by placing accelerometers at several locations on the pile and then hitting the pile or as necessary to obtain conclusive and accurate test results, with different size hammers and then measuring the response of the pile to the impact. The results are used to estimate the length of the pile. For higher quality of results, it is recommended to access the pile at the top and the side. Pile integrity testing will be performed in accordance with ASTM D5882 *Standard Test Method for Low Strain Impact Integrity Testing of Deep Foundations*.

PIT will be performed by an agreed upon subcontractor. The PIT subcontractor will work under the direction of the Consultant and, subsequent to the testing, will provide raw data and interpretive reports to all Participants. Depending on the data available for the design and as-built length of piles, PIT must be performed a few days prior to the geotechnical boring investigation for all Phase 1 test pits to determine the necessary depth of borings. Sufficient time will be provided to allow interpretation of PIT data and if inconclusive results are obtained, alternative testing techniques should be implemented. Refer to G19 Parallel Seismic logging.

G5 Crosshole Seismic Testing

Crosshole seismic testing will be performed between boreholes as directed during the field investigation as identified in Appendix B. Crosshole Seismic Testing will measure compression and shear waves between two adjacent boreholes at various depths according to ASTM D4428. The Consultant will determine the final holes to be tested based on findings of the borehole sampling and initial test results. Additional borings shall be made if necessary to satisfy the requirements of ASTM D4428. A crosshole testing plan will be provided by the consultant prior to execution.

G6 through G17, Laboratory Testing

Select samples from the field investigation of all joint borings and the test pits will be tested in a commonly agreed upon laboratory proposed by the Consultant. Specific tests and testing locations will be agreed upon on a rolling basis by all Participants as the field work is completed, and drilling logs have been provided. Reasonable efforts will

be made depending on available sample quantities to provide every requested test. Participants may request to use samples that were not selected for laboratory testing as part of this Protocol, for laboratory testing at their own expenses. The Consultant shall provide weekly drilling/investigation logs and testing will be agreed upon and ordered every week based on the field observations and the drill logs.

G6 Grain Size Analysis

Soils consist of particles of various shapes and sizes. The particle-size distribution (gradation) of soil can be determined from representative specimen following ASTM D6913 *Standard Test Methods for Particle-Size Distribution (Gradation) of Soils Using Sieve Analysis*. In cases where the gradation of particles smaller than No. 200 (75- μ m) sieve is needed, *Test Method D7928* may be used. All samples subjected to grain size evaluation with more than 10% fines should include hydrometer testing such that the D10 of the material can be determined for estimation of permeability.

G7 Moisture content

The moisture content of soil and rock can be determined in the laboratory by drying the material and measuring the reduction in mass of the material. The water content is the ratio of the mass of water contained in the pore spaces of soil or rock material to the solid mass of particles expressed as a percentage. The test is performed according to ASTM D2216 *Standard Test Methods for Laboratory Determination of Water (Moisture) Content of Soil and Rock by Mass*.

G8 Organic Content

Percentage of organic matter is important for classifying or evaluating peat or other organic soil, and for geotechnical and general soil classification purposes. This test can be used to determine the moisture content, ash content, and percent organic matter in the soil. The test is performed according to ASTM D2974 *Standard Test Methods for Moisture, Ash, and Organic Matter of Peat and Other Organic Soils*.

G9 Incremental Consolidation

The magnitude and rate of consolidation of fine-grained soil can be determined when it is restrained laterally and drained axially while subjected to incrementally applied controlled-stress loading. The test is performed on undisturbed soil samples in accordance with ASTM D2435 *Standard Test Methods for One-Dimensional Consolidation Properties of Soils Using Incremental Loading*. Loading increments specified for the lab will need to be agreed upon by all Participants.

At the request of any Participant, testing will be performed on reconstituted samples of sand to obtain stiffness parameters.

G10 Electrical Resistivity

Electrical resistivity for representative samples will be determined in situ and in the laboratory. Measurement of soil resistivity is used for the control of corrosion of buried structures and for the estimation of expected corrosion rates. The test will be performed according to ASTM G57 *Standard Test Method for Field Measurement of Soil Resistivity Using the Wenner Four-Electrode Method*.

G11 Chlorides, G12 pH, G13 Sulfates, G14 Sulfides

This group of tests is performed in a laboratory setting to determine chloride ion, sulfate ion, sulfide content, and pH level in the representative samples. The principal use of the pH test is to supplement soil resistivity measurements and thereby identify conditions under which the corrosion of metals in soil may be accentuated. The individual tests will be performed according to ASTM D512 *Standard Test Methods for Chloride Ion in Water*, ASTM D516 *Standard Test Method for Sulfate Ion in Water*, SM 4500 *Sulfide by Standard Methods Committee*, and G51 *Standard Test*

Method for Measuring pH of Soil for Use in Corrosion Testing. Water samples from bailed observation wells for groundwater chemistry testing shall be obtained.

G15 Unconfined Compression of Rock

The strength of intact rock core specimens in uniaxial compression will be determined following ASTM D7012C *Standard Test Methods for Compressive Strength and Elastic Moduli of Intact Rock Core Specimens under Varying States of Stress and Temperatures*. In the laboratory, a rock core specimen is cut to length and the ends are machined flat if possible and if not possible, as otherwise determined by the Participants. The specimen is placed in a loading frame, where the axial load on the specimen is increased and measured continuously until the sample fails. The uniaxial compressive strength is calculated as the maximum load at failure divided by the cross-sectional area of the specimen. Adequate instrumentation shall be included to determine Young's modulus.

G16 Unit weight of Rock Cores

Unit weight of rock cores will be determined in general accordance with ASTM D7263 *Standard Test Methods for Laboratory Determination of Density and Unit Weight of Soil Specimens*, method B (direct measurement). For a rock core specimen of cylindrical shape, the dimensions and mass of a specimen are measured. The density and unit weight are then calculated using these direct measurements.

G17 Specific Gravity

Specific gravity of soil samples will be determined in general accordance with ASTM D854, *Standard Test Methods for Specific Gravity of Soil Solids by Water Pycnometer*. These test methods cover the determination of the specific gravity of soil solids passing a sieve using a water pycnometer. These test methods are not viable for solids that can be altered by these methods, are contaminated with a substance that prohibits the use of these methods, or are highly organic (such as fibrous matter which floats in water).

G18 Seismic Cone Penetration Testing (CPT)

Seismic cone penetration testing estimates the small strain shear modulus of the soil by measuring shear wave velocities in the ground. The seismic CPT testing is performed as part of CPTU testing. The CPTU test will additionally measure continuous point and frictional resistance during penetration of the cone, pore pressures, and dissipation of pore pressures with time at constant depths. This testing will be carried out in general accordance with ASTM D5778 *Standard Test Method for Electronic Friction Cone and Piezocone Penetration Testing of Soils*.

The following geotechnical investigation is proposed:

- Seismic CPTs capped at 400-500 tsf with approximate locations as annotated in the attached Appendix B.
- CPTs will be pushed until refusal.
- Continuous measurements of point resistance, frictional resistance, pore pressure and shear wave velocity will be performed while the cone is pushed at a constant rate of 2 cm/sec. Three dissipation tests for each CPT location will be performed to establish groundwater and dissipation properties at those depths as directed by the Consultant.
- No sample recovery is performed with this testing.

The Seismic CPTs will be performed by the same mutually agreed upon contractor who will work with Consultant to observe and provide investigation reports. The Seismic CPT contractor will provide both raw data and reports including, but not limited to, cone tip resistance, frictional resistance, pore pressure measurement with depth, and pore pressure measurements with time during dissipation testing. Permeability values determined from pore pressure dissipation tests will be included.

G19 Parallel Seismic Logging

Parallel Seismic Logging (PSL) will be performed at indicated locations as annotated in the attached Appendix B. Boreholes will have casing and grout installed for the PSL testing that will be performed subsequent to the drilling activities. This testing involves sending a transceiver down the cased borehole to measure the depth of the buried sheet pile wall and will be carried out in general accordance with ASTM D8381 *Standard Test Methods for Measuring the Depth of Deep Foundations by Parallel Seismic Logging*.

Parallel seismic logging shall be accomplished as a supplemental means of determination of foundation lengths in the event that PIT interpretations are deemed inconclusive.

G20 Drained and Undrained Triaxial Testing of Fine-Grained Soil Samples

If fine-grained material is encountered and if thin-walled tube samples are used for obtaining intact specimens of fine-grained soils for laboratory tests to determine engineering properties, then triaxial testing will be performed on the specimens. This testing will be carried out in general accordance with ASTM D7181 *Standard Method for Consolidated Drained Triaxial Compression Test for Soils*, ASTM D4767 *Standard Test Method for Consolidated Undrained Triaxial Compression Test for Cohesive Soils*, and/or ASTM D2850 *Standard Test Method for Unconsolidated-Undrained Triaxial Compression Test on Cohesive Soils*.

At the request of any Participant, testing will be performed on reconstituted samples of sand to obtain shear strength parameters.

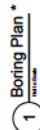
The Participants shall endeavor, but are not required, to agree on the Protocol of testing G20 including initial stress state before shearing, stress levels, strain rate, and drainage conditions during testing.

G21 Seismograph

Vibration monitoring shall be accomplished via a calibrated seismograph throughout the duration of all work accomplished on site at various locations, including these shown on Appendix B, with the monitors installed per location at different distances. The vibration monitoring shall start one week prior to the beginning of testing under the Protocol to establish a baseline at a trigger level of 0.05 in/s or lower, as required to record ambient/background levels; and shall continue 24- hours per day and 7-days per week for the duration of the work under the Protocol.

G22 MASW/ER

MASW and ER geophysical testing shall be accomplished along the entire length of the perimeter wall and on the extent of the basement slab.



Champlain Towers South - Joint Testing Protocol

Proj. No.	2021.4155
Date	December 10, 2021
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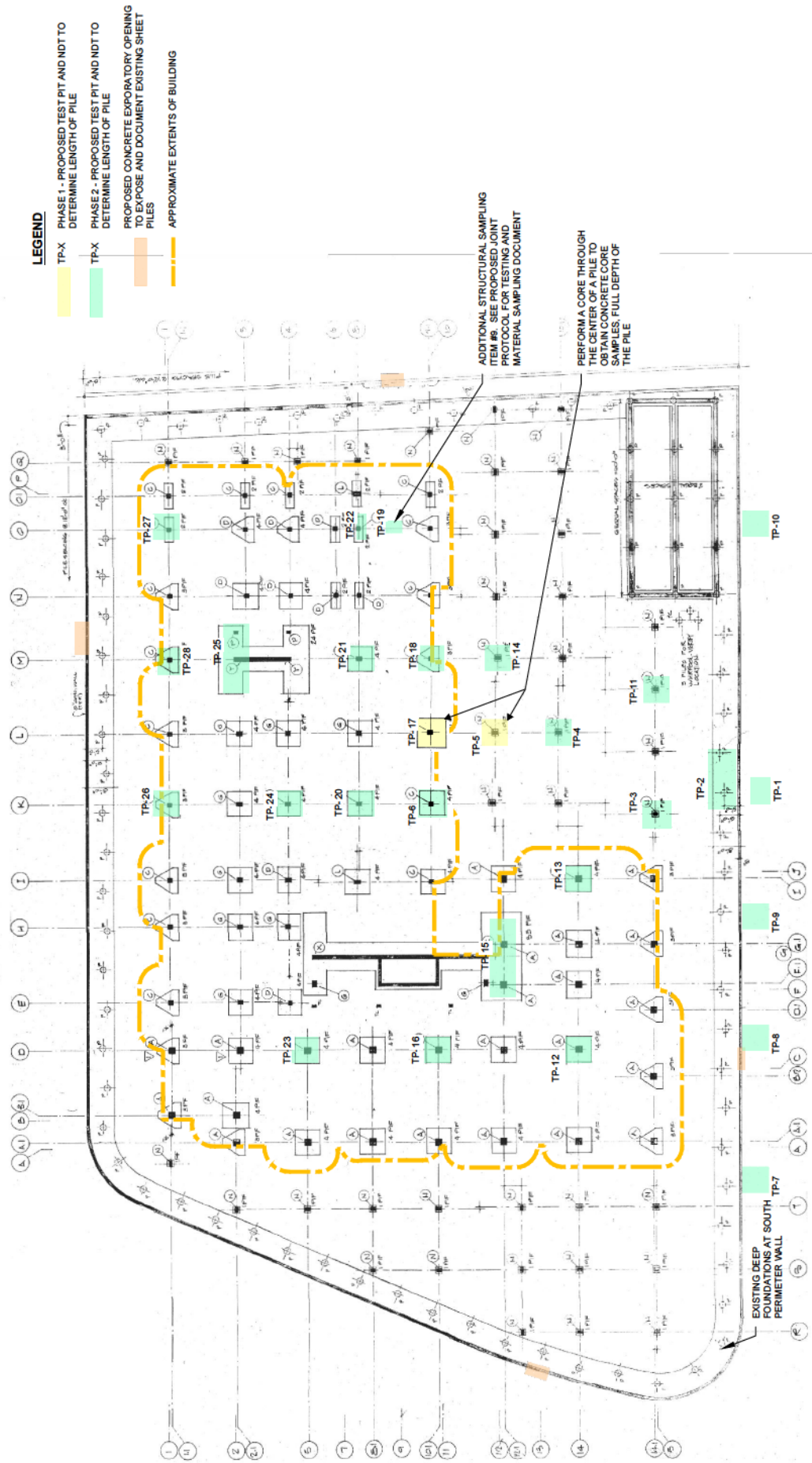
SK-01

Sheet No.

