

**IN THE CIRCUIT COURT OF THE ELEVENTH JUDICIAL
CIRCUIT IN AND FOR MIAMI-DADE COUNTY, FLORIDA**

CASE NO: 2021-015089-CA-01

SECTION: CA43

JUDGE: Michael Hanzman

In re:

Champlain Towers South Collapse Litigation

**RECEIVER'S EMERGENCY MOTION FOR ENTRY OF ORDER AUTHORIZING
RECEIVER TO SIGN PERMIT APPLICATION REQUESTED BY MIAMI-DADE
COUNTY AUTHORIZING BOFAM CONSTRUCTION COMPANY, INC.
TO UNDERTAKE EMERGENCY WORK TO BRACE THE
RETAINMENT WALLS AT THE PROPERTY**

Michael I. Goldberg (the "Receiver"), pursuant to Rule 4 of the Complex Business Litigation Rules, seeks entry, on an emergency basis, of an order authorizing the Receiver to sign a permit application requested by the Miami-Dade County Department of Regulatory and Economic Resources (the "County") authorizing Bofam Construction Company, Inc. ("Bofam") to undertake emergency work to brace the retainment walls at the former site of the Champlain Tower South Condominium located at 8777 Collins Avenue, Surfside, Florida 33154 (the "Property"). In support of this Motion, the Receiver states as follows:

1. At a hearing conducted on July 2, 2021 the Court ordered the appointment of Michael I. Goldberg as receiver for the Champlain Tower South Condominium Association.
2. This receivership is the result of multiple lawsuits that were filed after the tragic collapse of the Champlain Tower Condominium previously built on the Property.
3. The County maintains control over the Property as the County and NIST investigate the circumstances leading up to the collapse of the Champlain Tower South Condominium. The County has notified the Receiver that it believes that the retaining walls at the Property need to be buttressed for life safety reasons, otherwise, the retaining walls are at the risk of collapsing

potentially causing personal injury to those at or near the Property as well as property damage to the site and nearby structures, including adjacent roadways.

4. To that end, the County has hired Bofam to perform the necessary work to secure the retaining walls. The work Bofam intends to perform is more fully set forth in the engineering plans attached to this Motion as **Composite Exhibit "A"**.

5. The County has assured the Receiver that the work to be undertaken is necessary for life-safety reasons and will be undertaken in a manner best designed to preserve the evidence in accordance with this Court's prior orders. The Receiver is not an engineering expert and is relying on the County and its experts that the work to be undertaken is necessary and will preserve the evidentiary value of the Property as best as possible under the exigent circumstances.

6. The Receiver is technically the "owner" of the Property even though the Property is currently under the County's control. Accordingly, the County has requested the Receiver to execute the Permit Application filed by Bofam. A true and correct copy of the Permit Application is attached hereto as **Exhibit B**. The Permit Application identifies the Applicant, provides the Applicant's Contractor and Qualifier Numbers and a \$500,000 estimate for the emergency shoring work to be performed. Bofam seeks issuance of a new permit authorizing it to perform emergency shoring work in respect of the Property.

7. The Receiver has informed the County that he will not execute the permit application unless and until he receives Court authorization to do so, and only after notice and a hearing in which all parties in interest have an opportunity to consider the Motion and express their position to the Court with respect to the intended work and its potential impact on the Property.

8. The Receiver has conferred with lead counsel for the Plaintiffs who, in turn, has conferred with his group, and Plaintiffs have no objection to the relief requested in this Motion.

WHEREFORE, the Receiver respectfully requests that the Court enter an Order authorizing him to approve or sign-off on the Permit Application, to the extent necessary, and grant such other, further and related relief as may be appropriate under the circumstances.

Dated: August 5, 2021

Respectfully submitted,

BERGER SINGERMAN, LLP
Counsel for the Receiver
1450 Brickell Ave., Ste. 1900
Miami, Florida 33131
Telephone: (305) 755-9500
Fax: (305) 714-4340

By: s/ Paul Steven Singerman

Paul Steven Singerman
Florida Bar No. 378860
Jordi Gusó
Florida Bar No. 863580
Paul A. Avron
Florida Bar No. 50814
DRT@bergersingerman.com
Singerman@bergersingerman.com
Jguso@bergersingerman.com
pavron@bergersingerman.com
MDiaz@bergersingerman.com
Fsellars@bergersingerman.com

CERTIFICATE OF SERVICE

I HEREBY CERTIFY that on August 5, 2021, a copy of the foregoing was electronically filed with the Clerk of Court by using the Florida Courts E-Filing Portal, which served a copy of same to all counsel of record through the Florida Court's E-Filing Portal. I further certify that a true and correct copy of the foregoing was served by electronic transmission and first class, U.S. Mail on August 5, 2021, upon all parties on the attached Service List.

By: s/ Paul Steven Singerman
Paul Steven Singerman

SERVICE LIST

Austin Akinrin, President
Bofam Construction Co., Inc.
1600 NW 3rd Avenue
Miami, FL 33136
austin@bofaminc.com

COMPOSITE EXHIBIT “A”

GENERAL NOTES

CODES

1. FLORIDA BUILDING CODE 2020
2. A.S.C.E. 7-16 FOR WIND ANALYSIS
3. ACI 318-14 - BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE
4. AISC 14TH EDITION
5. AASHTO BRIDGE DESIGN SPECIFICATIONS 2017

LOADS - DESIGN CRITERIA

1. WIND PARAMETERS:
"ROOF" HEIGHT 10'-0"
BASIC WIND VELOCITY: 196 MPH (ULT)
RISK CATEGORY: IV
EXPOSURE C
Kd 0.85
PARTIALLY ENCLOSED BUILDING INTERNAL PRESSURE COEFFICIENT: +/-0.55
2. OTHERS LOADS:
LIVE SURCHARGE: 480 PCF

FOUNDATIONS

1. EXISTING STRUCTURAL CONCRETE SLAB. THICKNESS VARIES BETWEEN 6½" TO 9" (BASED ON INFORMATION PROVIDED IN MICRO FIELD MEASUREMENTS AND INFORMATION PROVIDED BY THE CONTRACTOR) SUPPORTED ON PILES.
2. SOIL PARAMETERS USED:
 - Ka= 0.33
 - γ_{Dry} = 100pcf
 - γ_{Sat} = 120pcf
 - γ_{SW} = 65pcf

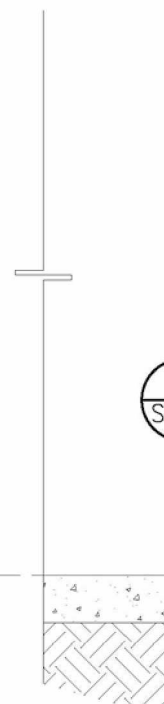
CONCRETE AND REINFORCING STEEL

1. SUBMIT CONCRETE MIX DESIGN TO ENGINEER FOR REVIEW PRIOR TO POURING.
2. ALL NORMAL-WEIGHT CONCRETE MUST BE MIXED IN ACCORDANCE WITH ACI 301 TO ACHIEVE A MINIMUM 28-DAY COMPRESSIVE 5,000PSI. WATER/CEMENT RATIO MUST BE 0.40 MAX. BY WEIGHT UON ON PLANS AND 0.15 MAX. WATER-SOLUBLE CHLORIDE ION CONCRETE (PERCENT BY WEIGHT OF CEMENT) IN ACCORDANCE TO TABLE 4.3.1 OF ACI 318-08.
3. SLUMP SHALL BE 4" (± 1).
4. CONTRACT AN INDEPENDENT TESTING LABORATORY TO PERFORM THE CONCRETE CYLINDER SAMPLING AND TESTING AS REQUIRED BY BUILDING CODE, 2020. SUBMIT TEST REPORTS TO ENGINEER.
5. AGGREGATES TO BE CLEAN AND WELL GRADED, MAX. SIZE 1". VERTICAL DROP NOT TO EXCEED 8'-0".
6. NO ADDITIONAL WATER SHALL BE ADDED TO THE CONCRETE MIX AT THE JOBSITE UNLESS PREVIOUSLY APPROVED.
7. PROVIDE ALL FORMING AND TEMPORARY SHORING.



WESTERNMOST LANE (MUST REMAIN CLOSED TO TRAFFIC)
MUST REMAIN CLOSED TO TRAFFIC
MUST REMAIN CLOSED TO TRAFFIC

(11) DIAG. STEEL BRACING @ 10'-0" MAX. O/C (TOP OF BEAM)
HEIGHT = ±88" FROM BASEMENT SLAB V.I.F.)
EXIST. D. COLLECT



SECT

3/8"=1'-

COLLINS
(MIDDLE
(NORMAL



STRUCTURAL CALCULATIONS

PROJECT: PROPOSED RETAINING WALL BRACING FOR CHAMPLAIN TOWERS COLLAPSE SITE
ADDRESS: 8777 COLLINS AVENUE, SURFSIDE, FL, 33154

APPLICABLE CODES

- FBC 2020
- ASCE 7-16
- AASHTO BRIDGE DESIGN SPECIFICATIONS
- ACI 318-14
- AISC 14TH EDITION

These Calculations contain manual and computer-generated structural calculations. Computations were performed to the best of my knowledge according to sound and generally accepted engineering principals and Code requirements. The sign and seal provided herein is meant to cover all computation sheets pages 1 through 15.

Masood Hajali, PhD, P.E.
Florida Reg.: 82038



PROJECT: PROPOSED RETAINING WALL BRACING FOR CHAMPLAIN TOWERS COLLAPSE SITE
ADDRESS: 8777 COLLINS AVENUE, SURFSIDE, FL, 33154

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Computer Programs Used:

- Mecawind
- Enercalc



1. DESIGN CRITERIA

Design Parameters

- Florida Building Code, 2020 Edition
- Wind Parameters:
 - ASCE 7-16
 - Wind Velocity= 195 MPH (ULT)
 - Risk Category= IV
 - Exposure= D
 - Mean Height=10'-0"
 - Kd= 0.85
 - GCpi= ± 0.55

Materials Used

- Existing Concrete Strength at 28 days - $f'c=3,000\text{psi}$ (as per Microfilm Sheet S-11 of S-14)
- Existing Reinforcing - $f_y=60,000\text{psi}$ (as per Microfilm Sheet S-11 of S-14)
- Structural Steel Tubes ASTM A500 GrB – $f_y=46,000\text{psi}$
- Structural Steel Plates ASTM A36 – $f_y=36,000\text{psi}$

Design Loads

- Refer to Section 2.

Foundation Parameters

- Existing Concrete Structural Slab on Piles and Fill (6 ½" to 9" Thickness).



2. LOADS AND PARAMETERS

LIVE SURCHARGE

LS= 480pcf

WIND LOADS (AWAY FROM SURFACE) (SEE REPORT BELOW)

WLat= 89.2psf (ASD)

SOIL PARAMETERS

Ka= 0.33
 γ_{Dry} = 100pcf
 γ_{Sat} = 120pcf
 γ_{SW} = 65pcf

WIND LOADS FOR PARTIALLY ENCLOSED

MecaWind v2340

Software Developer: Meca Enterprises Inc., www.meca.biz, Copyright © 2018

Basic Wind Parameters

Wind Load Standard	= ASCE 7-16	Exposure Category	= D
Wind Design Speed	= 195.0 mph	Risk Category	= II
Structure Type	= Building	Building Type	= Partially Enclosed

General Wind Settings

Incl LF	= Include ASD Load Factor of 0.6 in Pressures	= True
DynType	= Dynamic Type of Structure	= Rigid
NF	= Natural Frequency of Structure (Mode 1)	= 1.000 Hz
Zg	= Altitude (Ground Elevation) above Sea Level	= 0.000 ft
Bdist	= Base Elevation of Structure	= 0.000 ft
SDB	= Simple Diaphragm Building	= False
zi	= Level of highest opening in building or zero to use h	= 0.0 ft
Reacs	= Show the Base Reactions in the output	= False
MWFRSType	= MWFRS Method Selected	= No Analysis

Topographic Factor per Fig 26.8-1

Topo	= Topographic Feature	= None
Kzt	= Topographic Factor	= 1.000

Building Inputs

RoofType:	Building Roof Type	= Flat	RfHt	: Roof Height	= 10.000 ft
W	: Building Width	= 310.000 ft	L	: Building Length	= 200.000 ft
Frames	: Incl Transverse Frames	= False	n	: Number of Frames	= 3
e	: Solidity Ratio	= 0.350	Par	: Is there a Parapet	= False
Aog	: Tot Area of Openings	= 0.00 sq ft	Vi	: Unpart Int Volume	= 0.00 ft^3

Exposure Constants per Table 26.11-1:

Alpha:	Const from Table 26.11-1	= 11.500	Zg:	Const from Table 26.11-1	= 700.000 ft
At:	Const from Table 26.11-1	= 0.087	Bt:	Const from Table 26.11-1	= 1.070
Am:	Const from Table 26.11-1	= 0.111	Bm:	Const from Table 26.11-1	= 0.800
C:	Const from Table 26.11-1	= 0.150	Eps:	Const from Table 26.11-1	= 0.125

Overhang Inputs:

Std	= Overhangs on all sides are the same	= True
OHType	= Type of Roof Wall Intersections	= None

Components and Cladding (C&C) Calculations per Ch 30 Part 1:

h/W	= Ratio of mean roof height to building width	= 0.032
h/L	= Ratio of mean roof height to building length	= 0.050



h = Mean Roof Height above grade = 10.000 ft
 Kh = $Z < 15 \text{ ft [4.572 m]} \rightarrow (2.01 * (15/zg)^{(2/\text{Alpha})}) \text{ [Table 26.10-1]} = 1.030$
 Kzt = Topographic Factor is 1 since no Topographic feature specified = 1.000
 Kd = Wind Directionality Factor per Table 26.6-1 = 0.85
 Ri = Reduction Factor for Partially Enclosed Large Volume Buildings = 1.000
 GCpi = Ref Table 26.13-1 for Partially Enclosed Building: $0.55 * Ri = +/-0.55$
 LF = Load Factor based upon ASD Design = 0.60
 qh = $(0.00256 * Kh * Kzt * Kd * Ke * V^2) * LF = 51.15 \text{ psf}$
 LHD = Least Horizontal Dimension: $\text{Min}(B, L) = 200.000 \text{ ft}$
 a1 = $\text{Min}(0.1 * LHD, 0.4 * h) = 4.000 \text{ ft}$
 a = $\text{Max}(a1, 0.04 * LHD, 3 \text{ ft [0.9 m]}) = 8.000 \text{ ft}$
 h/B = Ratio of mean roof height to least hor dim: $h / B = 0.050$
 $0.2 * h$ = Parameter used to define Zone 3 = 2.000 ft
 $0.6 * h$ = Parameter used to define Zones 1 and 2 = 6.000 ft

Wind Pressures for C&C Ch 30 Pt 1
All wind pressures include a load factor of 0.6

Description ft	Zone	Width ft	Span ft	Area sq ft	1/3 Rule	Ref Fig	GCp Max	GCp Min	P Max psf	P Min psf
Wall	5	1.000	7.000	16.33	Yes	30.3-1	0.866	-1.192	72.43	-89.11

Area = Span Length x Effective Width
 1/3 Rule = Effective width need not be less than 1/3 of the span length
 GCp = External Pressure Coefficients taken from Figures 30.3-1 through 30.3-7
 p = Wind Pressure: $qh * (GCp - GCpi) \text{ [Eqn 30.3-1]} *$
 * Per Para 30.2.2 the Minimum Pressure for C&C is 9.60 psf [0.460 kPa] {Includes LF}
 Since Roof Slope $\leq 10 \text{ Deg}$, the GCp value is reduced by 10%

RETAINING WALL LOAD CASES FACTORS

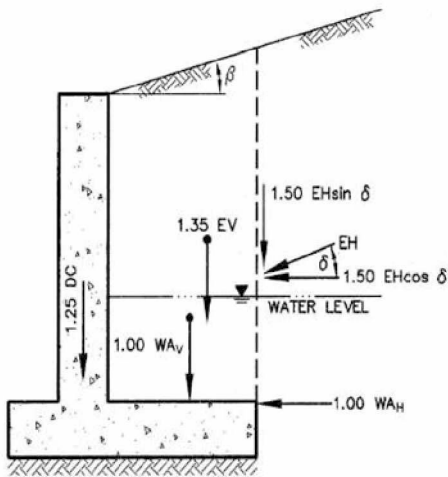


Figure C11.5.6-1—Typical Application of Load Factors for Bearing Resistance

3. RETAINING WALL LOAD ANALYSIS

γ' soil 100 pcf
 γ conc 150 pcf

γ_s soil 120 pcf
 γ_w 65 pcf

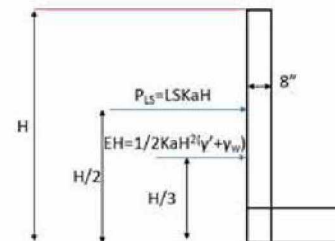
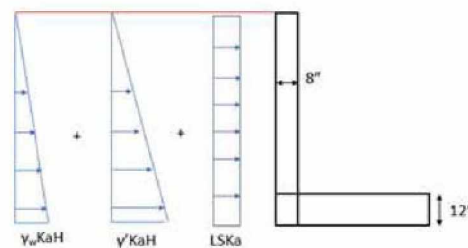
LOAD CASE EH			
H (ft)	Ka	Centroid (ft)	PEH(lb/ft)
7	0.33	2.33	809
8	0.33	2.67	1056
9	0.33	3.00	1337
10	0.33	3.33	1650

LOAD CASE WA			
H (ft)	Ka	Centroid (ft)	PEH(lb/ft)
7	0.33	2.33	526
8	0.33	2.67	686
9	0.33	3.00	869
10	0.33	3.33	1073

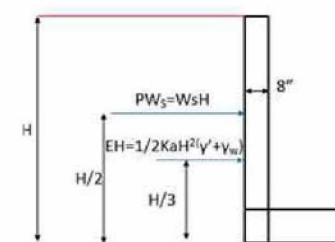
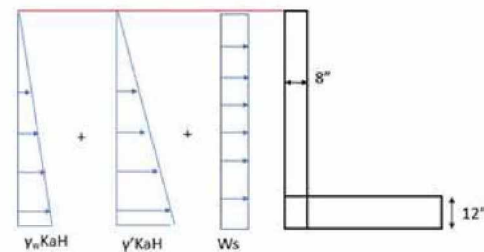
LOAD CASE LS W/ Heq=4ft & q=480 psf			
H (ft)	Ka	Centroid (ft)	PLs(lb/ft)
7	0.33	3.50	1109
8	0.33	4.00	1267
9	0.33	4.50	1426
10	0.33	5.00	1584

LOAD CASE WS W/ qws=89.2 psf		
H (ft)	Centroid (ft)	PWs(lb/ft)
7	3.50	624
8	4.00	714
9	4.50	803
10	5.00	892