* BASE PLAN FROM DRAWINGS TITLED "CHAMPLAIN TOWERS SOUTH", PREPARED BY BREITERMAN JURADO & ASSOCIATES CONSULTING ENGINEERS, DATED 9/27/79

Appendix B
Champlain Towers South Investigation - Joint Protocol for Testing and Material Sampling (Collapse Site)

5



1 Test Pits *

* BASE PLAN FROM DRAWINGS TITLED "CHAMPLAIN TOWERS SOUTH" PREPARED BY BREITHEIM JORDAO & ASSOCIATES CONSULTING ENGINEERS, DATED 9/27/19
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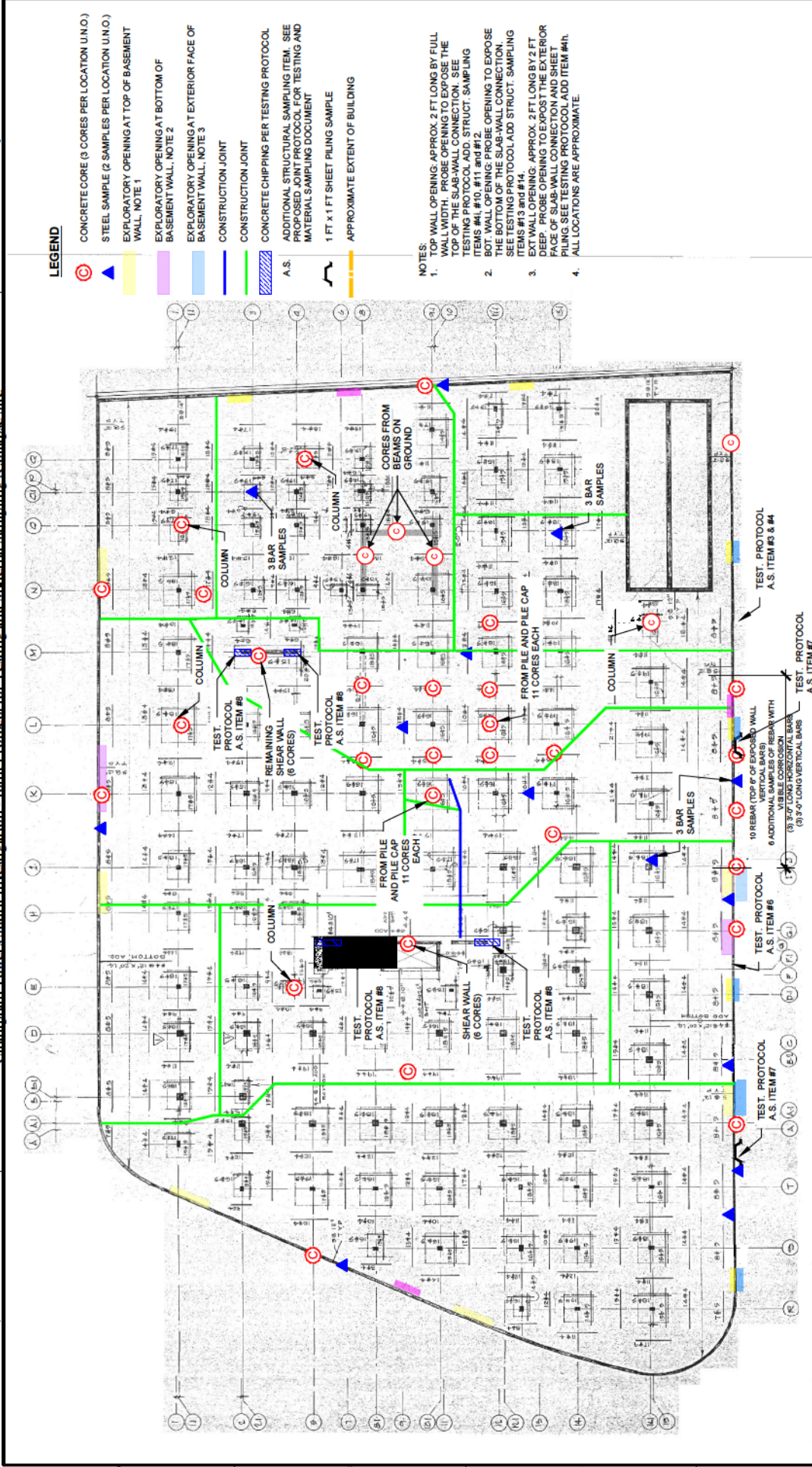
Revised: January 13, 2022

Project	Champlain Towers South - Joint Testing Protocol
Sheet Title	Test Pits
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SK-02

Sheet No.

Appendix B
Champlain Towers South Investigation - Joint Protocol for Testing and Material Sampling (Collapse Site)



1 Material Samples and Explanatory Opening Plan

Project	Champlain Towers South - Joint Testing Protocol
Sheet Title	Material Samples and Explanatory Openings
Project No.	2021 4155
Date	December 10, 2021
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Scale	As Noted

SK-03

Sheet No.

Project
Champlain Towers South - Joint Testing Protocol
8777 Colne Ave., Sunrise, Florida 33154

Sheet Title
Material Samples and Explanatory Openings

Project No.
2021 4155

Date
December 10, 2021

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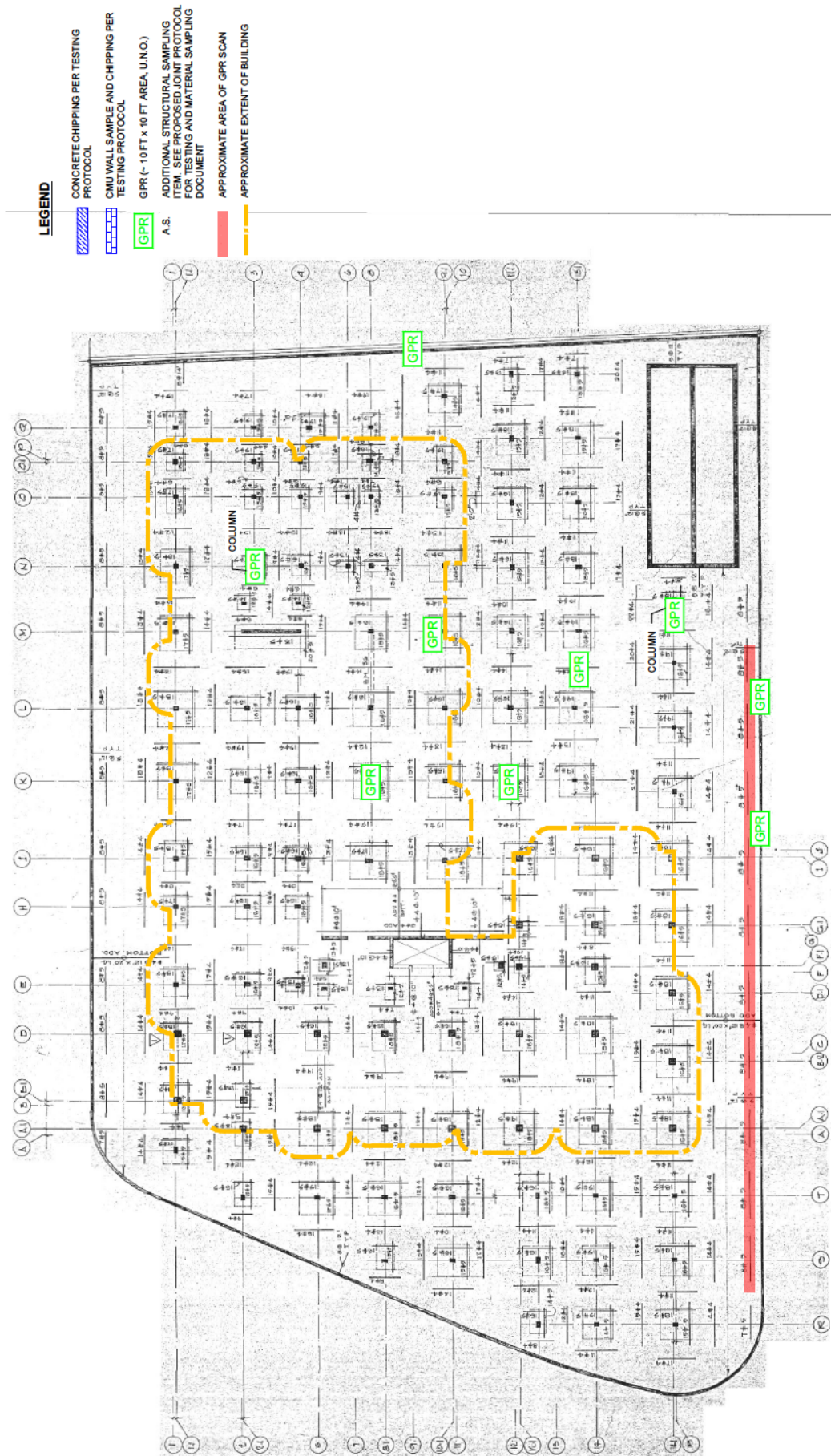
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Material Samples and Explanatory Opening Plan

Base Plan from Drawings Titled "CHAMPLAIN TOWERS SOUTH" PREPARED BY BREITHEIMAN JURADO & ASSOCIATES CONSULTING ENGINEERS, DATED 9/27/19

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Appendix B
Champlain Towers South Investigation - Joint Protocol for Testing and Material Sampling (Collapse Site)

5



1 Basement Plan

Revised: January 13, 2022

Project	Champlain Towers South - Joint Testing Protocol
Sheet Title	Basement NDT
Project No.	2021.4155
Date	December 10, 2021
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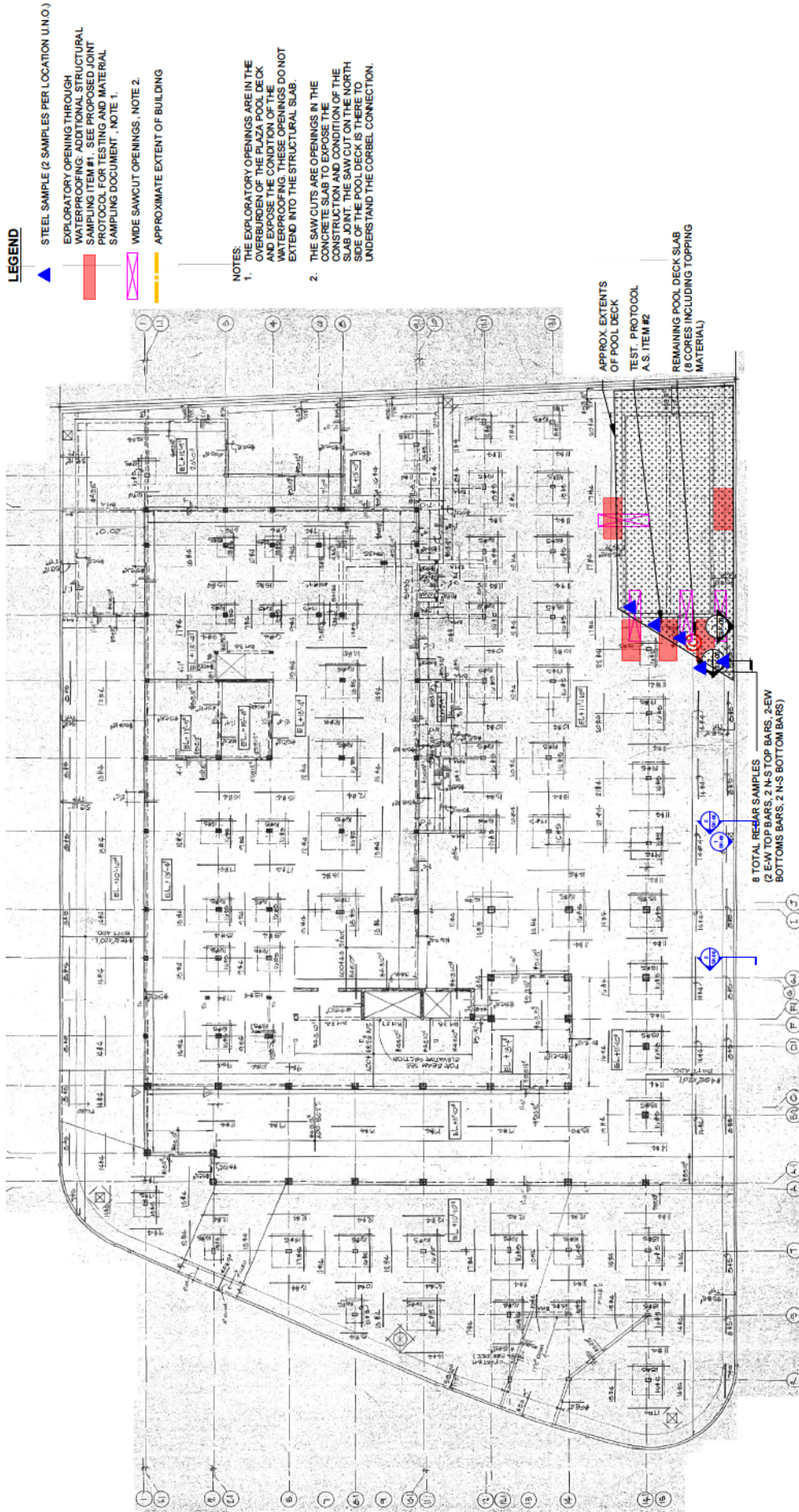
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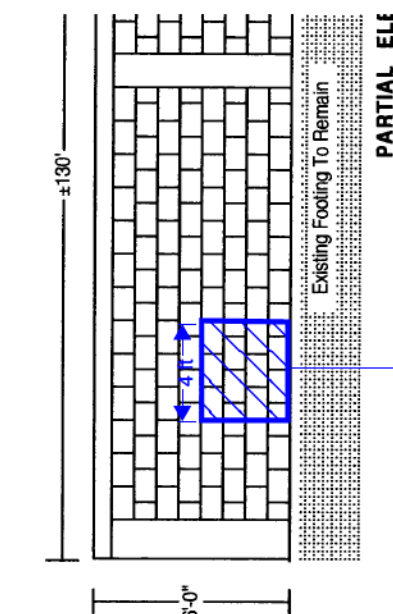
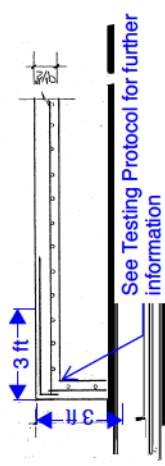
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Appendix B Champlain Towers South Investigation - Joint Protocol for Testing and Material Sampling (Collapse Site)



Project Champlain Towers South - Joint Testing Protocol 8777 Collins Ave., Suite 3154 Miami Beach, Florida 33154		Proj. No. 2021.4155 Date December 10, 2021 Drawn SKA Checked MFF Scale As Noted
Engineers WJE ARCHITECTS MATERIALS SCIENTISTS Wils, Jernsey, Elstner Associates, Inc. 330 Piquette Road Naperville, IL 60563 Tel: 630.277.2000 Fax: 630.277.2001		SK-05 Sheet No.
Base Plan from Drawings Titled "CHAMPLAIN TOWERS SOUTH" PREPARED BY BREITERMAN JURADO & ASSOCIATES CONSULTING ENGINEERS, DATED 02/7/79 1/10/2021 1:59 PM by Admin, Suite 315 - Suite 315 SK-05 Pool Deck Level.dwg		Revised January 13, 2022



TESTING PROTOCOL ADDITIONAL
STRUCTURAL ITEM #4

TESTING PROTOCOL ADDITIONAL
STRUCTURAL ITEM #3

NOTE:
AT GRIDLINE K, REMOVAL CONSISTS OF THE
PRIVACY WALL, FOUNDATION WALL, AND
RESIDUAL SLAB.
AT GRIDLINE H, REMOVAL CONSISTS OF THE
FOUNDATION WALL AND RESIDUAL SLAB.