Loads for strength I



Loads for strength III



WALL ACTNG AS SIMPLY SUPPORTED BEAM

EH	Soil pressure							
H (ft)	Centroid (ft)	PEH(lb/ft)	a (ft)	b (ft)	Top R (lb)	Bot R (lb)	B in L/3 (lb-ft)	B in L/2 (lb-ft)
7	2.33	808.50	4.67	2.33	270	539	1258	314
8	2.67	1056.00	5.33	2.67	352	704	1877	469
9	3.00	1336.50	6.00	3.00	446	891	2673	668
10	3.33	1650.00	6.67	3.33	550	1100	3667	917

WA	Hydrostatic pressure							
H (ft)	Centroid (ft)	PEH(lb/ft)	a (ft)	b (ft)	Top R (lb)	Bot R (lb)	B in L/3 (lb-ft)	B in L/2 (lb-ft)
7	2.33	525.53	4.67	2.33	175	350	817	204
8	2.67	686.40	5.33	2.67	229	458	1220	305
9	3.00	868.73	6.00	3.00	290	579	1737	434
10	3.33	1072.50	6.67	3.33	358	715	2383	596

PLS	Live surcharge							
H (ft)	0.00	0.00	a (ft)	b (ft)	Top R (lb)	Bot R (lb)	B in L/3 (lb-ft)	B in L/2 (lb-ft)
7	3.50	1108.80	3.50	3.50	554	554	1940	1294
8	4.00	1267.20	4.00	4.00	634	634	2534	1690
9	4.50	1425.60	4.50	4.50	713	713	3208	2138
10	5.00	1584.00	5.00	5.00	792	792	3960	2640

(Strength I)

Shear superposition	Top R (lb)					Bot R (lb)			
Factor/	1.5	1	1.75	Ult	Unfactored	1.5	1	1.75	Ult
H (ft)	Gs	Gw	LS	Gs+Ls+Gw	Gs+Ls+Gw	Gs	Gw	LS	Gs
7	270	175	554	1550	999	539	350	554	
8	352	229	634	1866	1214	704	458	634	
9	446	290	713	2205	1448	891	579	713	
10	550	358	792	2569	1700	1100	715	792	

Moment superposition	B in L/3 (lb-ft)					B in L/2 (lb-ft)			
Factor/	1.5	1	1.75	Ult	Unfactored	1.5	1	1.75	Ult
H (ft)	Gs	Gw	LS	Gs+Ls+Gw	Gs+Ls+Gw	Gs	Gw	LS	Gs
7	1258	817	1294	4968	3369	314	204	1940	
8	1877	1877	1690	7650	5444	469	305	2534	
9	2673	2673	2138	10425	7484	668	434	3208	
10	3667	3667	2640	13787	9973	917	596	3960	

PWS	Wind load							
H (ft)	0.00	0.00	a (ft)	b (ft)	Top R (lb)	Bot R (lb)	B in L/3 (lb-ft)	B in L/2 (lb-ft)
7	3.50	624.40	3.50	3.50	312	312	1093	728
8	4.00	713.60	4.00	4.00	357	357	1427	951
9	4.50	802.80	4.50	4.50	401	401	1806	1204
10	5.00	892.00	5.00	5.00	446	446	2230	1487

(Strength III)

Shear superposition	Top R (lb)					Bot R (lb)			
Factor/	1.5	1	1.4	Ult	Unfactored	1.5	1	1.4	Ult
H (ft)	Gs	Gw	WS	Gs+Ls+Gw	Gs+Ls+Gw	Gs	Gw	WS	Gs
7	270	175	312	1017	757	539	350	554	
8	352	229	357	1256	938	704	458	634	
9	446	290	401	1520	1136	891	579	713	
10	550	358	446	1807	1354	1100	715	792	

Moment superposition	B in L/3 (lb-ft)					B in L/2 (lb-ft)			
Factor/	1.5	1	1.4	Ult	Unfactored	1.5	1	1.4	Ult
H (ft)	Gs	Gw	WS	Gs+Ls+Gw	Gs+Ls+Gw	Gs	Gw	LS	Gs
7	1258	817	728	3724	2804	314	204	1093	
8	1877	1877	951	6025	4706	469	305	1427	
9	2673	2673	1204	8368	6550	668	434	1806	
10	3667	3667	1487	11248	8820	917	596	2230	



4. BEAM AND BRACING DESIGN

Beam Design at top of Wall)

Span= 10'-0" (Multiple)

From Previous Section, for Critical Case Strength I,
Uniform Load = 1700plf (Unfactored)

From attached calculations use HSS 6"x6"x3/8".

Reactions

RMax= 17.0kips (ASD) for Typical Span

Brace Design)

Max. Span= $8' / \sin 45 = 11'-4"$
Point Load= 17kips (ASD)
Max. Axial= $17 / \sin 45 = 24.1\text{kips}$

Reactions

Pmax= 17kips (ASD)
Vmax= $17 * 1.51 = 25.7\text{kips}$ (ULT)

From attached calculations use HSS 4"x4"x3/8" w/3/4"x12"x16" base plate and (6) 5/8" Diameter Hilti HAS Rods SS 304 w/HIT-RE 500V3 Epoxy (4" emb.)

Check Welds

For 1/4" E70xx,
Vall= 3.71kip/in
Available Length= $4 / \sin 45 = 5.65\text{in} * 2 = 11.3\text{in}$
Allowable Shear= $3.71 * 11.3 = 42\text{kips} > 17\text{kips}$ OK (See attached table)

Rev: 580002

Multi-Span Steel Beam

Description Steel Beam

General Information

Code Ref: AISC 9th ASD, 1997 UBC, 2003 IBC, 2003 NFPA 5000

Fy - Yield Stress 46.00 ksi Load Duration Factor 1.00
Spans Considered Continuous Over Supports

Span Information

Description									
Span	ft	10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00
Steel Section		HSS6X6X3/8	HSS6X6X3/8	HSS6X6X3/8	HSS6X6X3/8	HSS6X6X3/8	HSS6X6X3/8	HSS6X6X3/8	HSS6X6X3/8
End Fixity		Pin-Pin	Pin-Pin	Pin-Pin	Pin-Pin	Pin-Pin	Pin-Pin	Pin-Pin	Pin-Pin
Unbraced Length	ft	10.00	10.00	10.00	10.00	10.00	10.00	10.00	10.00

Loads

Live Load Used This Span ?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Dead Load	k/ft								
Live Load	k/ft	1.700	1.700	1.700	1.700	1.700	1.700	1.700	1.700

Results

Mmax @ Cntr	k-ft	13.22	5.76	7.45	7.01	7.01	7.45	5.76	13.22
@ X =	ft	3.93	5.27	4.93	5.00	5.00	5.07	4.73	6.07
Max @ Left End	k-ft	0.00	-17.96	-13.14	-14.46	-14.02	-14.46	-13.14	-18.0
Max @ Right End	k-ft	-17.96	-13.14	-14.46	-14.02	-14.46	-13.14	-17.96	0.00
fb : Actual	psi	16,372.2	16,372.2	13,177.6	13,177.6	13,177.6	13,177.6	16,372.2	16,372.2
Fb : Allowable	psi	27,600.0	27,600.0	27,600.0	27,600.0	27,600.0	27,600.0	27,600.0	27,600.0
		Bending OK	Bending OK	Bending OK	Bending OK	Bending OK	Bending OK	Bending OK	Bending OK
fv : Actual	psi	2,458.5	2,144.7	2,061.0	2,040.1	2,040.1	2,061.0	2,144.7	2,458.5
Fv : Allowable	psi	18,400.0	18,400.0	18,400.0	18,400.0	18,400.0	18,400.0	18,400.0	18,400.0
		Shear OK	Shear OK	Shear OK	Shear OK	Shear OK	Shear OK	Shear OK	Shear OK

Reactions & Deflections

Shear @ Left	k	6.70	8.98	8.37	8.54	8.46	8.63	8.02	10.30
Shear @ Right	k	10.30	8.02	8.63	8.46	8.54	8.37	8.98	6.70
Reactions...									
DL @ Left	k	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
LL @ Left	k	6.70	19.28	16.39	17.18	16.91	17.18	16.39	19.28
Total @ Left	k	6.70	19.28	16.39	17.18	16.91	17.18	16.39	19.28
DL @ Right	k	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
LL @ Right	k	19.28	16.39	17.18	16.91	17.18	16.39	19.28	6.70
Total @ Right	k	19.28	16.39	17.18	16.91	17.18	16.39	19.28	6.70
Max. Deflection	in	-0.168	-0.041	-0.074	-0.065	-0.065	-0.074	-0.041	-0.168
@ X =	ft	4.40	5.33	4.93	5.00	5.00	5.07	4.67	5.60
Span/Deflection Ratio		714.8	2,917.8	1,628.8	1,835.4	1,835.4	1,628.8	2,917.8	714.8

Rev: 580002

Steel Column

Description Steel Brace

General Information

Code Ref: AISC 9th ASD, 1997 UBC, 2003 IBC, 2003 NFPA 5000

Steel Section	HSS4X4X3/8	Fy	46.00 ksi	X-X Sidesway :	Restrained
		Duration Factor	1.000	Y-Y Sidesway :	Restrained
Column Height	11.330 ft	Elastic Modulus	29,000.00 ksi		
End Fixity	Pin-Pin	X-X Unbraced	11.330 ft	Kxx	1.000
Live & Short Term Loads Combined		Y-Y Unbraced	11.330 ft	Kyy	1.000

Loads

Axial Load...				
Dead Load	k	Ecc. for X-X Axis Moments	0.000 in	
Live Load	24.10 k	Ecc. for Y-Y Axis Moments	0.000 in	
Short Term Load	k			

Summary

Column Design OK

Section : HSS4X4X3/8, Height = 11.33ft, Axial Loads: DL = 0.00, LL = 24.10, ST = 0.00k, Ecc. = 0.000in

Unbraced Lengths: X-X = 11.33ft, Y-Y = 11.33ft

Combined Stress Ratios	Dead	Live	DL + LL	DL + ST + (LL if Chosen)
AISC Formula H1 - 1		0.3189	0.3189	0.3189
AISC Formula H1 - 2		0.1827	0.1827	0.1827
AISC Formula H1 - 3				

XX Axis : Fa calc'd per Eq. E2-1, $K^*L/r < C_c$

YY Axis : Fa calc'd per Eq. E2-1, $K^*L/r < C_c$

Stresses

Allowable & Actual Stresses	Dead	Live	DL + LL	DL + Short
Fa : Allowable	15.81 ksi	15.81 ksi	15.81 ksi	15.81 ksi
fa : Actual	0.00 ksi	5.04 ksi	5.04 ksi	5.04 ksi
Fb:xx : Allow [F3.1]	27.60 ksi	27.60 ksi	27.60 ksi	27.60 ksi
fb : xx Actual	0.00 ksi	0.00 ksi	0.00 ksi	0.00 ksi
Fb:yy : Allow [F3.1]	27.60 ksi	27.60 ksi	27.60 ksi	27.60 ksi
fb : yy Actual	0.00 ksi	0.00 ksi	0.00 ksi	0.00 ksi

Analysis Values

Pex : DL+LL	17,408 psi	Cm:x DL+LL	0.60	Cb:x DL+LL	1.00
Pey : DL+LL	17,408 psi	Cm:y DL+LL	0.60	Cb:y DL+LL	1.00
Pex : DL+LL+ST	17,408 psi	Cm:x DL+LL+ST	0.60	Cb:x DL+LL+ST	1.00
Pey : DL+LL+ST	17,408 psi	Cm:y DL+LL+ST	0.60	Cb:y DL+LL+ST	1.00
Max X-X Axis Deflection	0.000 in at	0.000 ft	Max Y-Y Axis Deflection	0.000 in at	0.000 ft

Rev: 580002

Steel Column

Description Steel Brace

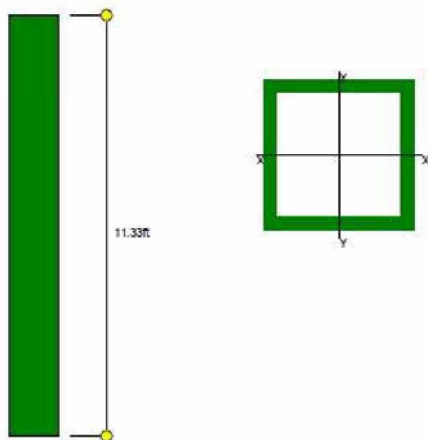
Section Properties HSS4X4X3/8

Depth	4.000 in	Weight	16.24 #/ft	Values for LRFD Design....	
Web Thick	0.349 in	Ixx	10.300 in4	J	17.500 in4
Width	4.000 in	Iyy	10.300 in4	Cw	9.14 in6
Flange Thick	0.349 in	Sxx	5.130 in3	Zx	6.390 in3
Area	4.78 in2	Syy	5.130 in3	Zy	6.390 in3
Rt	0.000 in	Rxx	1.470 in		0.000
		Ryy	1.470 in		