See Testing Protocol for further information

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WJE
ENGINEERS
ARCHITECTS
MATHEMATICAL SCIENTISTS
Wesley, Janney, Elstner Associates, Inc.
330 Pines Road
Boca Raton, Florida 33434
Tel: 561.997.2400 Fax: 561.997.2401
www.wje.com

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* BASE PLAN FROM UNDATED DRAWING SHEET NO. S1 TITLED CHAMPLAIN TOWERS SOUTH
PREPARED BY BREITERMAN JURADO & ASSOCIATES CONSULTING ENGINEERS
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EXHIBIT “B”

IN THE CIRCUIT COURT OF THE 11TH
JUDICIAL CIRCUIT IN AND FOR
MIAMI-DADE COUNTY, FLORIDA

IN RE: CHAMPLAIN TOWERS SOUTH
COLLAPSE LITIGATION,

CLASS REPRESENTATION

CBL DIVISION

CASE NO: 2021-015089-CA-01

**DEFENDANT 8701 COLLINS DEVELOPMENT, LLC'S
REQUEST FOR ENTRY UPON LAND FOR INSPECTION**

Pursuant to Fla. R. Civ. P. 1.350(a), Defendant 8701 Collins Development, LLC ("8701 Collins Development") requests that Defendant 8701 Collins Avenue Condominium Association, Inc. (the "8701 Condo Association") provide access to property for inspection, measuring, surveying, photographing, testing, and sampling on April 18, 2022 and continuing from day to day until completion.

1. 8701 Collins Development requests access to land controlled by the 8701 Condo Association for the purpose of inspection, measuring, surveying, photographing, testing, and sampling as set forth in the Champlain Towers South Collapse Investigation: Joint Protocol for Testing and Material Sampling – Collapse Site (the "Joint Testing Protocol"). The Joint Testing Protocol was approved by the Court on January 21, 2022. Copies of the January 21 Order and the Joint Testing Protocol are attached as Composite Exhibit A.

2. The Joint Testing Protocol calls for testing to be performed in a beach access easement located over this property (the "Beach Access Area"). The Beach Access Area is situated at the northern edge of the property owned by the 8701 Condo Association and stretches from Collins Avenue eastward to the beach. The Joint Testing Protocol identifies, in Appendix B, the nature and the specific locations of the testing.

3. Testing pursuant to the Joint Testing Protocol in the Beach Access Area controlled by the 8701 Condo Association is scheduled to begin on Monday, April 18, 2022 and to continue from day to day until it is completed.

4. Accordingly, 8701 Collins Development requests access to the Beach Access Area beginning on April 15, 2022, and continuing until completion of the testing. This access shall extend to all individuals and entities necessary to complete the required testing set forth in the Joint Testing Protocol.

Dated: April 14, 2022

Respectfully submitted,

GREENBERG TRAURIG, P.A.
Counsel for 8701 Collins Development, LLC
Terra Group, LLC and
Terra World Investments, LLC
333 S.E. 2nd Avenue, Suite 4400
Miami, Florida 33131-3238
Telephone No.: (305) 579-0506
Facsimile No.: (305) 579-0717
Email: thomasmic@gtlaw.com
Email: barnettch@gtlaw.com

By: Michael J. Thomas
MICHAEL J. THOMAS
Florida Bar No.: 21309

CERTIFICATE OF SERVICE

I CERTIFY that a true and correct copy of the foregoing was furnished, either via transmission of Notices of Service of Court Document generated by the E-Portal or in some other authorized manner for those counsel or parties who are excused from e-mail service on this 14th day of April 2022.

/s/ Michael J. Thomas
MICHAEL J. THOMAS

EXHIBIT “A”

CHAMPLAIN TOWERS SOUTH COLLAPSE INVESTIGATION:
Joint Protocol for Testing and Material Sampling – Collapse Site

In accordance with the Court Order dated September 1, 2021, this document presents the protocol for testing and material sampling at the collapse site (the "Protocol") and has been developed considering input from involved Participants.

DEFINITIONS

- **Consultant** – Geosyntec Consultants, Inc., an independent consultant approved by the Receiver to coordinate, plan, oversee, and provide factual reporting as defined in this Protocol to the Experts/Participants. The Consultant will employ Testing Agencies and Contractors to meet the goals of the Protocol. Payment to Consultant for its fees and costs shall be paid by all Participants as defined in the "Payment of Costs" section below.
- **Contractor** – Company, or companies, approved by the Receiver to engage in providing construction, samples, borings, security, or other support for execution of the Protocol. The Contractors shall be engaged by the Consultant. Payment for services shall be remitted to Consultant for payment to Contractors. Payment for Contractors' services shall be paid by all Participants as defined in the "Payment of Costs" section below.
- **Court** – Circuit Court of the 11th Judicial Circuit in and for Miami-Dade County, Florida.
- **Expert** – Engineers (or other disciplines) investigating the collapse on behalf of Participants.
- **Litigation** -- *In Re: Champlain Towers South Collapse Litigation*, Case No. 2021-015089-CA-01, pending in the Eleventh Judicial Circuit Court in and for Miami-Dade County, Florida
- **Participant** – A party or entity authorized or defined by the *Order Approving Protocol for Inspection, Documentation, and Storage of Components, Remnants, and Debris of The Champlain Towers South Collapse*. Notwithstanding anything to contrary herein, non-parties to the Litigation are not Participants; *however*, such non-parties may participate in the testing and investigations set forth in this Protocol if authorized to do so by the Court.
- **Protocol** – This document, titled *CHAMPLAIN TOWERS SOUTH COLLAPSE INVESTIGATION: Proposed Joint Protocol for Testing and Material Sampling – Collapse Site*.
- **Receiver** – Michael I. Goldberg, Esq., on behalf of the Champlain Towers South Condominium Association, Inc.
- **Testing Agency** – An independent company, approved by the Receiver and working under the direction of the Consultant and/or Receiver to facilitate surveying, sampling, borings, materials testing, and report factual results. The Testing Agency will use Contractors as necessary to carry out the Protocol. The Testing Agencies shall be engaged by the Consultant. Payment to Testing Agencies for their services shall be paid by all Participants as defined in the "Payment of Costs" section below.

GENERAL

Scope. The Protocol applies to the Champlain Towers South site. A separate or amended protocol for testing and materials sampling at the primary evidence facility will be developed later.

Objective. The Protocol is intended to provide reliable *factual* information needed to support the collapse investigations by the Experts, including but not necessarily limited to the following:

- Mechanical, physical, and chemical characteristics of construction materials
- Subsurface conditions, including soils, rocks, water, pile caps, and piles

- Physical configuration of structural and waterproofing components, including the plaza deck, perimeter foundation walls, shear walls, columns, and basement slab.

Reporting. It is envisioned that the Consultant, who does not represent Participants, will obtain this factual information. The factual information will be shared among Participants. Except as noted below, reporting to Participants shall include factual information only (without interpretation or opinion), including the following:

- Testing standards and methodology
- Documentation of the locations and extraction of samples for materials testing
- Test results in a format usable for engineering analysis and interpretation
- Given that geophysical testing requires interpretation to prepare result reports, one report for each geophysical test shall be prepared for the benefit of all parties. Geophysical consultants shall prepare a written narrative that shows the basis for any interpretation in each report.

Project Updates. The Consultant will conduct daily meetings at the end of each day at the site to discuss the work completed on that day and the work anticipated for the following day. Additionally, the Consultant will conduct weekly virtual meetings to discuss the work completed during that week and the multi-week look-ahead planned work. The Consultant will issue project updates weekly. The project updates will include:

- Field testing/sampling
 - Anticipated start dates and testing duration
 - At least one week of notice prior to commencement of testing
- Laboratory testing
 - Anticipated start dates and testing duration
 - At least one week of notice prior to commencement of testing
- Issues/deviations encountered at site
- Daily site visit reports (to be uploaded to a secure Extranet)

Photos and videotaping of site sampling operations; videotaping of laboratory testing shall be conducted and agreed upon for each laboratory test.

Extranet. The Consultant will provide data, photos, videos, and other factual information on a secure FTP/Sharepoint site (or equivalent).

Access to Site. The site is located at 8777 Collins Ave. in Surfside, Florida. The entrance and security check-in trailer are in the northeast corner of the site. Participants, Experts, Testing Agencies, Contractors, and other site visitors seeking access shall comply with the following:

- Submit all requests for access to the site through the Receiver.
- Notify the Receiver at least two (2) business days before the planned site visit.
- Sign a waiver releasing the Receiver of liability and provide proof of liability insurance coverage, naming the Receiver as additional insured.
- Comply with established procedures for signing in and signing out. These procedures include but are not limited to surrendering driver's licenses and wearing wristbands during each visit.

Observation of Site Sampling and Testing. Participants and Experts shall be permitted to observe sampling and testing conducted at the site and laboratories as agreed upon by all Participants prior to testing. For laboratory testing, virtual observation of testing will be provided and the ability to comment on test procedure for the first of any certain type of test, Participants shall be permitted to visit a lab and observe the first test. The first test of each test type shall also be videorecorded and promptly shared with the Participants for review and comments. The entirety of any test performed at a laboratory on a sample or material must be recorded by video if the sample or material either (i) cannot be reconstituted or retested, or (ii) will be altered by reason of the performance of the

test. Site testing can be observed in person, and any objections to the methodology can be raised on site, during daily meetings, and/or weekly virtual meetings.

Safety. All site visitors shall comply with state and federal safety regulations and the site safety protocol established by the Consultant. The Receiver may prohibit an individual from accessing the site in the event such individual fails to adhere to state and federal safety regulations and the site safety protocol established by the Receiver.

Samples. Except as permitted herein or expressly approved by the Receiver for additional testing, samples of any materials shall not be taken or removed from the site.

Reserved/Backup Samples. In addition to samples designated for testing under the Protocol, additional samples shall be taken and reserved for additional testing to address failed tests, ambiguous results, or disputed results. These samples shall be identified in the Testing Plans.

Testing Plans. Consultant and its Contractors and testing agencies shall prepare a detailed testing plan of each field and laboratory test to be performed meeting the requirements of this Protocol. The testing plans shall be shared for review by all Participants prior to executing the work.

If consensus is not reached, any dissenting Participant may bring the disagreement to the Receiver's attention for resolution. If a resolution is not achieved, counsel for the Participant may choose to bring the disagreement to the Court for resolution.

Chain of Custody. All sample extraction and preparation in the field will be documented by photo and videotaping as agreed upon by the Participants. Samples will be extracted, protected per appropriate testing standards, and labeled. A chain of custody form will be issued for each sample/specimen. Samples will be placed in appropriate containers for the intended laboratory testing. After testing, the samples will be stored at a secure location (TBD) and preserved for future testing or observation.

Additional or Specialized Testing. Each Participant may, if they choose, spend additional days on site after the joint testing is complete to take additional samples for private testing and analysis by a laboratory of their choosing or to conduct additional testing on site. The Consultant will notify the Participants when the sampling for the Protocol is complete and such additional testing can begin. If the services of the Consultant are needed for additional or specialized testing, Participants will submit additional or specialized testing requests in writing to the Consultant. The Consultant will coordinate sampling and scheduling of samples. Any such additional samples taken or testing conducted over the additional days on site shall occur immediately after work performed under this Protocol. The Consultant/Contractor will remain on site following the conclusion of this Protocol for any Participant that wishes to take additional samples. Any additional or specialized testing, or any portion of it, may be completed by the Participant's own consultants and contractors without the participation of the Consultant (1) should the Participant so choose at its discretion, or (2) should the Consultant and its contractors lack the capacity, expertise or preparation to accomplish the additional testing. The Receiver shall be consulted for the scheduling of any additional or specialized testing and the Receiver shall make the site available for such additional or specialized testing coordinated with the Consultant and the Receiver. If any non-party Participant wishes to conduct additional testing, leave of Court is required. All Participants may observe additional specialized testing at the site.

For geotechnical testing using the Consultant's services, any Participant that chooses to remain on site following the conclusion of this Protocol for the purpose of collecting additional samples will be limited to additional borings at locations to be identified and provided in advance to the Consultant to be conducted within the agreed upon period for additional testing. The location(s) where any additional samples are taken from will be appropriately documented by the Consultant. For concrete and rebar sampling, the Participants shall be authorized to take companion samples of material taken under this Protocol during the agreed upon period for additional testing or as required by the Consultant or, depending upon the sequencing of work, while joint geotechnical testing is underway provided that the taking of additional concrete and rebar sampling will not interfere with other work on site.

The Receiver will permit Non-Destructive Testing to be coordinated and proceed before sampling.

Payment of Costs. Except for the costs attributable to additional or specialized testing, which costs are the sole responsibility of the Participant seeking such additional or specialized testing, each Participant's or other third-party's allocation of the costs incurred through performance of this Protocol shall be determined pursuant to an order from the Court.

Audio and Video Recording. Some or all of the work performed pursuant to this Protocol will be recorded via video and/or audio. Although the purpose of the video and/or audio recordings is to document and record the work performed pursuant to this Protocol, it is understood that said video and/or audio recordings may also capture conversations or statements made by on-site Participants and/or the Consultant. It is expressly agreed between the Participants that any conversations or statements that are captured and/or recorded in the video and/or audio recordings of the work performed pursuant to this Protocol shall not be admissible at trial for any purpose and may not be used by any Participant in this litigation or any other legal or administrative proceeding, including but not limited to at depositions, in hearings, or in any filings.

Water Control. Until sampling and site testing is complete, the Receiver will continue to arrange for removal of excess water that accumulates on the lower-level slab from leakage and rain. The Consultant shall implement a comprehensive dewatering program throughout the entirety of the slab and as otherwise as necessary throughout the site.

General. Nothing in this Protocol shall serve to confirm authorization or access to conduct inspections or testing on any property or portion thereof that is not under the ownership and control of the Champlain Towers South Condominium Association (in the "CTS Property"). There may be several boring, test pits and other tests anticipated under this Protocol that are not located on the CTS Property and are subject to access authorized by the property owner and/or Court approval.

TESTING AGENCIES AND CONTRACTORS

Testing Agencies and Contractors extracting samples shall be named (including credentials) by the Consultant and selected by consensus among the Participants for the testing scope listed in Table 2. Laboratories performing the work shall be ISO 17025, AASHTO R 18 or USACE accredited (or acceptable equivalent) for the tests to be performed. Persons performing the work shall have documentation/training to perform the specific tests and/or tasks, (e.g., ACI certified laboratory technician performing compressive strength tests on concrete). All testing shall, at a minimum, meet the industry standard of care. The Consultant will coordinate the extraction and delivery of samples to Testing Agencies and collect and deliver testing reports to the Receiver. A Participant may request permission from the Receiver to visit (in person or virtually) one or more testing agency to examine samples and view testing procedures. Such visits will be coordinated by the Receiver and Consultant.

Table 1. Testing Agencies and Contractors

Specialty	Testing Scope	Testing Agency/Contractor [website]
Structural	Demolition	To be determined
	Coring	To be determined
	Surveying	To be determined
Surveyor	Surveying	To be determined
Laboratory Testing	Concrete	To be determined
	Metals	To be determined
Petrography	Petrography	To be determined
Non-Destructive Testing	Ground penetrating radar	To be determined
Geophysics	MASW/ER	To be determined
	Seismograph	To be determined

Specialty	Testing Scope	Testing Agency/Contractor [website]
Geotechnical	Borings and observation wells	To be determined
	Test pits	To be determined
	Pile integrity testing (PIT)	To be determined
	Crosshole seismic testing	To be determined
	Laboratory testing	To be determined
	Seismic CPT testing	To be determined
	Parallel seismic testing for deep foundations and sheet piles	To be determined

TESTING METHODS AND STANDARDS

The Consultant, Testing Agencies, and Contractors shall adhere to the following standards for tests carried out. Proposed deviations from the standards shall be brought to the attention of all Participants and Experts through the Receiver. In general, non-destructive testing will occur before sampling. In addition, the following should be noted:

- General descriptions of the test methods are provided in Appendix A.
- Proposed testing locations are provided in Appendix B.

Table 2. Testing Standards

Category	Test	Destructive	Standard/Guide
Concrete Materials	C1 Compressive strength	Yes	ASTM C39 and C42
	C2 Elastic modulus and Poisson's ratio	Yes	ASTM C469
	C3 Splitting tensile strength	Yes	ASTM C496
	C4 Air void parameters	Yes	ASTM C457
	C5 Petrography*	Yes	ASTM C856 and C1723
	C6 Chloride profile	Yes	ASTM C1218 and C1152
	C7 Ionic Species (e.g., sulphur, magnesium, sodium, and potassium) profile	Yes	ASTM C114
Reinforcing Steel	R1 Reinforcing bar location survey using ground penetrating radar (GPR)	No	ASTM D6432 and ACI 228.2R-13
	R2 Tensile properties	Yes	ASTM A615 and ASTM A370
	R3 Rockwell hardness	Yes	ASTM E18
	R4 Chemistry	Yes	ASTM E415, ASTM A751
	R5 Section Loss / Pitting	Yes	ASTM G46, ACI 364.14T-17
Geotechnical	G1 Soil/rock borings**	Yes	ASTM D1586, FDOT Soils and Foundation Handbook (as applicable) Participants request including pitcher samplers (ASTM D7015, ASTM D1587), X-ray of undisturbed samples (ASTM D4452), rock coring with inner spring barrel (ASTM D2113), RQD (D6032). Soil/rock characterization (ASTM D2487, D2488, and D5878). Permeability tests (ASTM D5912)
	G2 Observation well	Yes	ASTM D5092; ASTM D5912.
	G3 Test pit excavation	Yes	N/A
	G4 Pile integrity testing (PIT)	Yes	ASTM D5882
	G5 Crosshole seismic testing	No***	ASTM D4428
	G6 Grain size analysis	Yes	ASTM D6913, ASTM D7928
	G7 Moisture content	Yes	ASTM D2216
	G8 Organic content	Yes	ASTM D2974
	G9 Incremental consolidation	Yes	ASTM D2435
	G10 Chlorides	Yes	AASHTO T291
	G11 Electrical resistivity	Yes	ASTM G57
	G12 pH	Yes	ASTM D4972 or G51
	G13 Sulfates	Yes	ASTM D516
	G14 Sulfides	Yes	SM4500
	G15 Unconfined compression of rock	Yes	ASTM D7012C
	G16 Unit weight of Rock Cores	Yes	ASTM D7263
	G17 Specific Gravity	Yes	ASTM D854, ASTM D5778
	G18 Seismic CPT	Yes	ASTM D5778
	G19 Parallel Seismic Testing	Yes	ACI 228.2R and ASTM D8381
	G20 Drained and Undrained Triaxial Testing of Fine-Grained Soil Samples	Yes	ASTM D7181, ASTM D4767, ASTM D2850
	G21 Seismograph	No	N/A

Category	Test	Destructive	Standard/Guide
	G22 MASW/ER	No	NA
* Due to the nature of Petrographic testing samples will be provided to labs as requested by Participants and available			
**Includes SPT, and obtaining and testing of soil and rock samples			
***Uses borings; however, the test is non-destructive			

CONCRETE SAMPLING AND TESTING PLAN

The table below lists tests of concrete core samples. Final locations are to be determined by the consensus of the Experts and depend on site conditions and locations of interest and shall be provided to the Receiver at least seven (7) days prior to the commencement of the concrete core sampling.

Table 3. Concrete Sampling and Testing Plan

Core Sample Phase/Location	Element	Original Element Location	Number of Cores***	Test Type and Number (See Table 2)				
				C1 Comp.	C2 E & v	C3 Tens.	C4 & C5* Voids/Petro	C6 & C7 Cl & Ionic Species
Site	Slab	Basement	33	13	3**	3	3	3
Site	Perimeter Wall	Basement	33	13	3**	3	3	3
Site	Pile Cap	Below Basement Slab	22	6	4	4	4	0
Site	Pile	Below Pile Cap	22	6	4	4	4	0
Site	Pool Deck Slab (remaining area at SE corner of site, including all layers (i.e., structural slab, topping, waterproofing, pavers)	Pool Deck at Lobby Floor	8	3	1**	2	1	1
Site	Shear Walls	Gridline M at Basement and West Core	12	6	2**	0	2	0
Site	Columns (rubble within extant rebar)	Various columns on gridlines E, L, M, O, and Q	15	0	0	0	10	0
Site	Beams	Between Gridlines N-O and 8-10	9	6	2	0	2	0
Total***			154	53	19	16	29	7

*Tests C4 and C5 will be conducted on the same core sample; separate core samples are needed for all other tests.

**One of the three cores used for compressive strength will also be used to determine modulus of elasticity and Poisson's ratio.

***Collect spare samples at each location for backup/additional testing (Consultant/Testing Agency to provide plan)

REINFORCING STEEL SAMPLING AND TESTING PLAN

Table 4 lists the general location and number of samples. Final locations are to be determined by the consensus of the Experts and depend on site conditions and locations of interest and shall be provided to the Receiver at least seven (7) days prior to the commencement of the steel sampling.

Table 4. Reinforcement Sampling and Testing Plan

Bar Sample Location	Element	Original Element Location	Bar Size	Number of Bars*****	Test Type and Number 8 (See Table 2)				
					R1* GPR	R2** Tens.	R3** Hard.	R4** Chem.	R5 Sect. Loss
Site	Slab	Basement	#5	6	4	6	3	1	6
Site	Wall	Basement	#5	23	4	23	23	4	23
Site	Column	Basement	N/A	N/A	2	0	0	0	0
Site	East Shear Wall	Basement	#10/#11	0	N/A	0	0	0	0
Site	Slab	Pool Deck	#4/#5****	16	0	16	16	4	16
Site	South Wall	Pool Deck	#5*****	10	0	0	0	0	10
Site	Columns (rebar protruding through slab)	Type N Columns***	#7	3	N/A	3	3	1	3
Site	Columns (rebar protruding through slab)	Various Type A and L Columns	#10	3	N/A	3	3	1	3
Site	Columns (rebar protruding through slab)	Various Type C, D, and G Columns	#11	3	N/A	3	3	1	3
Site	Column	Various Ties from Columns	#3/#4/#5	3	N/A	3	3	1	3
Total				67	10	57	54	13	67

*Non-Destructive Testing (approximately 10 ft x 10 ft area)

**Tests R1, R2 and R3 to be conducted on the same bar sample

***No. 7 rebar remaining in the slab at columns that supported the pool deck will be over-cored and removed for inspection only.

****Two bars top and bottom bars in each direction

*****Exposed Vertical Bars, perform fractography examination of exposed bar ends.

*****Collect spare samples at each location for backup/additional testing (Consultant/Testing Agency to provide plan)

ADDITIONAL STRUCTURAL INSPECTION/SAMPLING PLAN

The following inspections and samples shall be taken by the Consultant and examined on site or in the lab as applicable:

1. Plaza pool deck investigative work, including five (5) exploratory borings in the overburden, examination, field testing, and sampling of the waterproofing for laboratory testing. NOTE: This waterproofing sampling, field testing, and exploratory work must happen before destructive work on the underlying structure.
2. Pool deck slab N/S construction joint probe:
 - a. Provide additional pool deck slab shoring as required;
 - b. Shore top edge of adjoining privacy wall against lateral out-of-plane movement as required;
 - c. Locate all bars passing through construction joint along a 4 ft length of construction joint;
 - d. In the presence of all interested parties and under video observation, sawcut and remove a 4 ft (north-south) x 3 ft (east-west) slab of the existing pool deck slab and topping slab along the joint, taking care not to damage or drop the slab during sawcutting and removal;
 - e. Two (2) sawcut openings in the west edge of the remaining pool deck plaza to expose the cold joint assembly;
 - f. One (1) sawcut opening in the west edge of the remaining pool deck plaza to expose the slab-to-hot tub wall connection;
 - g. One (1) sawcut in the north edge of the remaining pool deck plaza to expose the slab-to-corbel connection;
 - h. Seal the preserved slab in heavy plastic and set aside for retrieval by laboratory for controlled and video-documented deconstruction; and
 - i. Measure and photo document the remaining sawcut cross section of the construction joint.
3. Privacy wall probe:
 - a. Provide additional pool deck slab shoring as required;
 - b. Shore top edge of adjoining privacy wall against lateral out-of-plan movement as required;
 - c. In the location where the pool deck slab is remaining west of the pool, GPR a 4 ft wide section (wider if necessary) of the CMU wall to locate the wall vertical bar(s) and dowels,
 - d. Make two vertical sawcuts, 4 ft apart and capturing the location of the vertical bar(s), to isolate CMU wall to be inspected/demolished from remainder of privacy wall;
 - e. In the presence of all interested parties and under video observation, sawcut and remove a 3 ft (east-west) x 3 ft (tall) portion of the existing wall, taking care not to damage or drop the wall during sawcutting and removal;
 - f. Seal the preserved wall portion in heavy plastic and set aside for retrieval by laboratory for controlled and video-documented deconstruction; and
 - g. Measure and photo document the remaining sawcut cross section of the wall.
4. South foundation wall to pool deck slab joint probe:
 - a. Provide additional pool deck slab shoring as required;
 - b. Shore top edge of adjoining privacy wall against lateral out-of-plan movement as required;
 - c. Within the 4 ft wide privacy wall probe, GPR a 3 ft x 3 ft area of the pool deck slab and a 3 ft x 3 ft area of the supporting wall to locate the rebar;
 - d. In the presence of all interested parties and under video surveillance, sawcut a 3 ft (north-south) x 3 ft (east-west) slab of the existing pool deck slab and topping slab along the joint, taking care not to damage or drop the slab during sawcutting;
 - e. After sawcutting through the slab, in the presence of all interested parties and under video observation, continue to sawcut the supporting wall and remove the L-shaped portion of the slab-to-wall connection as shown in page 6 of the attached figures;
 - f. Seal the preserved wall portion in heavy plastic and set aside for retrieval by laboratory for controlled and video-documented deconstruction;
 - g. Measure and photo document the remaining sawcut cross section of the slab and wall;
 - h. Provide six (6) test pits (as shown on SK-03) to expose the beach access walkway assembly and the top of the south perimeter sheet pile wall driven for the construction of 8777 Collins Ave. Note,

- these are not test pits to the bottom of the foundation wall;
- i. Provide six (6) exploratory openings in the exposed interior face of the southern perimeter basement wall that align with the test pits in item (h). Exploratory openings shall expose the sheet pile wall and embedded steel reinforcement; and
 - j. Visually survey and use a carpenter/mason's hammer sound the exposed interior face of the wall to map cracks and detect delamination as described in ASTM D4580. At delaminated locations perform selective demolition of the concrete perimeter wall from the basement interior to expose the sheet pile wall and embedded steel reinforcement. Remove one (1) sample of embedded reinforcement steel and sheet pile wall at each opening location.
5. Measure the depth of the foundation wall sheet piling at two locations along the south wall using parallel seismic testing (cross-reference test G19);
 6. Survey top edge geometry of sheet piling at 20 ft on center maximum, capturing all abrupt elevation changes (absolute elevation or elevation relative to bottom of pool deck slab):
 - a. Where required, chip the concrete to expose the top of steel sheet pile;
 - b. Optically survey the top of the sheet piles at 20 ft on center maximum, capturing all abrupt elevation changes;
 - c. Optically survey the elevation of the bottom of pool deck slab near the pool;
 - d. Reference the surveyed elevations to a datum.
 7. Take 1 ft x 1 ft samples of sheet pile at two locations along the south wall for measuring thickness, material properties, and corrosion quantity and quality (uniform loss or pitting);
 8. Chip concrete at each shear wall boundary zone to expose boundary element ties for observation and documentation by all interested parties;
 9. Provide test pit at undocumented column at grid O/8.5;
 10. Provide two (2) exploratory openings to expose the top of the west perimeter sheet pile wall driven for the construction of 8777 Collins Ave;
 11. Provide two (2) exploratory openings to expose the top of the north perimeter sheet pile wall driven for the construction of 8777 Collins Ave;
 12. Provide two (2) exploratory openings to expose the top of the east perimeter sheet pile wall driven for the construction of 8777 Collins Ave;
 13. Provide two (2) exploratory openings to expose the slab-to-wall connection at the bottom of the south perimeter basement wall.
 14. Provide one (1) exploratory opening to expose the slab-to-wall connection at the bottom of the north, west and east perimeter basement wall; and
 15. Perform level survey of the basement slab on grade capturing slab elevation at all column locations, adjoining location at the perimeter wall as applicable, shear walls, and the midspans of bays. In addition, the surveyed elevations to a datum must be referenced.