Section Type = HSS-Square

Sketch & Diagram

Axial DL = 0k
Axial LL = 24.1k
Axial ST = 0k



Rev: 580000

Steel Column Base Plate

Description Base Plate

General Information

Code Ref : AISC 9th Ed ASD, 1997 UBC, 2003 IBC, 2003 NFPA 5000

Loads		Steel Section		HSS4x4x3/8	
Axial Load	20.00 k	Section Length	4.000 in		
X-X Axis Moment	0.00 k-ft	Section Width	4.000 in		
		Flange Thickness	0.349 in		
		Web Thickness	4.000 in		
Plate Dimensions		Allowable Stresses			
Plate Length	16.000 in	Concrete f'c	3,000.0 psi		
Plate Width	12.000 in	Base Plate Fy	36.00 ksi		
Plate Thickness	0.688 in	Load Duration Factor	1.000		
Support Pier Size		Anchor Bolt Data			
Pier Length	36.000 in	Dist. from Plate Edge	1.500 in		
Pier Width	36.000 in	Bolt Count per Side	3		
		Tension Capacity	5.500 k		
		Bolt Area	0.442 in ²		

Summary

Baseplate OK

Concrete Bearing Stress Bearing Stress OK
 Actual Bearing Stress 104.2 psi
 Allow per ACI318-95, A3.1
 $= 0.3 \cdot f'c \cdot \sqrt{A2/A1} \cdot LDF$ 1,800.0 psi
 Allow per AISC J9 2,100.0 psi

Full Bearing : No Bolt Tension

Plate Bending Stress Thickness OK
 Actual fb 24,601.7 psi
 Max Allow Plate Fb 27,000.0 psi

Tension Bolt Force Bolt Tension OK
 Actual Tension 0.000 k
 Allowable 5.500 k

Table 5.13.1 Allowable Resistance of Fillet Welds, kips/in.
 (Shielded Metal Arc Welding)

Allowable Shear in Fillet Welds, R_w , (kips/in. of weld)							
Nominal Size (in.)	Effective Throat (AISC-1.14.6) (in.)	Minimum Tensile Strength of Weld (ksi)					
		60	70	80	90	100	110
1/8	0.088	1.59	1.86	2.12	2.39	2.69	2.92
3/16	0.132	2.38	2.78	3.18	3.58	3.97	4.37
1/4	0.177	3.18	3.71	4.24	4.77	5.30	5.83

Company:
Specifier:
Address:
Phone | Fax:
E-Mail:

Page:
Project:
Sub-Project | Pos. No.:
Date:

1
CT Collapse Site
Base Anchors
8/3/2021

Specifier's comments:

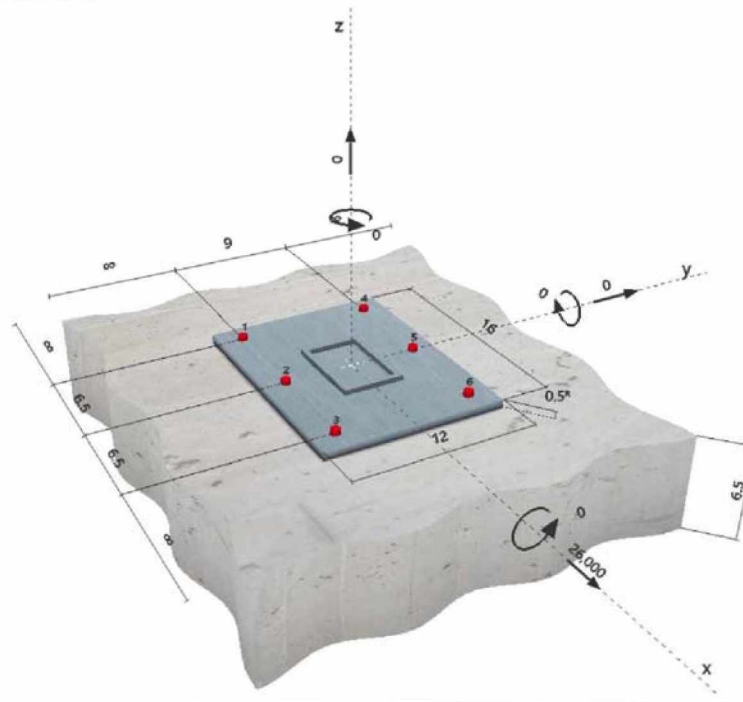
1 Input data

Anchor type and diameter:	HIT-RE 500 V3 + HAS-R 304/316 SS 5/8
Effective embedment depth:	$h_{ef,act} = 4.000$ in. ($h_{ef,limit} = -$ in.)
Material:	ASTM F 593
Evaluation Service Report:	ESR-3814
Issued Valid:	1/1/2020 1/1/2021
Proof:	Design method ACI 318-08 / Chem
Stand-off installation:	$e_b = 0.000$ in. (no stand-off); $t = 0.500$ in.
Anchor plate:	$l_x \times l_y \times t = 16.000$ in. \times 12.000 in. \times 0.500 in.; (Recommended plate thickness: not calculated)
Profile:	Rectangular HSS (AISC), HSS6X4X.375; ($L \times W \times T$) = 6.000 in. \times 4.000 in. \times 0.375 in.
Base material:	cracked concrete, 3000, $f_c' = 3,000$ psi; $h = 6.500$ in., Temp. short/long: 32/32 °F
Installation:	hammer drilled hole, Installation condition: Dry
Reinforcement:	tension: condition B, shear: condition B; no supplemental splitting reinforcement present edge reinforcement: none or < No. 4 bar
Seismic loads (cat. C, D, E, or F)	no



R - The anchor calculation is based on a rigid anchor plate assumption.

Geometry [in.] & Loading [lb, in.lb]



Input data and results must be checked for agreement with the existing conditions and for plausibility!
PROFIS Anchor (c) 2003-2009 Hilti AG, FL-9494 Schaan Hilti is a registered Trademark of Hilti AG, Schaan

www.hilti.us

Profis Anchor 2.9.2

Company:

Page:

2

Specifier:

Project:

CT Collapse Site

Address:

Sub-Project | Pos. No.:

Base Anchors

Phone | Fax:

Date:

8/3/2021

E-Mail:

2 Proof | Utilization (Governing Cases)

Loading	Proof	Design values [lb]		Utilization	Status
		Load	Capacity	β_N / β_V [%]	
Tension	-	-	-	- / -	-
Shear	Pryout Strength (Bond Strength controls)	26,000	36,715	- / 71	OK

Loading	β_N	β_V	ζ	Utilization $\beta_{N,V}$ [%]	Status
Combined tension and shear loads	-	-	-	-	-

3 Warnings

- Please consider all details and hints/warnings given in the detailed report!