GEOTECHNICAL INVESTIGATION PLAN

The following geotechnical plan is developed as a basis for understanding site conditions and the foundation system. The test details and standards are to be developed based on the Experts' input and the Consultant engaged in carrying out the plan. The table lists general locations of borings and tests/samples. Final locations are to be determined by consensus of the Participants and will depend on site conditions and locations of interest. Details on sampling within the borings are shown in the attached plan (Appendix B) and described in section *G1 Soil/Rock Borings* (Appendix A).

The locations and numbers of tests for G6-G17 will be recommended by the Experts in consultation with the Consultant and approved by the Receiver and/or the Court. Note: Nothing in this Protocol shall serve to confirm authorization or access to conduct inspections or testing on any property or portion thereof that is not CTS Property. There may be several boring and test pits anticipated under this Protocol that are not located on the CTS Property and are subject to access authorized by the property owner and/or Court approval. The Protocol schedule shall be adjusted if circumstances outside of the Receiver's, the Consultant's, or the Participants' control, such as, but not limited to, permitting delays, access to adjoining properties, contractors' availability, equipment and material shortages, damage to samples collected, weather events, or COVID-related issues, materially impact the schedule.

Table 5. Geotechnical Sampling and Testing Plan – Receiver to coordinate any request for additional testing by site purchaser

Sample/Test Location	Test Type and Number*** (See Table 2)						
	G1 Borings	G2 Obs. Wells	G3* Test Pits	G4 PIT	G5 Crosshole Seis.	G18 CPT	G19 PSL
Slab Corners	0	0	0	0	0	0	0
Within Building Footprint	33	6	23	22	5	12	0
Outside Building Footprint	7	1	5	5	0	2	4
Total	40	7	28	27	5	14	4

*Core drill through pile cap and full length of pile

**Non-Destructive Testing

SCHEDULING AND ESTIMATED TIME REQUIRED ONSITE

The table below shows the estimated start date and time on site required for sampling and testing.

Table 6. Estimated testing start date and time required onsite

Sampling/Test	Estimated Start Date	Estimated Time Required on Site
Concrete Sampling	TBD	TBD
Steel Sampling	TBD	TBD
Concrete/Steel Laboratory Testing	TBD	TBD
G1 Soil Borings, G2 Observation Wells, G5 Crosshole Seismic, G18 Seismic CPT, G20	TBD	TBD

G3 Test Pits, G4 Pile Integrity Testing, G19 Parallel Seismic	TBD	TBD
G6-G17,G20 Laboratory Testing	TBD	TBD
G21, G22 MASW/ER, Seismographs	TBD	TBD

APPENDIX A

This appendix is intended to provide a brief description and outline requirements for each testing method in the Protocol. These descriptions should not be considered comprehensive and Participants should consult with their Experts for further information. Note: for all testing, the most recent and applicable testing standards and guides are to be used.

SITE SAFETY

The safety and wellbeing of all individuals on site, as well as the safety and wellbeing of those that may be impacted by the work performed under this Protocol shall at all times be the first and foremost priority of all individuals and companies involved in the performance of work. All work shall be performed in accordance with all local, state and federal rules and safety regulations and guidelines, including OSHA. Consultant will develop a site safety plan for the work which anyone entering the site must accept and adhere to. If anything in this Protocol calls for work to be done in a manner deemed unsafe by the Consultant or any Contractor, then said Consultant or Contractor shall not proceed with the work and all parties shall discuss how to proceed more safely. The Participants shall not be responsible for the implementation for any safety program nor have any responsibility for site safety, with the exception of making sure that they themselves comply with the safety program implemented by the Consultant. The Consultant may request that Receiver expel a Contractor or Participant that violates the safety plan.

CONCRETE TESTING

Concrete Core Removal

All core samples shall be taken using a 4-inch diameter core barrel (core diameter = \pm 3.7-inch) or largest barrel suitable to fit between bars at a specific location. Each sample shall be marked, measured (diameter and overall length), described, and photographed, including the size and condition of any embedded reinforcement.

Core samples shall be taken and conditioned per ASTM C42. Before coring, the surface shall be sounded with a hammer as described by ASTM D4580, and reinforcing bars in the vicinity shall be located using ground penetrating radar (GPR) or pachymeter. Cores for petrographic examination will be taken through near-surface reinforcing bars, especially where delamination planes have been identified by sounding. Otherwise, coring operations should avoid reinforcement. Core holes shall be inspected to determine if any cracks in the core samples are also present in the concrete. Cores intended for compression testing or splitting tensile testing shall be retaken if the cores include reinforcement, large voids, or cracks that preclude obtaining a 6-inch long sawn sample without such characteristics. After cores have been drilled, wipe off surface drill water and allow remaining surface moisture to evaporate. When surfaces appear dry, but not later than 1 hour after drilling, place cores in separate plastic bags or nonabsorbent containers, seal to prevent moisture loss, and label for identification.

Core holes through the Level 1 slab and basement wall shall be patched as required to prevent groundwater entry. Table 4 lists the general location and number of cores. Concrete core samples will be extracted with a diamond impregnated, water-cooled core drill. After removal, the core holes will be photographed and then patched with a rapid-setting, cementitious repair material. The condition and location of each core will be documented with photographs and notes.

C1 Compressive Strength

To determine concrete compressive strength, concrete cores shall be removed using a coring drill, trimmed, prepared for testing using sulfur capping, and loaded in a loading frame at a rate within a specified range. The compressive strength of the specimen will be calculated by dividing the maximum axial load sustained by the sample by the cross-sectional area of the specimen. Concrete core samples will be prepared and tested according to ASTM

C42 *Standard Test Method for Obtaining and Testing Drilled Cores and Sawed Beams of Concrete* and ASTM C39 *Standard Test Method for Compressive Strength of Cylindrical Concrete Specimens*

C2 Elastic Modulus and Poisson's Ratio

These parameters will be measured using appropriate instrumentation in a loading frame during compressive loading of the specimens. Elastic modulus and Poisson's ratio will be measured according to ASTM C469 *Standard Test Method for Static Modulus of Elasticity and Poisson's Ratio of Concrete in Compression*.

C3 Splitting Tensile Strength

Splitting tensile strength testing provides measurements to calculate the concrete tensile strength of the tested specimen. This test method consists of applying a diametral compressive force at a rate within a prescribed range along the length of a concrete core specimen in a load frame until failure occurs. The maximum load sustained by the specimen is divided by appropriate geometrical factors to obtain the splitting tensile strength. Splitting tensile strength will be measured in accordance with ASTM C496 *Standard Test Method for Splitting Tensile Strength of Cylindrical Concrete Specimens*.

C4 Air Void Parameters

Determining air void parameters involves examining a concrete cross-section under a microscope. One of three procedures will be used to determine air void parameters, including specific surface, void frequency, spacing factor, and paste air ratio of the air-void system in hardened concrete:

1. Linear-traverse method
2. Modified point-count method
3. Contrast-enhanced method

Air void parameters will be determined in accordance with ASTM C457 *Standard Test Method for Microscopical Determination of Parameters of the Air-Void System in Hardened Concrete*.

C5 Petrography

A petrographic examination consists of a series of observations of hardened samples that are interpreted to draw conclusions about the composition, quality, water to cement ratio, and probable cause(s) of potential problems associated with concrete or other construction materials. To conduct a petrographic examination, a core sample will generally be sliced in half lengthwise. One of the resultant halves will be lapped (polished) to accentuate the appearance of the components of the concrete (air voids, aggregates, and cement paste). Additional thin sections and smaller polished sections can also be made for petrographic microscope and scanning electron microscope (SEM) evaluations of cement paste and including microscopic chemical analysis.

A powder mount is obtained by scratching off a very small portion of the paste and mounting the resultant powder on a glass microscope slide in immersion liquid. Examination of a powder mount allows the properties of the cement paste to be studied in detail. Petrographic examination will be carried out in accordance with ASTM C856 *Standard Practice for Petrographic Examination of the Hardened Concrete* and ASTM C1723 *Standard Guide for Examination of Hardened Concrete Using Scanning Electron Microscopy*.

All samples and epoxied discs shall be preserved for review by other experts if needed and access granted when requested. The petrographic report shall include descriptions of the method(s) of preparation of the samples and full documentation of the images and other aspects of the samples that the lab personnel have based their interpretations on. The preparation and cutting of the samples by the lab technicians shall be video recorded.

C6 Chloride Profile

Chloride ion content testing will be performed on selected core samples to determine whether chloride contamination from salts has accumulated to sufficient levels to promote corrosion of reinforcing steel. Selected core samples will be cut into slices at pre-determined depths, and the slices will be pulverized to facilitate chloride

content analysis in general conformance with ASTM C1152 *Test Method for Acid-Soluble Chloride in Mortar and Concrete* and ASTM C1218 *Standard Test Method for Water-Soluble Chloride in Mortar and Concrete*. The acid-soluble chloride content represents both water-soluble and chemically-bound chloride within the cement paste and any chloride that may be present in the aggregate. The water-soluble chloride content represents only the chlorides in the cement paste and aggregates that are soluble in water. Slices will be taken at depth prescribed by ASTM C1556 *Standard Test Method for Determining the Apparent Chloride Diffusion Coefficient of Cementitious Mixtures by Bulk Diffusion*, Table 1. Table 1 in C1556 describes the depth at which slices should be taken given a known water to cement ratio. Thus, the water to cement ratio should be determined from petrographic analysis and then used to determine the number of slices to be analyzed chloride and other ionic species.

C7 Ionic Species Profile

The same powder used for analysis of chloride content by ASTM C1218 and ASTM C1152 can be used to determine the content of sulfur, magnesium, sodium, and potassium found in the concrete by utilizing testing methods described in ASTM C114 *Standard Test Methods for Chemical Analysis of Hydraulic Cement*. In fact, C1152 and C1218 rely on the method for chloride analysis in C114. As these elements are part of the ocean salts or cement paste, changes in depth and concrete location can be useful in pinpointing deterioration mechanisms.

REINFORCING STEEL TESTING

Reinforcing Steel Sample Removal

All reinforcing steel samples shall be extracted such that the samples are straight and undamaged by chipping hammers or other removal equipment. Deep saw cutting along and perpendicular to the bar is envisioned. Samples shall be at least 36 inches long. Each extracted sample shall be measured and photographed. Repair of sample locations may be necessary to prevent water intrusion.

R1 Reinforcing Bar Location Survey Using Ground Penetrating Radar (GPR) – Non-Destructive

To determine location (depth and spacing) and concrete cover of the reinforcing steel, ground penetrating radar (GPR) will be used. In addition to the non-destructive testing, limited destructive measures (e.g., drilling small ~1/2 in. diameter holes) will be used for correlation and calibration. GPR involves the use of a high-frequency radar antenna, which transmits electromagnetic radar pulses along with a discrete longitudinal scan at the surface of a structural element. Electromagnetic signals reflected from material interfaces having different dielectric properties (i.e., reinforcing steel embedded in concrete) are collected by the antennae and displayed graphically for direct interpretation in the field. Guidelines for GPR considered during this work included ACI 228.2R-13 *Report on Nondestructive Test Methods for Evaluation of Concrete in Structures* and ASTM D6432 *Standard Guide for Using the Surface Ground Penetrating Radar Method for Subsurface Investigation*.

R2 Tensile Properties

Tensile properties of reinforcing bar specimens include yield strength, ultimate tensile strength, elongation, and elastic modulus. Tensile properties will be determined by testing as-rolled bar specimens in a load frame. Testing of tensile property shall be carried out in accordance with the applicable portions of ASTM A615 *Standard Specification for Deformed and Plain Carbon-Steel Bars for Concrete Reinforcement* and ASTM A370 *Standard Test Methods and Definitions for Mechanical Testing of Steel Products*.

R3 Rockwell Hardness

The Rockwell hardness test is an indentation hardness test that involves the use of a verified machine to force a diamond spheroconical indenter or tungsten carbide (or steel) ball indenter into the surface of a material. Hardness is determined based on the size of the indentation. Rockwell hardness testing will be carried out in accordance with ASTM E18 *Standard Test Methods for Rockwell Hardness of Metallic Materials*.

R4 Chemistry

A spectrometric analysis will be used to determine the quantity of carbon, iron, and residual elements in carbon steel (copper, nickel, arsenic, lead, tin, chromium, etc.) for the determination of compliance with compositional specifications (in this case, ASTM A615). Testing will be carried out in accordance with ASTM E415 *Standard Test Method for Analysis of Carbon and Low-Alloy Steel by Spark Atomic Emission Spectrometry*.

R5 Section Loss/Pitting

The extent of section loss and pitting on samples (particularly those tested in R2) will be determined according to ASTM G46 *Standard Guide for Examination and Evaluation of Pitting Corrosion* and ACI 364.14T-17 *Section Loss Determination of Damaged or Corroded Reinforcing Steel Bars*.

GEOTECHNICAL INVESTIGATION

G1 Soil/Rock Borings

The following geotechnical investigation is proposed:

- Forty (40) borings with approximate locations as annotated in the attached Appendix B are proposed. Borings SPT-8, SPT-9, SPT-10 and SPT 14, which lie outside the perimeter of the Champlain tower walls to the south, will be performed with a smaller rig that fits in the narrow passageway.
- All borings are generally expected to extend to a rock layer at about 80 feet deep unless determined otherwise by the Participants.
- Boreholes for sampling shall be kept full of drilling fluid at all times. If needed, bentonite or suitable alternative should be added to keep the borehole full of drilling fluid and avoid borehole squeeze.
- The driller will open the SPT sampler and allow Participants to visually inspect the contents. Samples from the SPT sampler will be collected at various depths within the boreholes as directed by the Consultant and sealed in watertight containers. The drilling contractor will perform SPT testing and obtain samples at a maximum interval of every 5 feet, or as otherwise directed by the Participants. SPT samples will be collected continuously as directed by the Participants.
- Samples of the interbedded limestone/sand stratum will be obtained using Pitcher samplers in addition to split-spoon samples during the initiation of the subsurface investigation. The Consultant, in agreement with the Participants, shall evaluate and determine the need for subsequent Pitcher sampling as the subsurface investigation continues; however, pitcher sampling shall not be eliminated and shall be incorporated as a supplement in select borings to provide a comparison between split-spoon samples and pitcher samples. An alternative to the Pitcher sampler may be the use of a triple-tube core barrel to increase sample recovery and quality.
- The drilling subcontractor shall use methods and procedures to maximize core recovery. Coring shall be in accordance with FDOT standards. In addition, coring should be continuous, whether 5-ft or 10-ft core runs are used.
- Samples of soil and rock cores will be retrieved and shipped for further laboratory testing at a mutually agreed upon geotechnical testing laboratory. The Consultant will develop a protocol for adequate sample preservation and shipping.
- All Participants will be allowed to visually observe the joint investigation. Participants will not direct the drilling contractor for the forty (40) joint borings. All soil and rock samples will be allowed to be visually evaluated by all Participants on-site during the investigation.

The forty (40) boring locations are to be drilled by the same mutually agreed upon drilling contractor who will work with the Consultant to observe, log samples, and provide investigation reports. The drilling contractor shall provide sample boring and drilling logs and list of items to be included in the visual description for review by Participants. The Consultant will provide a sample boring log also for review by Participants. The Consultant will provide reports including, but not limited to, visual-manual soil identification per ASTM D2487 and D2488, SPT blow counts, RQD

and water level observations. The same contractor shall remain on site after the completion of the 40 borings so each Participant can perform additional tests, if so requested, for an agreed upon duration of time. The testing performed by each Participant after the first 40 borings can be visually observed by all Participants; however, the samples extracted will belong to the specific Participant coordinating the testing and can be destructively tested as needed.

In the standard penetration test (SPT) procedure, a 2-inch outer diameter split-barrel sampler is driven into the soil by repeatedly dropping a 140-pound hammer from a height of 30 inches. The length of the samplers can be 18 or 24 inches. The number of blows required to advance the sampler every 6 inches are recorded. The sum of the second and third blows is recorded as the SPT N values for that depth. SPT N values are utilized to correlate strength parameters for the soils. Accompanying energy measurements will be reported. Soil and rock borings shall be carried out in accordance with ASTM D1586 *Standard Test Method for Standard Penetration Test (SPT) and Split-Barrel Sampling of Soils*.

- Soil borings will be advanced using the standard penetration test (SPT) procedure described above unless otherwise directed by the Consultant's geologist or engineer. The test provides samples for identification purposes and provides a measure of penetration resistance which can be used for geotechnical design purposes. The standard penetration test recovers a highly disturbed sample, which is generally not suitable for tests that measure properties of the in-situ soil structure, such as density, strength, and consolidation characteristics. The samples will be labeled, sealed, and transported to a laboratory for further index and classification testing. If fine-grained material is encountered, thin-walled tube samples should be used for obtaining intact specimens of fine-grained soils for laboratory tests to determine engineering properties. Similarly, if soils containing cohesive materials (clays) are encountered, thin-walled tube sampling should be performed to obtain minimally-disturbed samples. Drained and undrained triaxial testing will be performed on any samples of clay to establish strength parameters.
- The Consultant should provide draft investigation logs every week or sooner of all work completed the prior week.

G2 Observation Wells and Water Quality Samples

A standpipe will be installed after the completion of specified boreholes. Observation wells should be screened in separate strata. Certain observation wells should be screened in the sand stratum above the interbedded limestone/sand, and others should be screened in the interbedded limestone/sand stratum itself. These standpipes will be monitored for groundwater levels in the subsequent days of the investigation. An electronic transducer connected to a datalogger shall be installed in each well. The groundwater should be monitored at least six times daily until it is determined by Participants that it can be stopped, to develop a record of seasonal and other variations. A barometer shall be installed at the site. Sufficient dataloggers shall be installed at the site to read all installed sensors with provisions for the data to be provided to the Participants at regular intervals. Water quality samples will be collected and sent to an approved laboratory for testing. Field permeability testing in the sand and the interbedded limestone/sand strata in general shall be performed accordance with ASTM D5912. The Participants shall identify the location of each field permeability test. The Participants shall fully develop the requirements and details for the observation wells after the Consultant obtains subsurface information from the borings.

Monitoring wells will be installed at the locations shown on Appendix B and any other location identified by the Participants to determine long-term, stabilized groundwater readings. A slotted pipe will be installed within the borehole and packed with sand on the outside to ensure the slots in the pipe are not clogged. A bentonite seal shall be established above the top of the screened zone. In addition, grout will be placed near the top of the pipe to ensure surface water does not percolate into the observation well. The top of the pipe will be capped and sealed to ensure precipitation does not enter the observation well. Observation wells will be installed and monitored in accordance with ASTM D5092 *Standard Practice for Design and Installation of Groundwater Monitoring Wells*.

Samples at 10-ft intervals within the borehole shall be obtained for the purposes of water quality sample testing. Samples shall be obtained in accordance with the FDOT Soils and Foundation Handbook for investigations for Structures and shall be tested in accordance with Section 4.10 – Environmental Corrosion Test Guidelines.

G3 Test Pit Excavation

Two (2) test pit excavations (TP-5 and TP-17) and full-length cores at the piles identified as TP-5 and TP-17 on Appendix B shall be performed prior to commencement of any borings in order to establish the lengths of the borings. The remainder of the test pits shall be performed after the borings are completed.

The structural slab and remaining column shall be extracted labeled and set aside for inspection and additional sampling if necessary. Care should be taken to remove the column and structural slab as to minimize damage.

Test pits shall be sufficiently deep to expose the pile caps and the top 2 to 3 feet of the pile. At each test pit, core drill or saw cut through the pile cap and portion of the pile to extract samples and permit pile integrity testing. Test pits shall be dewatered as required to permit observation and pile integrity testing. Contractor shall cut through concrete, remove materials and provide needed dewatering under the direction of the Consultant. At the basement wall, test pits shall be sufficiently deep to expose or extend to the elevation of the bottom of the foundation wall.

Walkway test pits shall be sufficiently deep to expose or extend to the elevation of the bottom of the foundation wall. Test pits shall be dewatered as required to permit observation. Contractor shall cut through concrete, remove materials and provide needed dewatering under the direction of the designated engineer/geologist.

The Consultant will engineer each test pit excavation for safe access. The test pit designs will be shared with all parties. OSHA standards will be complied with in relation to design, execution and access.

G4 Pile Integrity Testing (PIT)

PIT Testing is performed by placing accelerometers at several locations on the pile and then hitting the pile or as necessary to obtain conclusive and accurate test results, with different size hammers and then measuring the response of the pile to the impact. The results are used to estimate the length of the pile. For higher quality of results, it is recommended to access the pile at the top and the side. Pile integrity testing will be performed in accordance with ASTM D5882 *Standard Test Method for Low Strain Impact Integrity Testing of Deep Foundations*.

PIT will be performed by an agreed upon subcontractor. The PIT subcontractor will work under the direction of the Consultant and, subsequent to the testing, will provide raw data and interpretive reports to all Participants. Depending on the data available for the design and as-built length of piles, PIT must be performed a few days prior to the geotechnical boring investigation for all Phase 1 test pits to determine the necessary depth of borings. Sufficient time will be provided to allow interpretation of PIT data and if inconclusive results are obtained, alternative testing techniques should be implemented. Refer to G19 Parallel Seismic logging.

G5 Crosshole Seismic Testing

Crosshole seismic testing will be performed between boreholes as directed during the field investigation as identified in Appendix B. Crosshole Seismic Testing will measure compression and shear waves between two adjacent boreholes at various depths according to ASTM D4428. The Consultant will determine the final holes to be tested based on findings of the borehole sampling and initial test results. Additional borings shall be made if necessary to satisfy the requirements of ASTM D4428. A crosshole testing plan will be provided by the consultant prior to execution.

G6 through G17, Laboratory Testing

Select samples from the field investigation of all joint borings and the test pits will be tested in a commonly agreed upon laboratory proposed by the Consultant. Specific tests and testing locations will be agreed upon on a rolling basis by all Participants as the field work is completed, and drilling logs have been provided. Reasonable efforts will

be made depending on available sample quantities to provide every requested test. Participants may request to use samples that were not selected for laboratory testing as part of this Protocol, for laboratory testing at their own expenses. The Consultant shall provide weekly drilling/investigation logs and testing will be agreed upon and ordered every week based on the field observations and the drill logs.

G6 Grain Size Analysis

Soils consist of particles of various shapes and sizes. The particle-size distribution (gradation) of soil can be determined from representative specimen following ASTM D6913 *Standard Test Methods for Particle-Size Distribution (Gradation) of Soils Using Sieve Analysis*. In cases where the gradation of particles smaller than No. 200 (75- μ m) sieve is needed, *Test Method D7928* may be used. All samples subjected to grain size evaluation with more than 10% fines should include hydrometer testing such that the D10 of the material can be determined for estimation of permeability.

G7 Moisture content

The moisture content of soil and rock can be determined in the laboratory by drying the material and measuring the reduction in mass of the material. The water content is the ratio of the mass of water contained in the pore spaces of soil or rock material to the solid mass of particles expressed as a percentage. The test is performed according to ASTM D2216 *Standard Test Methods for Laboratory Determination of Water (Moisture) Content of Soil and Rock by Mass*.

G8 Organic Content

Percentage of organic matter is important for classifying or evaluating peat or other organic soil, and for geotechnical and general soil classification purposes. This test can be used to determine the moisture content, ash content, and percent organic matter in the soil. The test is performed according to ASTM D2974 *Standard Test Methods for Moisture, Ash, and Organic Matter of Peat and Other Organic Soils*.

G9 Incremental Consolidation

The magnitude and rate of consolidation of fine-grained soil can be determined when it is restrained laterally and drained axially while subjected to incrementally applied controlled-stress loading. The test is performed on undisturbed soil samples in accordance with ASTM D2435 *Standard Test Methods for One-Dimensional Consolidation Properties of Soils Using Incremental Loading*. Loading increments specified for the lab will need to be agreed upon by all Participants.

At the request of any Participant, testing will be performed on reconstituted samples of sand to obtain stiffness parameters.

G10 Electrical Resistivity

Electrical resistivity for representative samples will be determined in situ and in the laboratory. Measurement of soil resistivity is used for the control of corrosion of buried structures and for the estimation of expected corrosion rates. The test will be performed according to ASTM G57 *Standard Test Method for Field Measurement of Soil Resistivity Using the Wenner Four-Electrode Method*.

G11 Chlorides, G12 pH, G13 Sulfates, G14 Sulfides

This group of tests is performed in a laboratory setting to determine chloride ion, sulfate ion, sulfide content, and pH level in the representative samples. The principal use of the pH test is to supplement soil resistivity measurements and thereby identify conditions under which the corrosion of metals in soil may be accentuated. The individual tests will be performed according to ASTM D512 *Standard Test Methods for Chloride Ion in Water*, ASTM D516 *Standard Test Method for Sulfate Ion in Water*, SM 4500 *Sulfide by Standard Methods Committee*, and G51 *Standard Test*

Method for Measuring pH of Soil for Use in Corrosion Testing. Water samples from bailed observation wells for groundwater chemistry testing shall be obtained.

G15 Unconfined Compression of Rock

The strength of intact rock core specimens in uniaxial compression will be determined following ASTM D7012C *Standard Test Methods for Compressive Strength and Elastic Moduli of Intact Rock Core Specimens under Varying States of Stress and Temperatures*. In the laboratory, a rock core specimen is cut to length and the ends are machined flat if possible and if not possible, as otherwise determined by the Participants. The specimen is placed in a loading frame, where the axial load on the specimen is increased and measured continuously until the sample fails. The uniaxial compressive strength is calculated as the maximum load at failure divided by the cross-sectional area of the specimen. Adequate instrumentation shall be included to determine Young's modulus.

G16 Unit weight of Rock Cores

Unit weight of rock cores will be determined in general accordance with ASTM D7263 *Standard Test Methods for Laboratory Determination of Density and Unit Weight of Soil Specimens*, method B (direct measurement). For a rock core specimen of cylindrical shape, the dimensions and mass of a specimen are measured. The density and unit weight are then calculated using these direct measurements.

G17 Specific Gravity

Specific gravity of soil samples will be determined in general accordance with ASTM D854, *Standard Test Methods for Specific Gravity of Soil Solids by Water Pycnometer*. These test methods cover the determination of the specific gravity of soil solids passing a sieve using a water pycnometer. These test methods are not viable for solids that can be altered by these methods, are contaminated with a substance that prohibits the use of these methods, or are highly organic (such as fibrous matter which floats in water).