Fastening meets the design criteria!

4 Remarks; Your Cooperation Duties

- Any and all information and data contained in the Software concern solely the use of Hilti products and are based on the principles, formulas and security regulations in accordance with Hilti's technical directions and operating, mounting and assembly instructions, etc., that must be strictly complied with by the user. All figures contained therein are average figures, and therefore use-specific tests are to be conducted prior to using the relevant Hilti product. The results of the calculations carried out by means of the Software are based essentially on the data you put in. Therefore, you bear the sole responsibility for the absence of errors, the completeness and the relevance of the data to be put in by you. Moreover, you bear sole responsibility for having the results of the calculation checked and cleared by an expert, particularly with regard to compliance with applicable norms and permits, prior to using them for your specific facility. The Software serves only as an aid to interpret norms and permits without any guarantee as to the absence of errors, the correctness and the relevance of the results or suitability for a specific application.
- You must take all necessary and reasonable steps to prevent or limit damage caused by the Software. In particular, you must arrange for the regular backup of programs and data and, if applicable, carry out the updates of the Software offered by Hilti on a regular basis. If you do not use the AutoUpdate function of the Software, you must ensure that you are using the current and thus up-to-date version of the Software in each case by carrying out manual updates via the Hilti Website. Hilti will not be liable for consequences, such as the recovery of lost or damaged data or programs, arising from a culpable breach of duty by you.

EXHIBIT “B”

NOTE: ALL SHEETS MUST BE REVIEWED

DEPARTMENT OF REGULATORY AND ECONOMIC RESOURCES

Herbert S. Saffir Permitting and Inspection Center

11805 SW 26th Street (Coral Way), • Miami, Florida 33175-2474 • (786) 315-2000

PERMIT APPLICATION

123, 01-52 PAGE 1 9/17

IF SUBSIDIARY PROVIDE MASTER PERMIT NUMBER HERE			
LOCATION OF IMPROVEMENTS	Job Address <u>8777 Collins Avenue</u> Folio <u>14-2235-025-</u> Lot _____ Block _____ Subdivision _____ PBpg _____ Metes and bounds _____	CONTRACTOR INFORMATION	Contractor No. <u>CGC062660</u> Last four (4) digits of Qualifier No. <u>6086</u> Contractor Name <u>Bofam Construction Co, Inc</u> Qualifier Name <u>Gbolahan Austin Akinrin</u> Address <u>1600 NW 3rd Avenue</u> City <u>Miami</u> State <u>FL</u> Zip <u>33136</u>
TYPE OF IMPROVEMENTS	<div style="display: flex; justify-content: space-between;"> <div style="width: 48%;"> <input type="checkbox"/> New Construction on Vacant Land <input type="checkbox"/> Alteration Interior <input type="checkbox"/> Alteration Exterior <input type="checkbox"/> Relocation of Structure <input type="checkbox"/> Short Term Event <input type="checkbox"/> New Roof <input type="checkbox"/> Recovery (Roof) <input type="checkbox"/> Permit by Affidavit </div> <div style="width: 48%;"> <input type="checkbox"/> Enclosure <input type="checkbox"/> Repair <input type="checkbox"/> Repair Due to Fire <input checked="" type="checkbox"/> Demolish <input type="checkbox"/> Shell Only <input type="checkbox"/> Addition Attached <input type="checkbox"/> Addition Detached <input type="checkbox"/> Re-Roof <input type="checkbox"/> Foundation Only </div> </div>	Current use of property <u>Vacant</u> Description of Work <u>Surfside Emergency Shoring</u> Sq. Ft. _____ Units _____ Floors _____ Value of Work <u>\$500,000</u>	
PERMIT TYPE	<div style="display: flex; justify-content: space-between;"> <div style="width: 48%;"> <input checked="" type="checkbox"/> Building* Category <u>015</u> <input type="checkbox"/> Electrical <input type="checkbox"/> Mechanical <input type="checkbox"/> Plumbing <input type="checkbox"/> LPGX </div> <div style="width: 48%; text-align: center; font-size: x-small; font-weight: bold;">CHANGE TO AN EXISTING PERMIT</div> <div style="width: 48%;"> <input type="checkbox"/> Chg. Contractor <input type="checkbox"/> Re-Issue <input type="checkbox"/> Extension <input type="checkbox"/> Supplement <input type="checkbox"/> Reinspection </div> </div>	OWNER'S NAME	
PERSON TO PICK UP PLANS	Name <u>Austin Akinrin</u> Address <u>1600 NW 3rd Aveue</u> City <u>Miami</u> State <u>FL</u> Zip <u>33136</u> Phone <u>(754) 245-0102</u>	ARCHITECT ENGINEER	
BONDING	Name _____ Address _____ City _____ State _____ Zip _____ Phone _____	MORTGAGE LENDER	
		Name _____ Address _____ City _____ State _____ Zip _____ Phone _____	

*See reverse side for Building Category

Application is hereby made to obtain a permit to do work and installation as indicated. I certify that all work will be performed to meet the standards of all laws regulating construction in this jurisdiction. I understand that separate permits are required for **ELECTRICAL, PLUMBING, SIGNS, POOLS, MECHANICAL, WINDOW, SHUTTERS and ROOFING WORK** and there may be additional permits required for other governmental entities.

OWNER'S/PERMIT APPLICANT AFFIDAVIT I certify that all of the foregoing information is accurate. I certify that I am not a named violator with: unpaid civil penalties; unpaid administrative costs of hearing; unpaid County investigative, enforcement, testing, or monitoring costs; or unpaid liens, any or all of which are owed to Miami-Dade County pursuant to the provisions of the Code of Miami-Dade County, Florida.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR ATTORNEY OR LENDER BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

"The issuance of the permit does not relieve the property owner from obtaining homeowner's association approval (if required) prior to beginning any work and in no way authorizes work that is in violation of any association rule or regulation."