G18 Seismic Cone Penetration Testing (CPT)

Seismic cone penetration testing estimates the small strain shear modulus of the soil by measuring shear wave velocities in the ground. The seismic CPT testing is performed as part of CPTU testing. The CPTU test will additionally measure continuous point and frictional resistance during penetration of the cone, pore pressures, and dissipation of pore pressures with time at constant depths. This testing will be carried out in general accordance with ASTM D5778 *Standard Test Method for Electronic Friction Cone and Piezocone Penetration Testing of Soils*.

The following geotechnical investigation is proposed:

- Seismic CPTs capped at 400-500 tsf with approximate locations as annotated in the attached Appendix B.
- CPTs will be pushed until refusal.
- Continuous measurements of point resistance, frictional resistance, pore pressure and shear wave velocity will be performed while the cone is pushed at a constant rate of 2 cm/sec. Three dissipation tests for each CPT location will be performed to establish groundwater and dissipation properties at those depths as directed by the Consultant.
- No sample recovery is performed with this testing.

The Seismic CPTs will be performed by the same mutually agreed upon contractor who will work with Consultant to observe and provide investigation reports. The Seismic CPT contractor will provide both raw data and reports including, but not limited to, cone tip resistance, frictional resistance, pore pressure measurement with depth, and pore pressure measurements with time during dissipation testing. Permeability values determined from pore pressure dissipation tests will be included.

G19 Parallel Seismic Logging

Parallel Seismic Logging (PSL) will be performed at indicated locations as annotated in the attached Appendix B. Boreholes will have casing and grout installed for the PSL testing that will be performed subsequent to the drilling activities. This testing involves sending a transceiver down the cased borehole to measure the depth of the buried sheet pile wall and will be carried out in general accordance with ASTM D8381 *Standard Test Methods for Measuring the Depth of Deep Foundations by Parallel Seismic Logging*.

Parallel seismic logging shall be accomplished as a supplemental means of determination of foundation lengths in the event that PIT interpretations are deemed inconclusive.

G20 Drained and Undrained Triaxial Testing of Fine-Grained Soil Samples

If fine-grained material is encountered and if thin-walled tube samples are used for obtaining intact specimens of fine-grained soils for laboratory tests to determine engineering properties, then triaxial testing will be performed on the specimens. This testing will be carried out in general accordance with ASTM D7181 *Standard Method for Consolidated Drained Triaxial Compression Test for Soils*, ASTM D4767 *Standard Test Method for Consolidated Undrained Triaxial Compression Test for Cohesive Soils*, and/or ASTM D2850 *Standard Test Method for Unconsolidated-Undrained Triaxial Compression Test on Cohesive Soils*.

At the request of any Participant, testing will be performed on reconstituted samples of sand to obtain shear strength parameters.

The Participants shall endeavor, but are not required, to agree on the Protocol of testing G20 including initial stress state before shearing, stress levels, strain rate, and drainage conditions during testing.

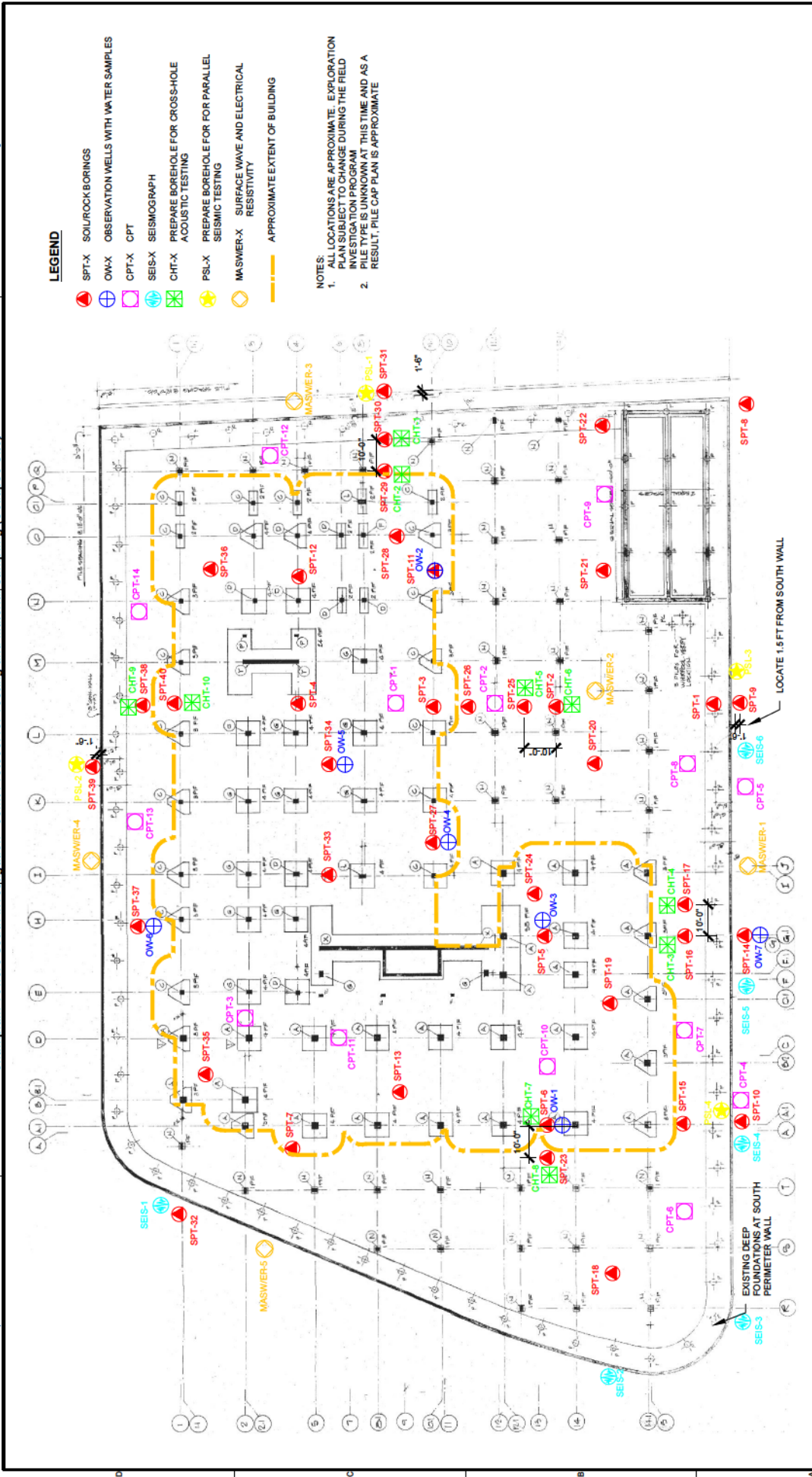
G21 Seismograph

Vibration monitoring shall be accomplished via a calibrated seismograph throughout the duration of all work accomplished on site at various locations, including those shown on Appendix B, with the monitors installed per location at different distances. The vibration monitoring shall start one week prior to the beginning of testing under the Protocol to establish a baseline at a trigger level of 0.05 in/s or lower, as required to record ambient/background levels; and shall continue 24- hours per day and 7-days per week for the duration of the work under the Protocol.

G22 MASW/ER

MASW and ER geophysical testing shall be accomplished along the entire length of the perimeter wall and on the extent of the basement slab.

Appendix B Champlain Towers South Investigation - Joint Protocol for Testing and Material Sampling (Collapse Site)

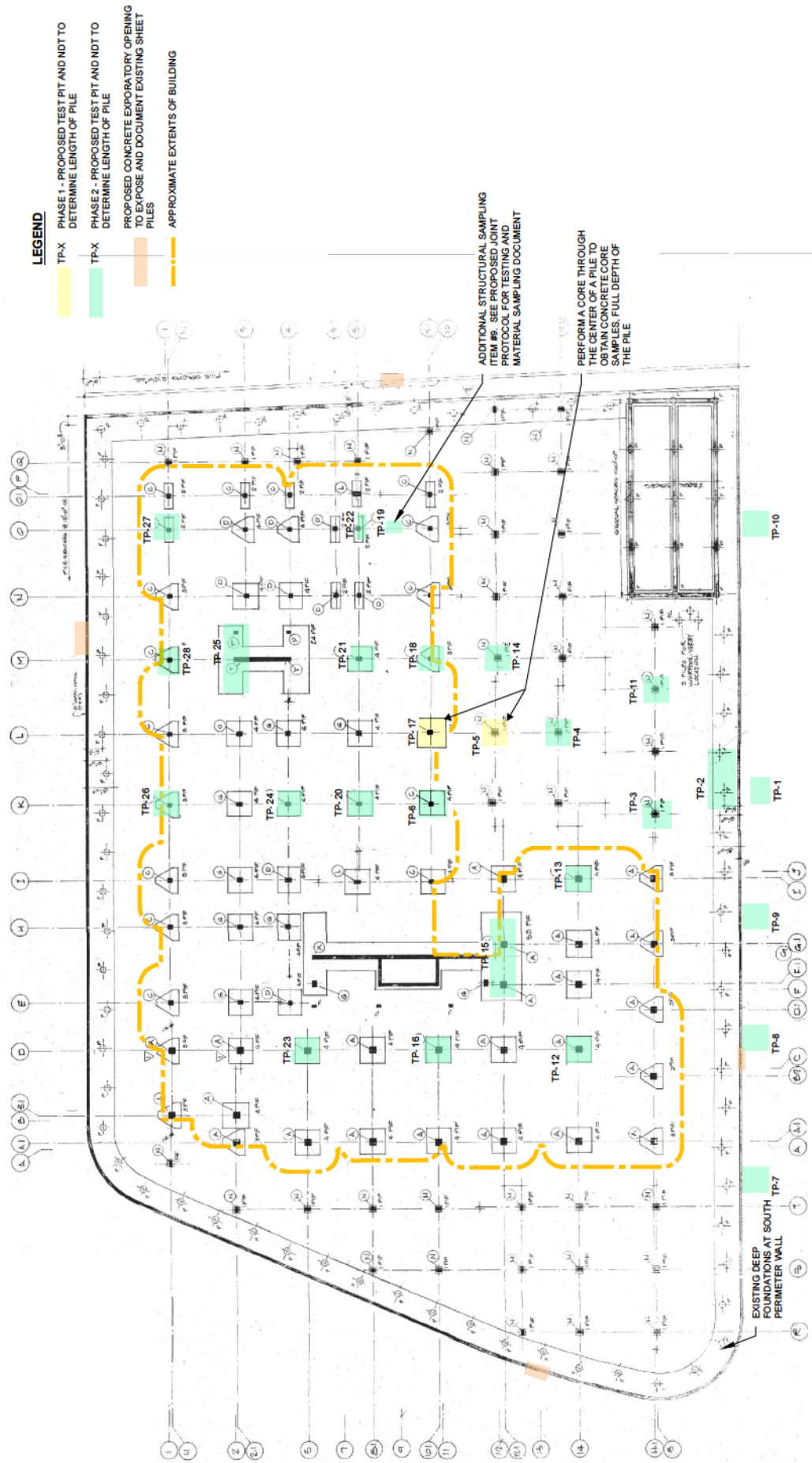


<p>Project: Champlain Towers South - Joint Testing Protocol 8777 Colne Ave., Burlington, Florida 33154</p>		<p>Proj. No. 2021.4155 Date December 10, 2021 Drawn SKA Checked MFF Scale As Noted</p>
<p>Sheet Title: Boring Plan</p>		<p>Sheet No. SK-01</p>

* BASE PLAN FROM DRAWINGS TITLED "CHAMPLAIN TOWERS SOUTH" PREPARED BY BREITERMAN JURADO & ASSOCIATES CONSULTING ENGINEERS, DATED 8/27/19
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Appendix B Champlain Towers South Investigation - Joint Protocol for Testing and Material Sampling (Collapse Site)

5



1 Test Pits *

Revised: January 13, 2022

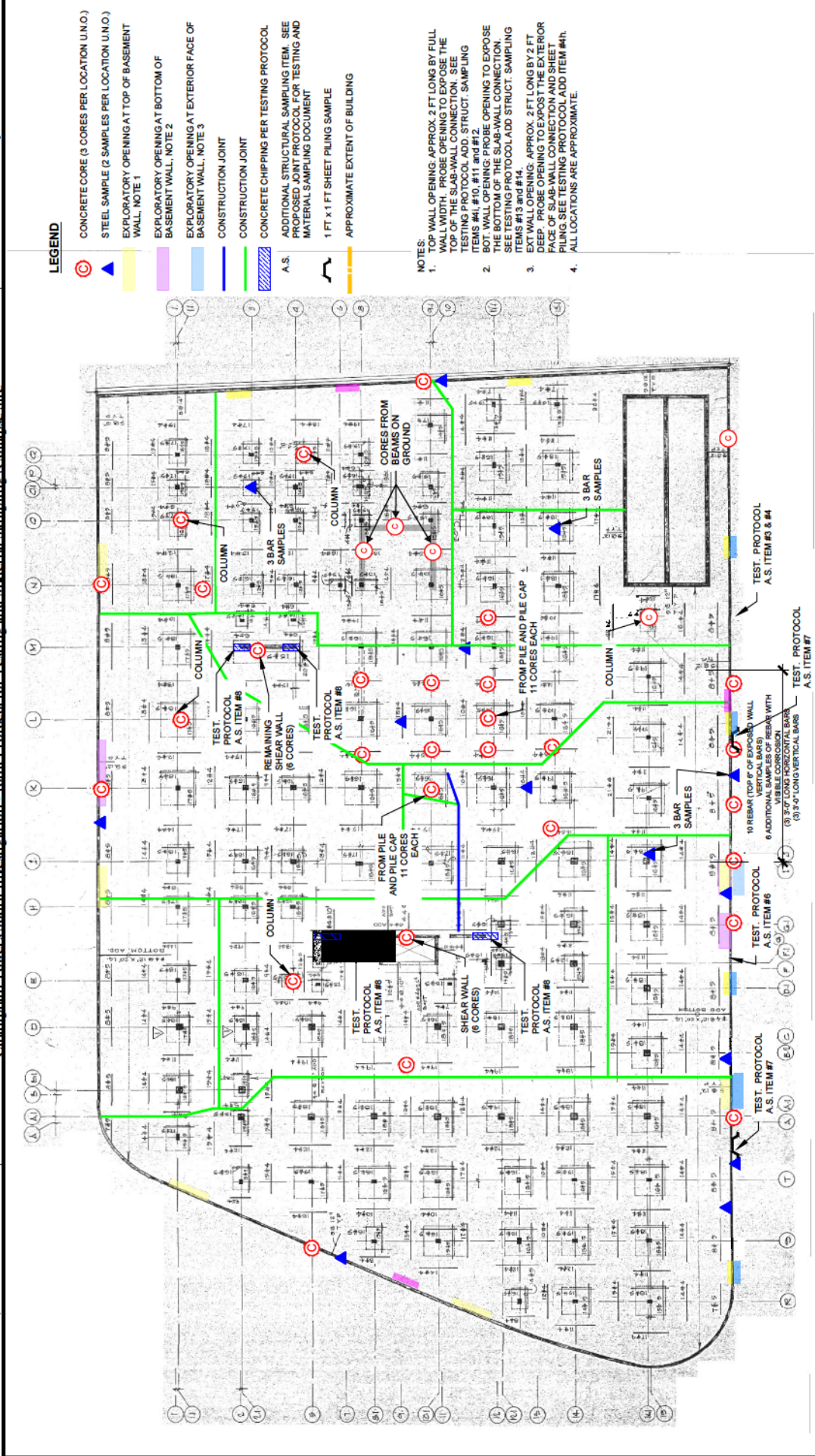
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Sheet Title	8777 Colina Ave., Surfside, Florida 33154	Date	December 10, 2021
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SK-02

Sheet No.

* BASE PLAN FROM DRAWINGS TITLED "CHAMPLAIN TOWERS SOUTH" PREPARED BY BREITERMAN JURADO & ASSOCIATES CONSULTING ENGINEERS, DATED 9/27/19
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Appendix B
Champlain Towers South Investigation - Joint Protocol for Testing and Material Sampling (Collapse Site)



Material Samples and Expatory Opening Plan

1

Project: Champlain Towers South - Joint Testing Protocol
8777 Colne Ave., Sunrise, Florida 33154

Sheet Title: Material Samples and Expatory Openings

Project No. 2021.4155
Date: December 10, 2021
Drawn: SKA
Checked: MFF
Scale: As Noted

SK-03

1/2" = 1'-0"

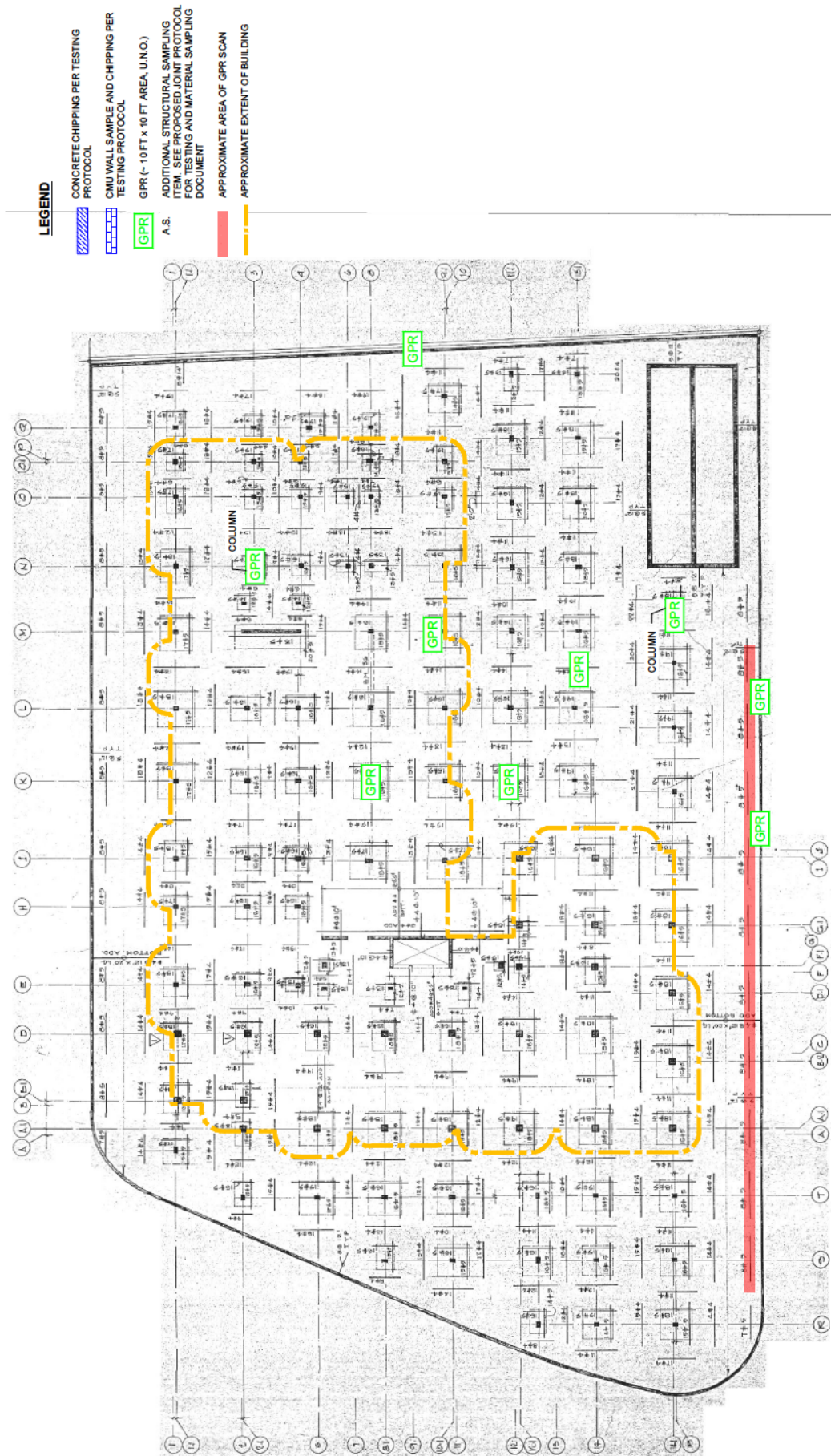
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* BASE PLAN FROM DRAWINGS TITLED "CHAMPLAIN TOWERS SOUTH" PREPARED BY BREITHEIM JORDAO & ASSOCIATES CONSULTING ENGINEERS, DATED 9/27/19

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Appendix B
Champlain Towers South Investigation - Joint Protocol for Testing and Material Sampling (Collapse Site)

5



1 Basement Plan

Project	Champlain Towers South - Joint Testing Protocol	Proj. No.	2021.4155
Sheet Title	8777 Colne Ave., Burlington, Florida 33154	Date	December 10, 2021
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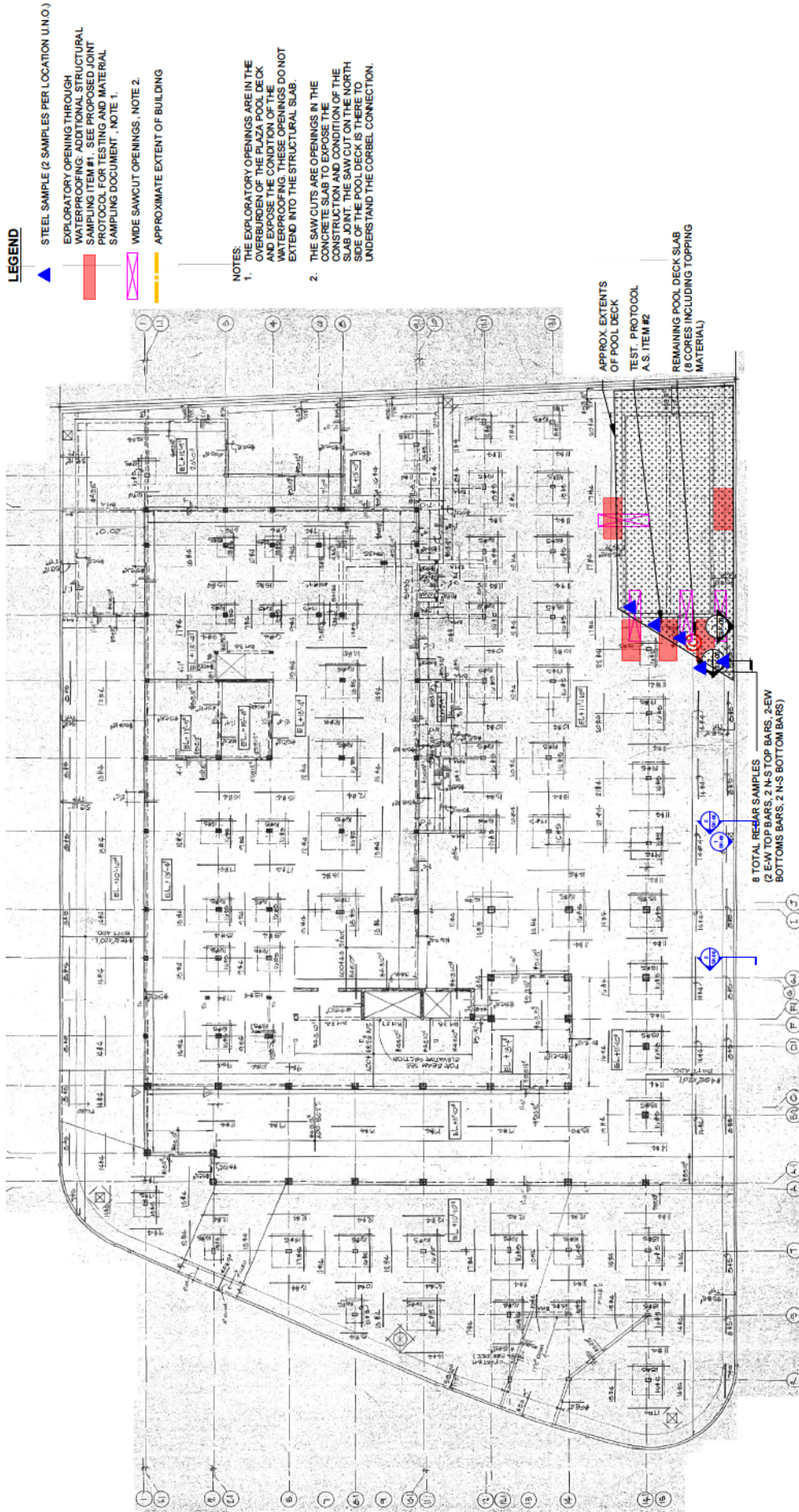
SK-04

Sheet No.

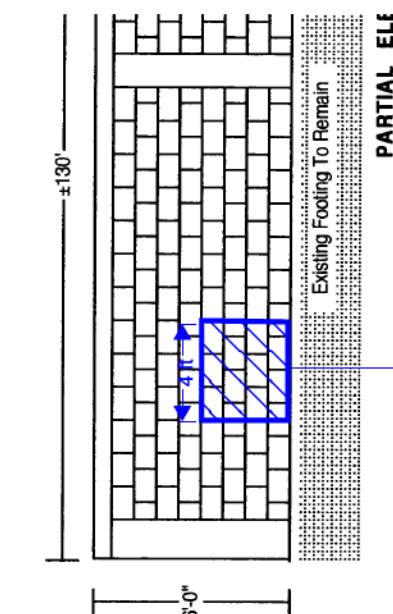
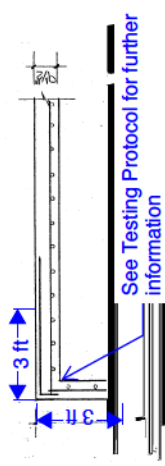
* BASE PLAN FROM DRAWINGS TITLED "CHAMPLAIN TOWERS SOUTH" PREPARED BY BREITHEIM JORDAO & ASSOCIATES CONSULTING ENGINEERS, DATED 07/27/19

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Appendix B Champlain Towers South Investigation - Joint Protocol for Testing and Material Sampling (Collapse Site)



Project Champlain Towers South - Joint Testing Protocol 8777 Collins Ave., Suite 3154 Miami, FL 33154		Proj. No. 2021.4155 Date December 10, 2021 Drawn SKA Checked MFF Scale As Noted
Engineers WJE ARCHITECTS MATERIALS SCIENTISTS Wm. J. E. Associates, Inc. 330 Piquette Road Northbrook, IL 60062 Tel: 847.277.2600 Fax: 847.490.0344 www.wje.com		SK-05 Sheet No.
* BASE PLAN FROM DRAWINGS TITLED "CHAMPLAIN TOWERS SOUTH" PREPARED BY BREITERMAN JURADO & ASSOCIATES CONSULTING ENGINEERS, DATED 9/27/19 File Name: C:\WJE\1012021\13-29-PHASE 1\Drawings\SK-05 - Champlain Towers South Pool Deck Level.dwg		



See Testing Protocol for further information

TESTING PROTOCOL ADDITIONAL
STRUCTURAL ITEM #3

1

TESTING PROTOCOL ADDITIONAL
STRUCTURAL ITEM #4

2

NOTE:
AT GRIDLINE K, REMOVAL CONSISTS OF THE
PRIVACY WALL, FOUNDATION WALL, AND
RESIDUAL SLAB.
AT GRIDLINE H, REMOVAL CONSISTS OF THE
FOUNDATION WALL AND RESIDUAL SLAB.

* BASE PLAN FROM UNDATED DRAWING SHEET NO. S1 TITLED CHAMPLAIN TOWERS SOUTH
PREPARED BY BREITERMAN JURADO & ASSOCIATES CONSULTING ENGINEERS

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Revised: January 13, 2022

WJE ENGINEERS ARCHITECTS METEOROLOGISTS Wesley, Jerry, Elmer Associates, Inc. 330 Phipps Road Naples, Florida 34102 813.277.2400 (N) 813.490.0304 (F) www.wje.com	Project	Champlain Towers South - Joint Testing Protocol
	Sheet Title	8777 Collins Ave., Surfside, Florida 33154
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Additional Items		Scale As Noted
Sheet No.		SK-06