Signature of Owner or Owner's Agent _____

PRINT NAME _____

STATE OF FLORIDA COUNTY OF MIAMI-DADE

Sworn to and subscribed before me this _____

day of _____, 20____,

by _____

Signature of Notary Public _____

Print Name _____

(SEAL)

Personally known _____

or Produced Identification _____

Signature of Qualifier _____

PRINT NAME Gbolahan Austin Akinrin

STATE OF FLORIDA COUNTY OF MIAMI-DADE

Sworn to and subscribed before me this _____

day of August, 2021,

by Gbolahan Austin Akinrin

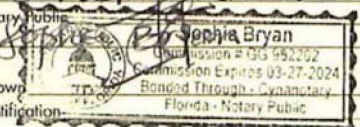
Signature of Notary Public _____

Print Name Josephia Bryan

(SEAL)

Personally known _____

or Produced Identification _____





Class V Dewatering Permit Application Form

For Departmental Use Only

Date Received: _____

Application #: _____

Fee Received: \$ _____

Tracking #: _____

1. Checklist:

☐ Application Fee: Dependent upon duration of dewatering permit (all fees include a 7.5% RER surcharge):

☐ 6 days or less \$520.00+\$39.00 = \$559.00*

☐ 7-30 days \$635.00+\$47.63 = \$682.63**

☐ 31-90 days \$980.00+\$73.50 = \$1053.50**

Note: After-the-Fact Permit applications will be charged a penalty fee amounting to 100% of the original fee, plus departmental administrative enforcement costs.

☐ Complete description of dewatering operation ***

☐ Complete dewatering operation calculations***

☐ Site Plan (site plan shall include project boundaries, location of proposed dewatering activity, sedimentation tanks, turbidity barriers, discharge points, berms, monitoring points, etc.)***

* Permit issued for less than 30 days, CANNOT BE EXTENDED, a new permit application will be required.

** Time extension requests have to file at least thirty calendar days prior to the time of permit expiration.

*** Must be signed and sealed by an engineer, architect or land surveyor, licensed in the State of Florida.

2. Project Information:

Project Name: Surfside Emergency Shoring Folio #: 14-2235-025-

This application is for a(n): ☒ New Permit ☐ After the Fact Permit

Location: 8777 Collins Avenue, Surfside, FL 33154

Section: _____ Township: _____ Range: _____

Municipality: _____

Proposed starting date: 8/1/2021 Estimated completion date: 8/30/2021

Is the proposed work in a contaminated site? ☒ Yes ☐ No ☐ Unknown

If yes, see Attachment "B"

3. Applicant Information:

This should be the applicant's information for contact purposes.

Name: Austin Akinrin

Company: Bofam Construction Co, Inc

Address: 1600 NW 3rd Avenue

Miami, FL _____ Zip Code: 33136

Phone #: 754-245-0102 Fax: 305-675-9269

Email: Austin@bofaminc.com

4. Applicant's Authorized Permit Agent:

Agent is authorized to process the application, furnish supplemental information relating to the application and bind the applicant to all requirements of the application.

Name: _____

Company: _____

Address: _____

_____ Zip Code: _____

Phone #: _____ Fax #: _____

Email: _____

5. Contractor Information:

Name: Austin Akinrin License # (County/State): CGC062660

Company: Bofam Construction Co, Inc

Address: 1600 NW 3rd Avenue, Miami, FL Zip Code: 33136

Phone #: 754-245-0102 Fax#: 305-675-9269 Email: Austin@bofaminc.com

6. Performance Bond and/or Mitigation Fee: (to be assessed by Water Control Section)

- This permit may require a performance bond to guarantee that work is accomplished according to plan and that no impact to adjacent properties is generated as a result of the permitted dewatering activity.
- A mitigation fee may be required to compensate for any loss of or impact to natural resources due to the extent and duration of the dewatering activity.

7. APPLICANT AFFIRMATION:

Application is hereby made for a Miami-Dade County Class V permit to authorize the activities described herein. I agree to or affirm the following:

- I possess the authority to authorize the proposed activities at the subject property, and
- I am familiar with the information, data and plans contained in this application, and
- To the best of my knowledge and belief, the information, data and plans submitted are true, complete and accurate, and
- I will apprise the Department of any changes to information provided in this application, and
- I will provide any additional information, evidence or data necessary to provide reasonable assurance that the proposed project will comply with the applicable State and County water quality standards both during construction and after the project is completed, and
- I am authorizing the permit agent listed in Section 4 of this application to process the application, furnish supplemental information relating to this application and bind the applicant to all requirements of this application, and
- I agree to provide entry to the project site to inspectors and authorized representatives of Miami-Dade County, with proper identification or documents as required by law, for the purpose of preliminary analysis, verification, sampling, monitoring, and observation of permitted activities.

A. IF APPLICANT IS AN INDIVIDUAL

Signature of Applicant

Print Applicant's Name

Date

B. IF APPLICANT IS OTHER THAN AN INDIVIDUAL OR NATURAL PERSON

(Examples: Corporation, Partnership, Trust, LLC, LLP, etc.)

Bofam Construction Co, Inc Corp

Print Name of Applicant (Enter the complete name as registered)

Type (Corp, LLC LLP, etc.)

State of Registration/Incorporation

Under the penalty of perjury, I certify that I have the authority to sign this application on behalf of the Applicant, to bind the Applicant, and if so required, to authorize the issuance of a bond on behalf of the Applicant. (If asked, you must provide proof of such authority to the Department). Please Note: If additional signatures are required, pursuant to your governing documents, operating agreements, or other applicable agreements or laws, you must attach additional signature pages (ATTACHMENT "A").



President

8/3/2021

Signature of Authorized Representative

Print Authorized Representative's Name

Title

Date

C. IF APPLICANT IS A JOINT VENTURE Each party must sign below (If more than two members, list on attached page)

Print Name of Applicant (Enter the complete name as registered)

Type (Corp, LLC LLP, etc.)

State of Registration/Incorporation

Print Name of Applicant (Enter the complete name as registered)

Type (Corp, LLC LLP, etc.)

State of Registration/Incorporation

Under the penalty of perjury, I certify that I have the authority to sign this application on behalf of the Applicant, to bind the Applicant, and if so required to authorize the issuance of a bond on behalf of the Applicant. (If asked, you must provide proof of such authority to the Department). Please Note: If additional signatures are required, pursuant to your governing documents, operating agreements, or other applicable agreements or laws, you must attach additional signature pages (ATTACHMENT "A").

Signature of Authorized Representative

Print Authorized Representative's Name

Title

Date

Signature of Authorized Representative

Print Authorized Representative's Name

Title

Date